

WHITSUNDAYS REGIONAL COUNCIL SEWER AND WATER NETWORK MODELLING

2020 LGIP OPTIMISATION

26 NOVEMBER 2020



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2020 LGIP OPTIMISATION

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1 EXECUTIVE SUMMARY

Whitsunday Regional Council's Water and Waste (Water and Waste) group are seeking explicit guidance from Whitsunday Regional Council (WRC) as a whole on strategic direction regarding water demand and demand management. To inform this discussion Whitsunday Water and Waste have commissioned Arcadis to assess WRC's water and sewer infrastructure and detail what augmentations/upgrades are required to meet projected future demand horizons in light of the recent Norling 2018 population estimation study. This assessment builds on previous Local Government Infrastructure Plan (LGIP) assessments and will provide an updated LGIP for WRC's water and sewer networks and assesses the value / costs of differing demand management / demand accommodative policy settings.

The LGIP infrastructure assessment (in which the current LGIP is based) and the recent revisions to the LGIP were developed based on current EP demands of 500L/EP/day. The outcome of these assessments indicate significant capital expenditure on infrastructure to meet the 2021 horizon and beyond. This impending cost burden was also identified in the 2016 Northern Whitsunday Water Supply Strategy completed by Arcadis and previous iterations of modelling undertaken by Arcadis (Then Hyder).

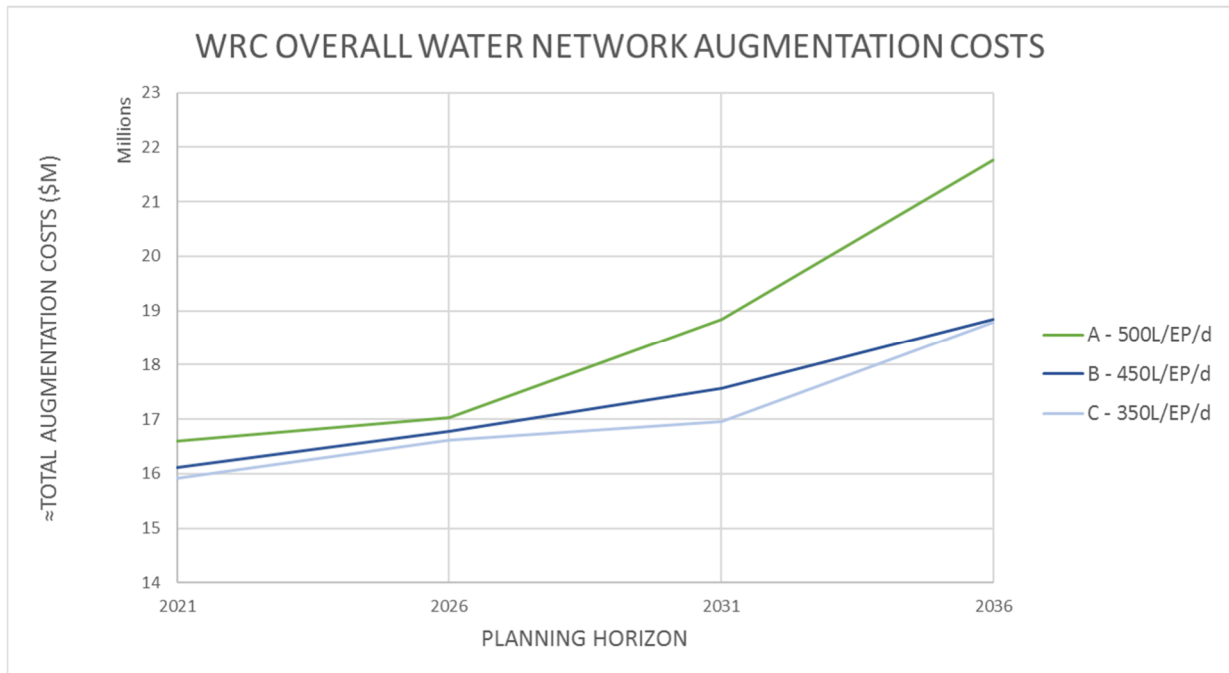
This report, prepared by Arcadis, with input from Water and Waste, for WRC, seeks to investigate demand management scenarios and apply them to WRC's water supply network model, developing infrastructure augmentations and resulting costs implications for each. From these outcomes, WRC should have a clear understanding of the potential impact for applying differing water demand management approaches to their water supply networks and the impact on WRC's financial future.

The assessment has identified augmentations that may be required to the WRC water supply network for varying demand profiles, demonstrating the significant impact design demand has on timing of network performance, infrastructure development and capital expenditure.

The study indicates that by implementing demand reduction options, some key/large infrastructure augmentations required to maintain desired services of standard to the network may be delayed to later design horizons or even omitted depending on the extent of the demand reduction.

When considering the substantial augmentation requirements, originally outlined in the 2016 WRC water strategy report, it is recommended that Council further investigate the opportunity to establish a water efficiency strategy which would allow for Council to delay a large number of augmentations to the network to later planning horizons. Depending on the adoption rate by the WRC community of the requested water demand targets, it may be possible to gradually decrease daily demand per person over a number of years, potentially achieving lower than 350L/EP/d.

Augmentation requirements are observed to increase in direct proportion to the assigned demand per EP, with the following graph illustrating the overall water network augmentation costs per assessment option.



Assessment of the lower demand options demonstrates that the most notable augmentations required to the network may be implemented during later horizons or even omitted depending on the extent of the reduction, with progressively decreasing network augmentation requirements as per person demand decreases in the assessed option B (450L/EP/d) and option C (350L/EP/d). This will allow for a lower long term water price.

WRC should note that there is a potential saving of up to ≈\$1M of pipework infrastructure CAPEX for the 2021 horizon if water consumption per EP is dropped by 150L/EP/D from 500 down to 350 L/EP/Day. This is a clear indication of the effectiveness of water demand management and the direct impact, with increasing savings in augmentation requirements and CAPEX in future planning horizons.

2 INTRODUCTION

On behalf of Whitsunday Regional Council (WRC), Whitsunday Water and Waste have commissioned Arcadis to carry out an update to the Local Government Infrastructure Plan for water and sewer reticulation networks within the Council's regions. This includes application of the estimated population growth as per the 2018 Norling report across the required demand horizons through to 2036.

In addition, Arcadis have been asked to build on the previously completed Water Supply Strategy completed in 2016 which sought focus on the potential of capital and operational expenditure reduction via a demand (reduction) management program, if WRC elected to pursue a policy change.

Different policy settings can encourage water efficiency, through demand management, and defer these projects, improving Whitsunday Water's and WRC's financial position. Other policy settings can encourage less efficient water consumption behaviours and may lead to larger capital works programs, which would necessitate additional funding.

Key outcomes of this report will show how the implementation of a water demand management strategy (water use reduction) will allow for the delay in capital expenditure around augmentation of infrastructure and the requirement to source additional raw water supplies.

Therefore, the dual purpose of this assessment is to:

- Provide high level costed options regarding infrastructure upgrade requirements over the next 20 years; and
- To guide WRC on further refinement of a general policy direction on water usage and rates and usage, from which can be analysed further and defined with additional clarity as required.

2.1 BACKGROUND

WRC relies on allocations from the Peter Faust Dam for our raw water, for our communities of Bowen, Cannonvale / Airlie and Proserpine. The WRWSSA Report undertaken by Council and the Queensland Department of Energy and Water Supply in 2016 identified a mid-term limit to WRC's water security (circa 2036 at 2013-16 per person water consumption).

Further studies into the groundwater reliability from the Don River alluvium have demonstrated that whilst important, this groundwater resource does not have urban reliability characteristics. This further frames WRC's water security perspective as being a finite resource that will need to be carefully managed in the short to medium term.

2.2 PREVIOUS WORK

2.2.1 Regional Water Strategy 2016

A regional water strategy for the Whitsunday Regional Council was developed by Arcadis and adopted by Council in April 2016. This assessed all stages of the Water supply from source to supply including:

- current and future water supply demands,
- available water supply sources , existing, future and alternative
- trunk lines and Water treatment plant limitations

the report indicated water source and treatment availability in the short term, but medium to long term shortages based on current water demand levels. In addition to exploring local and regional new raw water sources, the strategy investigated the potential for reducing water usage within the WRC water supply area through a water demand management program. Key outcomes of the strategy included:

- Water consumption at the current rate will result in the requirements of a new raw water source by 2036, and development of recycled water system by 2030.
- Reduced water consumption over the next 6 years negates the need for a new water source to well beyond 2036.

Refer to document number 0009-AA008791-AAR-04 North Whitsunday Water Strategy for further information.

2.2.2 2014 LGIP

Previous relevant studies undertaken by Arcadis to assess WRC network performance and augmentation requirements include the 2014 LGIP study, which sought to assess sewer network augmentation requirements and water network augmentation requirements under the baseline 500L/EP/d demand. This 2014 Network Model Update Report built upon the network models illustrating upgrade requirements based on 2014 population predictions and DSS, that were used as the base for this updated 2020 LGIP assessment. Refer to document number '**0001-AA006631-AAR-02**' for further details on the 2014 Network Model Update.

2.2.3 2017 LGIP

A subsequent study to the 2014 LGIP study, was undertaken focusing on the Whitsundays potable water network, involving the introduction of the future 12.5ML Cannon Valley reservoir, allowing for gravity service to the entire Whitsundays water catchment based on its central location and elevation. This 2017 Whitsundays water model and LGIP update also assessed the sensitivity of demand management beyond the baseline 500L/EP/d allocation to determine impacts on augmentation requirements, with all relevant outputs and conclusions contained in the Whitsundays Regional Council Cannonvale Water Network Modelling report '**0008-AA009827-AAR-02**' to be referred to for further information regarding this 2017 study.

2.3 LIMITATIONS OF REPORT

The outcomes of this Report are based on the base WRC hydraulic models adjusted by Arcadis from information provided by Whitsunday Regional Council. The information is at a master planning level. As such, the outputs (including opinion of costs) are at the feasibility stage and will require further information from preliminary through to detailed designs to refine final construction details.

The aim of this report is to inform key stakeholders of the required infrastructure augmentation to the system over the next 20 years and also the impact water demand management can have on infrastructure program costs and eventually on revenue. Arcadis take no responsibility for the information provided within this report if used by a third party and or/outside of its intended use.

3 PURPOSE OF REPORT

The purpose of this technical report is to undertake an assessment of the performance of WRC's water and sewer network models under the updated Norling 2018 population projection estimates and determine an updated 2020 LGIP set to allow all WRC networks to perform at the required Desired Standards of Service in accordance with the current WRC Development Manual.

The impact of demand management on potable water networks has also been included to assess sensitivity of demand assumptions on WRC LGIP requirements. By assessing different water use demands (above and below the current modelling) for future planning horizons up to 2036, WRC's key stakeholders will be able to understand the potential for delaying (or bringing forward) capital expenditure identified in the LGIP.

The LGIP update also serves as a review tool against previous LGIP recommendations, identifying augmentations carried out and those still outstanding. It can also reclassify and re-prioritise outstanding programmed LGIP upgrades based on new population growth estimates.

4 LGIP assessment Approach

To carry out the LGIP assessment and a review of the impact of water demand management the following steps were taken.

- Basis of assessment –
 - Population update - This included a review of all inputs including, population estimates, spatial distribution of new populations,
 - Design standard review (DSS) and scenario development
 - Model review and update including update of all physical inputs (pipe ID's, material type, invert levels, location, size)
 - Development of an opinion of cost approach
- Initial model outputs for review – assessment was based the Whitsunday Regional Council Development Manual
- Review and discussion with WRC staff on LGIP outputs via a workshop where prioritisation ranking of proposed water infrastructure was developed via a Multi-criteria Analysis.
- Update of model and LGIP updates based on workshop MCA

The above is covered in the following sections of the report.

5 BASIS OF ASSESSMENT

5.1 UPDATED POPULATION PROJECTION

All WRC water and sewer hydraulic network model Scenarios have been updated to reflect the latest Norling 2018 population estimate study. An extract of the total population estimate difference between the 2013 & 2018 revisions of the Norling population estimates are illustrated in Figure 5-1 below.

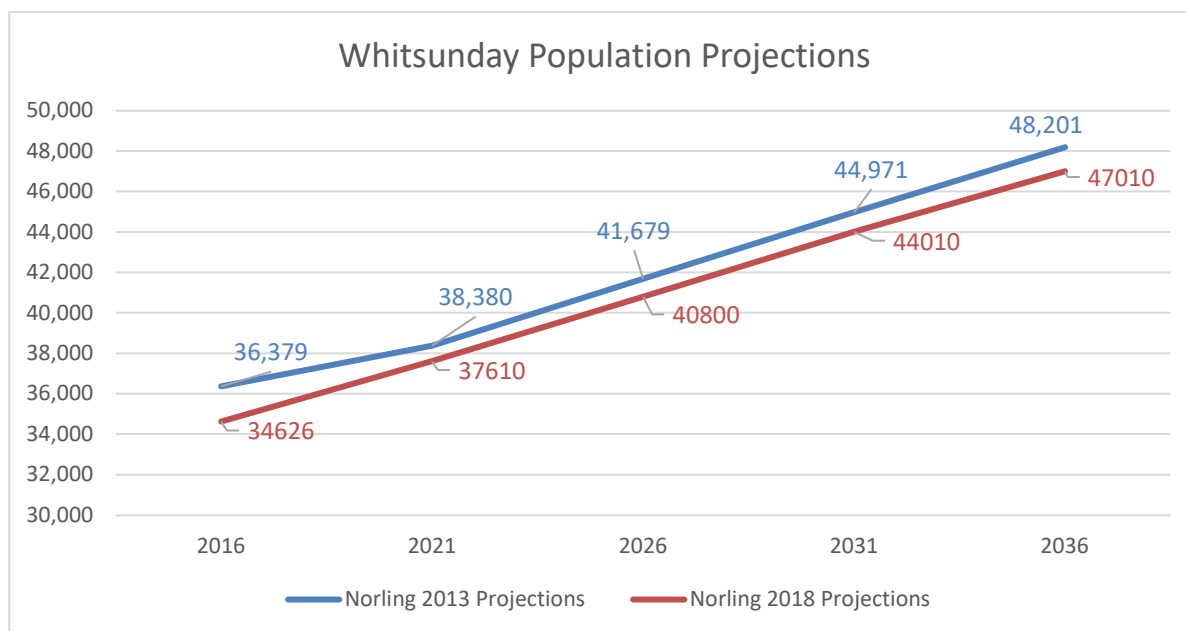


Figure 5-1 Norling 2013 vs 2018 Review Extract – WRC Population Projections

The statistics for the relevant catchments within WRC are summarised in Table 5-1 below.

Table 5-1 Norling 2018 Population Projection Summary

| CATCHMENT | 2016 | 2021 | 2026 | 2031 | 2036 |
|-----------------------------------|-------|-------|-------|-------|-------|
| Bowen | 9290 | 9330 | 10320 | 10920 | 11250 |
| Collinsville | 3496 | 3690 | 3710 | 3830 | 3840 |
| Airlie-Whitsundays | 13011 | 15090 | 17490 | 19740 | 22180 |
| Proserpine | 8829 | 9040 | 9280 | 9520 | 9720 |
| Whitsundays Region (TOTAL) | 34626 | 37610 | 40800 | 44010 | 47010 |

As shown population estimates have dropped overall, with over a 1000 EP's less estimated at the 2036 planning horizon. It should be noted that this is slightly misleading, as all regions had reduced from the 2014 estimates except Proserpine, which has risen significantly.

5.2 DESIGN SCENARIOS

5.2.1 WATER DESIGN SCENARIOS

To assess the impact of water demand management, adjustment of the water demand per EP was carried out. The demand management can be adjusted downwards via managed restriction or water use awareness. The following modelling scenarios were developed:

- Option – A – Base/Business as usual – 500 L/EP/d. Reflects current planning and water use, and what was adopted in all LGIP planning currently completed – this is considered the base case of which to assess impacts water management against.
- Option B – Modest 10% water use reduction of Base option A – 450L/EP/d; and
- Option C – Moderate 30% water use reduction of Base option A – 350L/EP/d.

5.2.2 WATER DEMAND SCENARIOS

To analyse the impact of reducing water demands, several new demand levels (as above) for each planning horizon were developed. The levels reflect a decrease in the estimate of water use per Equivalent Person, applied uniformly across all model demands and modified to the following:

- Option A – 500 L/EP/d
- Option B – 450 L/ EP/ d (modest efficiency 10% reduction); and
- Option C – 350L/EP/d (moderate efficiency, 30% reduction).

The resulting Average Day (AD) demands and modelled scenarios are summarised in Table 5-2 below.

Table 5-2 Average Day Demand

| Catchment | 2021 AD (ML/d) | | | 2026 AD (ML/d) | | | 2031 AD (ML/d) | | | 2036 AD (ML/d) | | |
|---------------------|----------------|-------|------|----------------|-------|------|----------------|-------|-------|----------------|-------|-------|
| | A | B | C | A | B | C | A | B | C | A | B | C |
| WHITSUNDAYS | 12.07 | 10.86 | 8.45 | 13.39 | 12.05 | 9.37 | 14.63 | 13.17 | 10.24 | 15.95 | 14.36 | 11.17 |
| BOWEN | 4.67 | 4.20 | 3.27 | 5.16 | 4.64 | 3.61 | 5.46 | 4.91 | 3.82 | 5.63 | 5.06 | 3.94 |
| COLLINSVILLE | 1.85 | 1.66 | 1.29 | 1.86 | 1.67 | 1.30 | 1.92 | 1.72 | 1.34 | 1.92 | 1.73 | 1.34 |

5.3 DESIGN STANDARDS

The assessment has been completed in accordance with the following standards:

- Whitsunday Regional Council Development Manual:
 - D5 Water Reticulation
 - D6 Sewer Reticulation
 - S5 Water Reticulation
 - S6 Sewer Reticulation

- Water Services Association:
 - WSA 03 Water Supply Code
 - WSA 02 Gravity Sewerage Code
 - WSA 04 Sewerage Pumping Station Code
 - WSA 07 Pressure Sewerage Code

The Desired Standards of Service (DSS) for water supply adopted from the abovementioned design standards are shown in Table 5-3 below.

Table 5-3 Water Supply Adopted Desired Standards of Service (DSS)

| Criteria | Standard |
|--|---|
| Minimum pressure | 22m |
| Maximum pressure | 80m |
| Minimum residual pressure under fire flow conditions | 12m at hydrant |
| Reservoir storage | 3 (PD – MDMM) + (Greater of Emergency Storage/Firefighting Storage) |
| Elevated reservoir storage | 6 (PH – MDMM/12) + firefighting reserve |
| Maximum velocity | 2.5m/s generally and up to 4m/s under fire flow conditions |
| Treated water pumps feeding ground level reservoir | MDMM over 20 hours |
| Treated water pumps feeding an elevated reservoir | 6PH – reservoir operating volume |

5.4 OPINION OF COST ANALYSIS BASIS

All opinion of costs developed were based on rates from the 2013 Unit Rates Report as prepared by Cardo for the Gold Coast City Council. The Unit Rates Report has been in circulation since 2002 and is revised on a regular basis by various Engineering Consultants. (The current edition of the document has been prepared by Cardno in 2013). Its costings are seen as relevant for preparing comparisons on water and waste capex works at the LGIP level. Several unit rates were cross checked against recent construction projects to verify appropriateness of cost.

Capex costs include the following:

- 40% contingency in line with high level cost estimate;
- 10% regional cost increase based on the Rawlingsons Australian Construction Handbook 2014 building indices;
- All costs are in a present value basis and as such inflation is not accounted for; and

- Upgrade to pump stations, pumps, wet well size, as required per option. Note it was assumed that all pump station upgrades were limited to pumps, pipework and electrical upgrades, with no change to pump station building sizes;

Exclusions:

- De-commissioning of any pump stations;
- Pump station building upgrades.

The costs developed have been estimated infrastructural capital and operational costs only with no allowance for site survey, design or management, and should not be used for any purpose other than feasibility comparison given in this report.

5.5 WATER NETWORK HYDRAULIC MODEL BASE

The following Base networks were adopted for the 2020 LGIP optimisation. Further amendments to the hydraulic model were carried out and is discussed in section 6 of this report.

Whitsundays Network:

The model used for this hydraulic assessment is as per the revised LGIP assessment undertaken in late 2017, with the same assumptions and exclusions – refer to Sections 3 & 4 of Report 0007-AA009827-AAM-02 ‘Cannonvale Water Supply Trunk Network Isolation’ dated 17/11/2017.

Bowen and Collinsville Networks:

The models used for this hydraulic assessment is as per the 2014 network model update and associated ‘2014 Network Model Update Report’ dated 1/08/2014. Refer to this report for information pertaining to the initial set up of this model, superseded by all amendments undertaken as part of this study and mentioned within this report.

6 WATER SUPPLY NETWORK MODEL AMENDMENTS

6.1 DEMAND GENERATION AMENDMENTS TO ALL WATER MODELS

A key modification to the definition of the hourly diurnal pattern was applied across all WRC water network models prior to undertaking of the 2020 LGIP revision. This was aimed at standardising the assessment of network performance based on the current WRC Development Manual criteria.

Demand generation criteria as defined by the WRC Development Manual are as follows:

- Average Daily Consumption (AD) = 500L/EP/d
- Mean Day Max Month (MDMM) = 1.50 x AD
- Peak Day (PD) = 2.25 x AD
- Peak Hour (PH) = 1/12 x PD

In the above, peak hour consists of a 2x instantaneous increase in PD flow rate (L/s) over the duration of one hour representing the peak hour of the peak day. Therefore, the peak hour multiplier of the diurnal patterns utilised to represent residential, commercial & industrial flows has been assigned as 2 as the WRC development manual does not differentiate different peak factors between differing development types. This is done whilst maintaining the overall demand throughout the day to still match total peak day demand volumes.

Past 2014 revisions of the network models utilised different diurnal patterns for residential, commercial and industrial loadings hitting the following peak factors:

- Residential = 2.1x
- Commercial = 1.5x
- Industrial = 1.5x

An example comparison of the previous vs updated diurnal pattern for industrial developments is included in Figure 6-1 below.

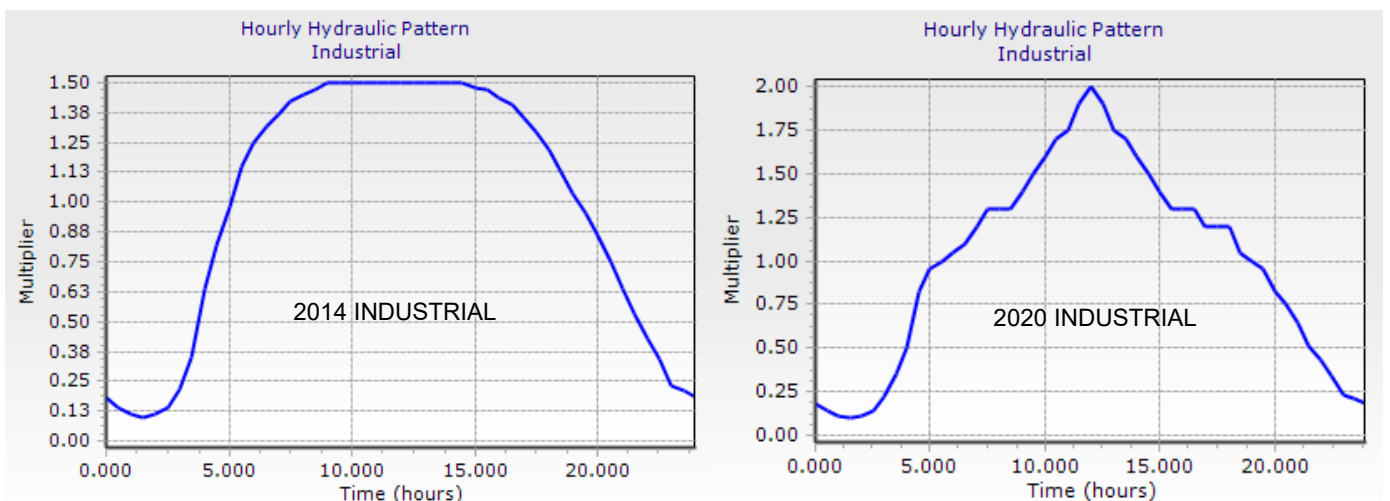


Figure 6-1 2014 Industrial Diurnal Pattern vs Current 2020 Updated Industrial Diurnal Pattern

6.2 WHITSUNDAYS WATER SUPPLY MODEL DEVELOPMENT

The hydraulic modelling undertaken in this assessment has been based on the base model Water Cad model established following the Arcadis 2017 Whitsundays Water Supply Trunk Network Isolation Report and associated updates to the Cannonvale (Whitsundays) potable water network model and revised 2017 LGIP for this network. This base model previously identified a list of LGIP augmentations for implementation into the network for adequate operation of the network under multiple assessment horizons, namely 2014, 2016, 2021, 2026, 2031 and 2036.

Following the introduction of the new 12ML Cannon Valley reservoir and rework of network operation, a new asset has since been included in this study. The bulk water pipeline constructed from the Proserpine WTP to downstream of Booster 2, consisting of an 559OD MSCL pipeline (523mm ID) ≈9300m in length, has been included in this revision of the Whitsundays water model which greatly improves transfer capacity of Proserpine WTP to the new Cannon Valley reservoir.

A number of other updates were subsequently identified to ensure the potable water supply model remains current and based on the latest information available which may not have been available at the time of the previous update iteration, sourced via communication with WRC and further described in the sections below.

6.2.1 TRUNK-RETICULATION CONNECTIONS

Following the update of the water supply model to reflect the most current information made available to Arcadis and with the inclusion of the new 12ML Cannon Valley reservoir, the trunk water supply line originating from Booster 2 pump station south of Whitsunday Quarry carrying water through Cannonvale, Airlie Beach, Jubilee Pocket and Shute Harbour was investigated. This investigation primarily focused on the multiple interconnections between this trunk water supply line and the adjacent reticulation network. The performance of the network following decommissioning of these interconnections was assessed in the 2021 planning horizon to determine the impact to the network's performance. A summary of the interconnecting pipes deactivated in this assessment is included in Table 6-1 below.

Table 6-1 Trunk / Reticulation Interconnections

| Pipe Label | Diameter | Material | Start Node | Stop Node | Status |
|------------|----------|----------|------------|-----------|-------------|
| CanW371 | 225 | DICL | BS56 | BS158A | Deactivated |
| CanW333 | 225 | DICL | BS166 | BS160 | Deactivated |
| CanW23 | 225 | AC | BS184 | BS161 | Deactivated |
| CanW419 | 225 | AC | BS182 | BS183 | Deactivated |
| CanW254 | 150 | uPVC | BS171 | CA204 | Deactivated |
| CANW399 | 100 | uPVC | BS262 | CA281 | Deactivated |
| CANW354 | 225 | DICL | BS209 | BS210 | Deactivated |
| AirW138 | 225 | DICL | BS212 | BS211 | Deactivated |
| AirW122* | 100 | DICL | BS227 | AB76 | Active* |
| AirW45 | 225 | DICL | BS228 | BS97 | Deactivated |

| Pipe Label | Diameter | Material | Start Node | Stop Node | Status |
|-------------|----------|----------|------------|-----------|-------------|
| AirW75 (on) | 225 | uPVC | BS176 | AB38 | Active |
| BS313_AB122 | 200 | CIOD | AB122 | BS314 | Deactivated |
| BS99_BS98 | 200 | DICL | BS98 | BS99 | Deactivated |
| BS229_AB130 | 200 | PVC-M | AB130 | BS229 | Deactivated |
| P-176 | 250 | DICL | BS254 | BS253 | Active |
| JubW131 | 250 | DICL | BS253 | BS254 | Active |
| JubW129 | 200 | MSCL | BS252 | BS251 | Deactivated |
| JUBW127 | 200 | DICL | BS250 | BS249 | Active |

**Reactivated since 2017 LGIP/Trunk Isolation Assessment*

6.2.2 BULK WATER PIPELINE

Following the introduction of the new 12ML Cannon Valley reservoir and rework of network operation, a new asset has since been included in this study. The bulk water pipeline constructed from the Proserpine WTP to downstream of Booster 2, consisting of an 559OD MSCL pipeline (523mm ID) ≈9300m in length, has been included in this revision of the Whitsundays water model which greatly improves transfer capacity of Proserpine WTP to the new Cannon Valley reservoir. This bulk water pipeline is supplied via a three parallel pump setup, comprised of 3x Flowserve Hydrotitan 150x125-315 pumps @ 301mm impellers.

6.2.3 MODEL DEMAND GENERATION – PEAK DAY ASSESSMENTS

A notable change has been applied to all peak day scenarios of the Whitsunday model, involving the change of the base demand applied to all junctions from the previously specified AD demand to the required PD demand as per the WRC development manual criteria for assessment of standard flow performance and reservoir performance. This change results in 2.25x the water demand applied to the entire Whitsundays model catchment, directly impacting the previously assumed network performance estimates. All other WRC water network models were set to peak day demands under PD assessment scenarios therefore this modification only applies to the Whitsundays water network model.

6.2.4 RESERVOIRS

The addition of the 12ML Cannon Valley reservoir into the model, has resulted in substantial change to the network. Physical and operational parameters were then reviewed to optimise operation of the systems reservoirs.

A breakdown of key reservoirs including amendments to operational characteristics is included below. The controls listed below can be subjected to minor adjustments to vary the operating levels of the listed reservoirs to operate between the desired levels based on operational preferences.

6.2.4.1 NEW 12ML RESERVOIR – CANNON VALLEY SITE

The proposed 12ML Cannon Valley reservoir has been modelled to the following specifications:

Physical Parameters

- Base elevation = RL89.0m
- Maximum elevation = RL102.0m
- Diameter = 39.41m

Operation

- Reservoir controlled by operation of Booster 2 pump station,
- If water level at Cannon Valley reservoir < RL91.0m, then booster 2 is on
- > RL100.00m, then booster 2 is off
- Pressure Reducing Valve (PRV) proposed prior to connection into reticulation network – set to decrease HGL to 70m prior to connection into reticulation network between the 2021 to 2031 horizons, increasing to 75m in the 2036 horizon.

A Pressure Reducing Valve (PRV) is proposed for implementation on the reservoirs reticulation supply line, due to the available static head at it's location. This then reduce pressures in the low-lying areas of the network being supplied via this reservoir and ensures the Coyne Rd Low Level reservoir is utilised as the supply reservoir for the majority of the residential areas in its vicinity by overcoming the head supplied via the proposed Cannon Valley reservoir.

6.2.4.2 EXISTING 12ML CANNONVALE RESERVOIR

The existing 12ML Cannonvale reservoir previously controlled via Booster 2 pump station has been modified for control via flow control valve to regulate trunk supply to this reservoir dependant on water level within.

Operation

- Reservoir controlled by operation of TCV-6 flow control valve,
- If water level at Cannon Valley reservoir < RL78.20m, then TCV-6 is open
- > RL79.70m, then TCV-6 is closed

The trunk supply network from the existing 12ML Cannonvale reservoir to the Airlie Beach, Jubilee Pocket and Shute Harbour area to the east has been disconnected from the main trunk feed originating from Booster 2 and the new 12ML Cannon Valley reservoir. This is done to ensure the areas to the east listed above are supplied via the existing Cannonvale reservoir to ensure maximum pressures in these areas remain below the DSS maximum of 80m pressure.

6.2.4.3 EXISTING COYNE ROAD LOW & HIGH LEVEL RESERVOIRS

The existing Coyne Road Low Level (LL) reservoir and Coyne Road High Level (HL) reservoir have been modified to meet the operating pressure DSS in the adjacent elevated service area and eastern residential areas. Presently, the only feed from Coyne Road LL supplies pump station WCPS03 pumping to Coyne Road HL, with the outlet from the pump station also connected to the adjacent reticulation network supplying lower lying areas. Due to the lack of a direct gravity feed, Coyne Road LL is substantially under-utilised. This is exacerbated by the connection between low lying areas and the elevated area adjacent to the Coyne Road HL leading to too large of a demand being applied on Coyne Road HL and its subsequent emptying following isolation of the trunk and supply networks.

6.2.4.3.1 COYNE ROAD LOW LEVEL

This reservoir is filled via gravity trunk supply from the new Cannon Valley reservoir.

Operation

- Reservoir controlled by operation of TCV-2 flow control valve,
- If water level at Coyne Rd LL reservoir < RL71.40m, then TCV-2 is open
- > RL73.20m, then TCV-2 is closed

Under-utilisation of this reservoir has been flagged as an issue with the existing system configuration. A direct connection to the gravity network adjacent to the tank allows this reservoir's use to be optimised and service the lower lying residential areas in the vicinity of the LL reservoir. This involves the implementation of a gravity supply pipeline from the LL reservoir to the existing network at the intersection of Shute Harbour Rd & Tropic Rd N, along with further smaller augmentations to strengthen the water supply to the surrounding areas. These augmentations detailed in Appendix A allow for the adequate utilisation of the Coyne Rd LL reservoir.

Under higher loading circumstances (beyond Average Day), the demand drawn by the main reservoirs in the northern network cause the HGL within the trunk supply line to drop too low to supply the Coyne Rd High Level reservoir via gravity. To mitigate this, it is proposed to utilise the Coyne Rd HL Pump Station during such events. Doing this will ensure a steady pumped supply to Coyne Rd HL reservoir during lower HGL events, whilst supplying the Coyne Rd HL reservoir via gravity the rest of the time.

6.2.4.3.2 COYNE ROAD HIGH LEVEL

Following the implementation of the 12ML Cannon Valley reservoir, the head within the trunk supply line is high enough to fill the Coyne Road HL tank via gravity under low to average demand conditions. However, the head within the trunk supply line at the connection point to Coyne Road HL tank is observed to fluctuate at various instances due to reservoir filling patterns.

It is proposed to connect the inlet of the Coyne Rd HL pump station direct from the trunk supply line to minimise any impacts to the capacity of the Coyne Rd LL reservoir and utilise the higher pressures available within the trunk supply line to reduce pump capacity requirements.

As such, it is proposed to maintain operation of Pump Station WCPS03 to pump water to the Coyne Rd HL tank in case the HGL at the connection point is insufficient, however primary operation of the Coyne Rd HL tank will attempt to feed via gravity from the trunk supply pipeline under low to average demand conditions and available head allows for gravity fill, with the option to pump direct from the trunk supply in cases where this is not possible.

There may be potential to alter the magnitude of head fluctuation at the Coyne Rd HL tank connection point via a more detailed assessment of the water supply network and establishment of restrictions to reservoir filling rates and staggering of reservoir filling times – whilst maintaining DSS.

Operation

Reservoir controlled by operation of TCV-6 flow control valve,

- If water level at Cannon Valley reservoir is: < RL94.45m, then TCV-6 is open
- > RL95.5m, then TCV-6 is closed

The residential area located south of Coyne Road LL in the vicinity of Panorama Ct generally lies at elevations RL45m+ reaching up to RL53.7m. This is beyond the service range of Coyne Rd LL to achieve adequate minimum pressure. As such, augmentations are proposed to service this elevated area via the Coyne Rd HL reservoir and isolating this area from the network supplied via Coyne Rd LL and illustrated in the augmentation summary maps provided in Appendix A.

The proposed isolation valves have been implemented in the following pipes:

- CanW143
- CanW137
- P-872
- P-845
- P-877

Implementation of the augmentations and operational changes discussed above allows for the Coyne Rd LL and HL reservoirs to be better utilised and replaces the previously identified LGIP upgrade of the Coyne Rd HL tank following clear definition of the elevated zone serviced via this tank.

6.2.5 PUMP STATIONS

The model used for the hydraulic assessment is as per the revised LGIP assessment undertaken in late 2017, with the same pump amendments and configuration – refer to Section 4.1 of Report 0007-AA009827-AAM-02 ‘Cannonvale Water Supply Trunk Network Isolation’ dated 17/11/2017.

6.3 BOWEN WATER SUPPLY MODEL DEVELOPMENT

6.3.1 CATCHMENT ISOLATIONS

The Bowen town centre area surrounding the reservoir site has been managed via the use of isolation valves to separate certain catchments to allow for more control of water allocation over the system. The areas south of Bowen have been split into separate south-east and south-west catchments, with proposed isolation valves implemented in the following pipes. These are illustrated as the orange separation in Figure 6-2 below.

- P-2100
- P-1646
- BA114A730
- B71A813
- P-2094
- BA113A623
- P-1640
- P-1638
- B125132

An additional two isolation valves have been included in the northern area within the Bowen south-west catchment to further separate this south-western network from northern demands, with proposed isolation valves implemented in the following pipes. These are illustrated as the green separation in Figure 6-2 below.

- BA282A284
- BA280A281

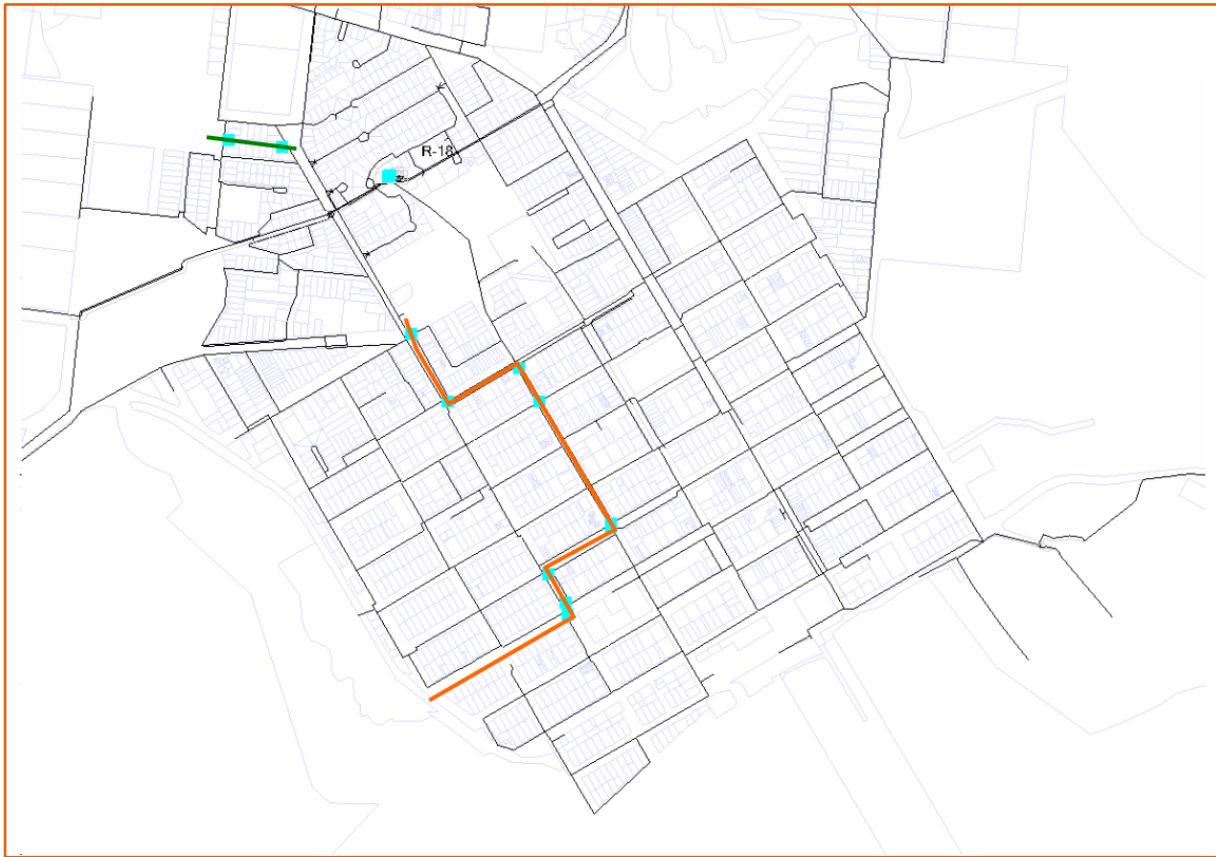


Figure 6-2 Bowen Town Centre Isolation Map

6.4 COLLINSVILLE WATER SUPPLY MODEL DEVELOPMENT

6.4.1 CATCHMENT ISOLATIONS

Similar to the Bowen network, an isolation valve set is proposed in the Collinsville water network east of the Peter Delemothe Rd high level tanks, aimed at separation of this eastern high level reticulation area for service via the Peter Delemothe Rd tanks and isolating the southern portion of the catchment for service via the Miller St tanks. Isolation valves are proposed for implementation in the following pipes and illustrated in Figure 6-3 below.

- P-45
- 20858
- 20909

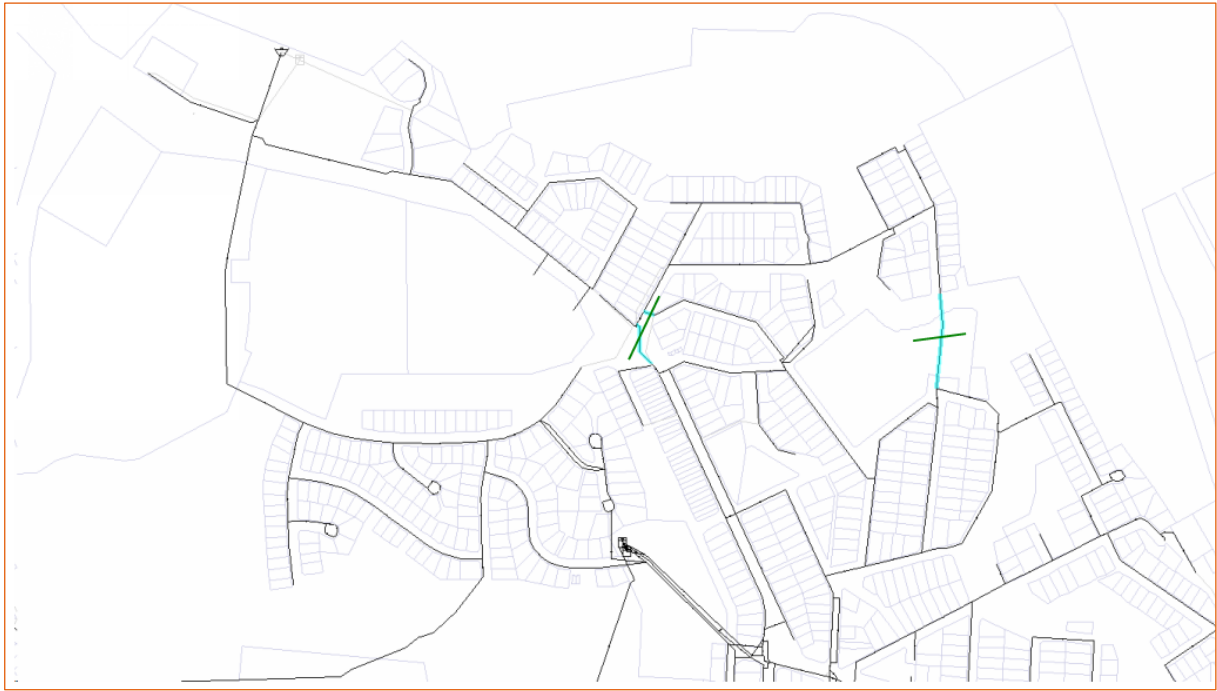


Figure 6-3 Collinsville Isolation Map

7 SEWER MODEL APPROACH AND AMENDMENTS

7.1 SEWER NETWORK ASSESSMENT DSS

The below table specifies the key DSS adopted for the assessment of the WRC sewer network models, in accordance with the WRC Development Manual D6.

Table 7-3 Sewerage Adopted Desired Standards of Service (DSS)

| Criteria | Standard |
|------------------------------------|------------------------------------|
| New gravity pipe max depth of flow | 75% |
| Existing gravity system | No overflows under PWWF conditions |

7.2 SEWER MODEL DEVELOPMENT

Previous work included sewer network models for Cannonvale, Proserpine, Bowen and Collinsville which were developed in 2014. It was necessary to update these models using current GIS data.

7.2.1 DATA INPUTS

The following sources of information were used to establish the InfoSWMM models for analysis of the sewer network

- 2014 LGIP H2OMap models
- 2019 GIS man hole and pipe data (provided by WRC)
- 2018 Norling Projections

Data from the 2014 H2OMap models were converted into InfoSWMM as the basis for the model update. A visual comparison was made between the previous model and the pipes included in the 2019 GIS data in order to locate where new pipes had been installed. Data associated with these new pipes and manholes was then directly added to the working model.

- Average Dry Weather Flow (ADWF) = 270 L/EP/d;
- Peak Dry Weather Flow (PDWF) = $C2 \times ADWF$;
 - $C2 = 4.7 \times EP^{-0.105}$
- Peak Wet Weather Flow (PWWF) = $5 \times ADWF$;
- Manning's roughness coefficient is taken as 0.013;
- Depth of flow @ PWWF – Existing system = Up to 1.0m below MH cover level and no spillage through overflow structures (as confirmed by Council Officers);
- Emergency Storage: 4 Hours @ ADWF;
- Single Pump Capacity (Duty & Assist): $Q = C1 \times ADWF$;
 - where $C1 = 15 \times EP^{-0.1587}$ but must range between 3.5 – 5
- Operational Storage: Ops Storage = $0.9 \times Q/N$ (See N below);
 - $N = 12$ (Motors <100Kw);-

- N = 8 (Motors 100-200Kw)
- N = 5 (Motors >200Kw)
- Ideal Pumps have been used to size pipe infrastructure;
- New pipe infrastructure depth of flow during PWWF does not exceed 75%.

7.2.2 PIPELINES

7.2.2.1 Pipe Diameters

Previous work on the Whitsunday sewer models had estimated the inner pipe diameter using the pipe DNs. In some cases, dependent on the material of the pipe, this was a significant over estimation. In the updated network model, it was important to enter the correct inner diameter for critical pipes. Using pipe material data from GIS, the accurate inner diameters were entered for all pipes above DN150.

Inspecting the DNs of the previous model against the DNs provided in the 2019 GIS data also identified where the incorrect pipe size was entered. Where differences were found, the new data was used in the working model.

Figure 7-1 compares a section of the Cannonvale sewer network modelled in 2014 and 2019. It illustrates some of the discrepancies in the pipe size.

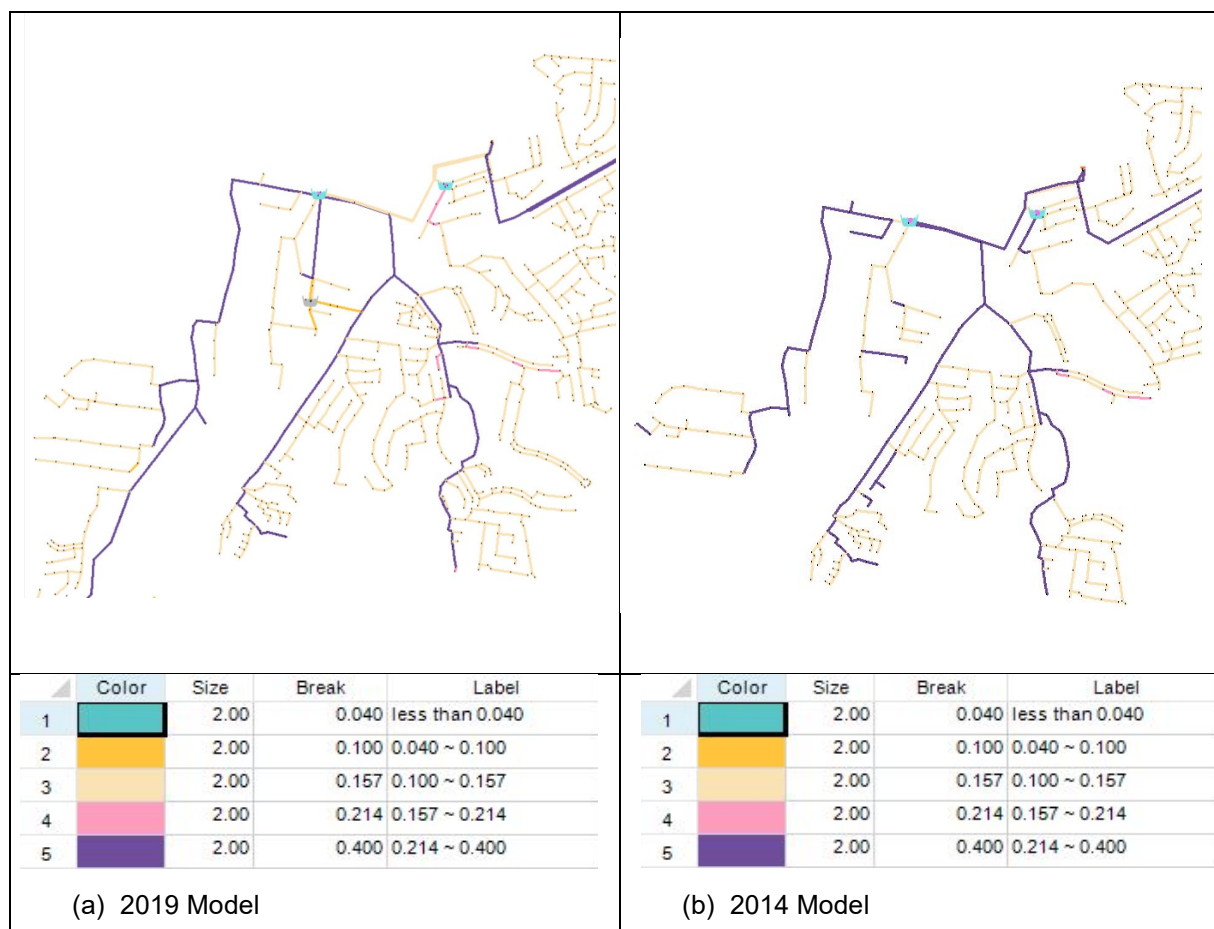


Figure 7-1 Pipe sized used in (a) 2019 model, (b) 2014 Model of the Cannonvale sewer network

7.2.2.2 Pipe Roughness

As per WRC Combined Development Manual V1.3 Table 6.3, all PVC and Poly pipes were assumed to have a Mannings roughness of 0.013. As all other pipe material types generally have a lower roughness value, for simplicity a Mannings roughness of 0.013 was used uniformly across all pipes.

7.2.2.3 Invert Levels

Invert level and pipe data was not available for all pipes and man-holes in the networks. Where information was not available, these areas of the network were de-activated and an equivalent sewer load entered at the nearest active point.

7.2.3 PUMP STATIONS

Pump station information including pump curves, start-up and shutoff heads, and wet well storage information was brought in from the H2OMap models.

A number of rising sewer mains identified in the GIS data did not have associated pump station information. For modelling purposes, an ideal pump station was entered into the model to service these rising mains.

7.2.4 SEWER LOADS

The following process was used to estimate sewer loads in each of the catchments for the planning horizons:

- Loads taken from appropriate horizon in the previous model
- Each individual load was scaled such that the total catchment load was equal to the 2018 Norling total. The same scaling factor was applied across the whole catchment.

7.2.5 MODEL OPTIMIZATION

As part of the model development, areas were augmented significantly to optimize the networks. These ultimately allowed for more efficient networks with less overall augmentations. These optimizations to the different models are described in detail below.

7.2.5.1 Cannonvale

A diversion from the CANN14 pump station was created to direct flow south directly into the rising main within Shute Harbour Road. The diversion allows for less flows towards the Broadwater drive gravity line which reduced significant pipe augmentations required if the full existing flows were maintained. An image of the optimization can be seen in sheet 2 of the Cannonvale Sewer Network Augmentation Layouts.

7.2.5.2 Bowen

A long extension of rising main was created along the south-western edge of the town in Norris and Dalrymple Streets directly to the 3-PUMPS pump station. The new rising main diverts significant flows from the north around the existing constricted gravity network alleviating flooding and backing up issues from the network. An image of the optimization can be seen in sheet 1 of the Bowen Sewer Network Augmentation Layouts.

7.2.5.3 Proserpine

A decommissioning of an existing pipe and extension of an existing rising main was created to direct flows directly to a rising main within Anzac Road. The diversion allowed large flows to be removed from the current gravity lines removing significant flooding and the need for large pipe augmentations in residential surrounds, while also allowing a smaller augmentation of the pump that it was directed to following the existing gravity route. In both scenarios it ultimately is directed to the same rising main toward the WWTP. An image of the optimization can be seen in sheet 2 of the Proserpine Sewer Network Augmentation Layouts.

7.3 SEWER MODEL DEMANDS

This report contains considerable focus on water demand reduction, which will impact potable water supply infrastructure augmentation, however it is noted that the majority of the decrease in water use through demand management will not carry over to reduction in flows to the sewer. Most of the flow reduction will be around water use external to the household, therefore the base demand scenario of Average Dry Weather Flow (ADWF) = 270 L/EP/d, is maintained with population increase over the demand horizons being considered as the variable to assess,

8 WRC WORKSHOP AND MCA

8.1 Initial Outputs

After application of the above hydraulic model changes to the WRC networks model, the scenarios were run, with a list of augmentations developed. This included, timing, location and cost of each augmentation/upgrade required. These initial outputs, shown in Appendix F & G were provided to Whitsundays Water and Waste design and operations staff for review, allowing for preliminary identification of areas requiring augmentations to be further refined based on knowledge of augmentation opportunities, preferred/alternate alignments, current augmentation plans and the likes.

8.2 Workshop

Based on initial feedback on [possible changes/variations to the proposed infrastructure a workshop in the ARCADIS offices was proposed to provide a more detailed analysis of each augmentation. The aim of the workshop included:

- Verify / ground truth requirement for augmentation based on site specific information such as asset life (i.e. old poor-quality asbestos pipe replacement to be bought forward)
- Rank and prioritisation of each augmentation based on MCA criteria given below:
 - Need to service
 - Operational (known bursts/construction fails)
 - Redundancy/management
 - Realistic growth/ability to service/climate adaption

Key outputs from the workshop are given in the following sections and are considered to be the final adjusted outputs from the 2020 LGIP assessment.

9 WATER SCENARIO OUTCOMES

With the updated models, the water demand scenarios A through to C were assessed against the planning horizons, 2021-2036. This illustrated a step change requirement of infrastructure over the next 15 years based on the water demand. This link and the importance of managing water usage is discussed in detail below, discussing how key infrastructure is impacted.

9.1 WHITSUNDAYS WATER NETWORK

9.1.1 BULK TREATED WATER SUPPLY IMPACT

A verification of the available bulk water supply against the predicted average day demands was undertaken to assess the availability of water external to the local Cannonvale potable water supply network. Table 9-1 & Figure 9-1 below demonstrates the requirement on the WTP drawn by the Cannonvale & Proserpine network under the assessed demand management scenarios.

Table 9-1 Bulk Treated Water Supply Summary

| Demand Scenario | MDMM (ML/d) 2021 | Available WTP Capacity (ML/d) |
|-----------------|------------------|-------------------------------|
| A – Base | 18.10 | 23.6 |
| B – 450L/EP/d | 16.29 | |
| C – 350L/EP/d | 12.67 | |

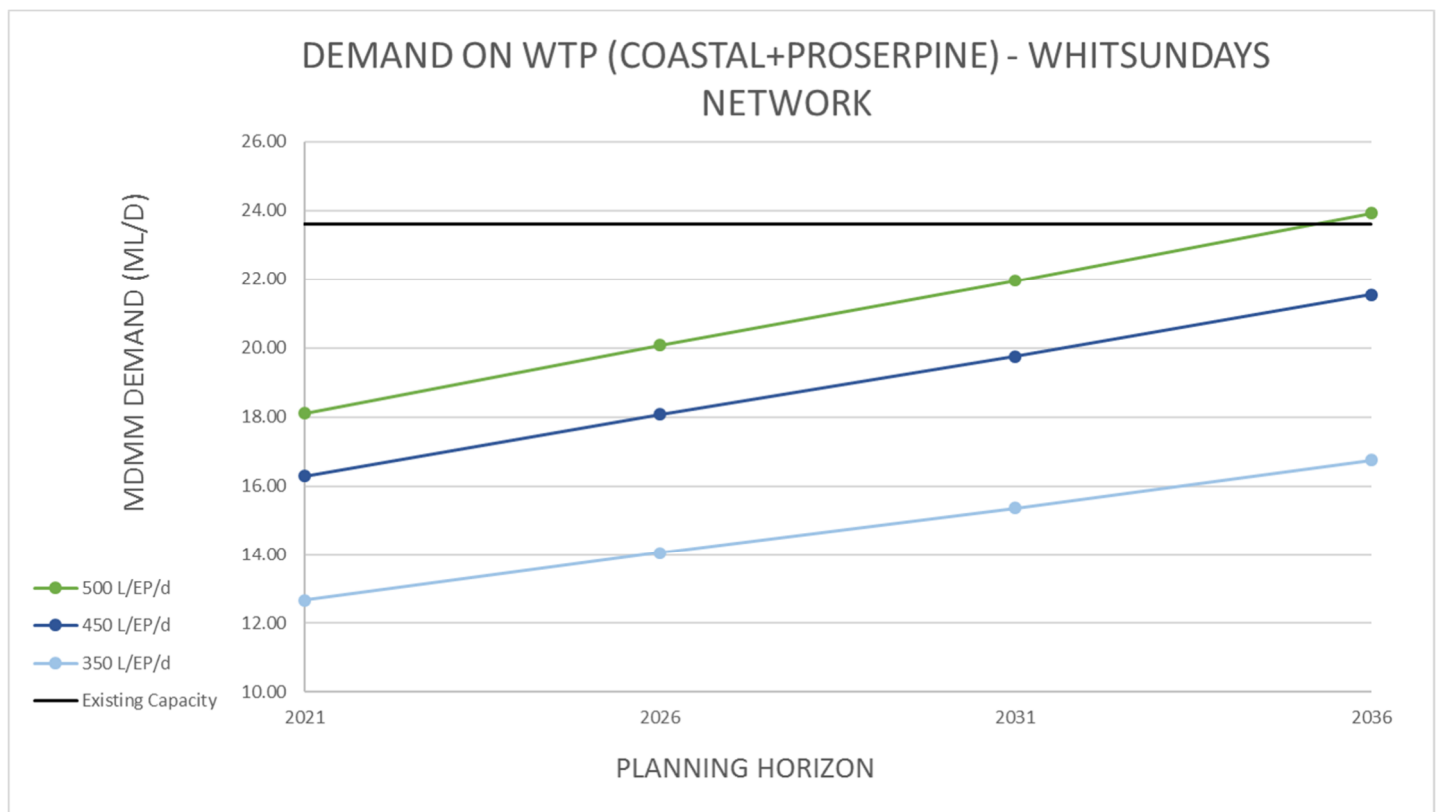


Figure 9-1 Whitsundays Water Network - WTP Demand Over Time

Based on the above, the Whitsundays network experiences a deficiency in existing WTP capacity in the 2036 base (500L/EP/d) scenario of 0.33ML/d which equates to a shortfall of 120.2ML/year, with demand first exceeding existing capacity in ≈2035. Should WRC remain at 500L/EP/d allocation/utilisation, an augmentation to the existing WTP capacity will be required at this time to make up for this shortfall. Based on the Urannah Dam Study (future water source), specifically Appendix 11 – Economics, users within the WRC network can expect a 2000ML/year water source costing \$2000/ML as the next planned increase to existing WTP capacity resulting in a drastic increase in required augmentation costs. Through the use of demand management, this augmentation may be delayed past the design 2036 horizon.

9.1.2 RESERVOIR IMPACTS

The service catchment of all reservoirs within the Whitsundays catchment has been isolated in the 2021 horizon and factored accordingly based on Norling 2018 population projections to determine reservoir requirements across the 2021-2036 planning cycle. The service catchment demand of each reservoir is summarised in Table 9-2 below, with a summary of the total spare capacity available within the Whitsundays network included in Table 9-3 based on total reservoir volumes.

Refer to the detailed reservoir calculation summary located in Appendix B for further information.

Table 9-2 Whitsundays Network Reservoir Summary

| Reservoir | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Service Catchment Demand (EPs) ^{N1} | | | |
|----------------------------------|-------------------|---|--|---------|---------|---------|
| | | | 2021 | 2026 | 2031 | 2036 |
| Proserpine (LL+HL) | 5.254 | 4.203 | 8895.20 | 9131.36 | 9367.51 | 9564.31 |
| Mt Julian | 0.722 | 0.577 | 382.74 | 443.61 | 500.68 | 562.57 |
| Cannon Valley ^{N2} | 15.858 | 12.686 | 4464.98 | 5175.12 | 5840.87 | 6562.84 |
| Coyne Rd LL | 2.278 | 1.823 | 2304.65 | 2671.19 | 3014.83 | 3387.48 |
| Coyne Rd HL | 0.132 | 0.106 | 290.88 | 337.14 | 380.51 | 427.55 |
| Cannonvale | 12.818 | 10.254 | 4913.17 | 5694.59 | 6427.17 | 7221.61 |
| Airlie Summit | 0.398 | 0.319 | 180.62 | 209.35 | 236.28 | 265.48 |
| Moonlight Dr | 0.247 | 0.198 | 69.58 | 80.65 | 91.02 | 102.27 |
| Sanctuary Dr (NEW) ^{N3} | 0.188 | 0.150 | - | - | - | - |
| Shute Harbour LL | 0.359 | 0.287 | 3.95 | 4.58 | 5.17 | 5.81 |
| Shute Harbour HL | 0.577 | 0.461 | 1176.85 | 1364.02 | 1539.50 | 1729.79 |
| Daydream | 0.393 | 0.314 | 798.31 | 925.28 | 1044.31 | 1173.39 |
| Satinwood Ct | 1.001 | 0.801 | 201.16 | 233.15 | 263.15 | 295.67 |
| Macona Cres | 0.267 | 0.214 | 178.02 | 206.33 | 232.88 | 261.66 |

| Reservoir | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Service Catchment Demand (EPs) ^{N1} | | | |
|----------------|-------------------|---|--|--------|--------|--------|
| | | | 2021 | 2026 | 2031 | 2036 |
| Pepperberry Ln | 0.427 | 0.342 | 219.16 | 254.02 | 286.69 | 322.13 |
| Hamilton Park | 0.039 | 0.031 | 50.70 | 58.76 | 66.32 | 74.52 |

^{N1} Some reservoir supply areas vary depending on loading scenario, above factored based on 2021 supply catchments

^{N2} EPs supplied = Dedicated supply from Cannon Valley res – does not include catchments of reservoirs supplied via Cannon Valley gravity feed

^{N3} Sanctuary Dr new proposed tank – primarily catering for fire flow requirements with small (<20ET) residential catchment peak day standard flow supplementation – service area TBD

Table 9-3 Whitsundays Network Reservoir Assessment Summary

| Demand Scenario | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Spare Capacity (ML) ^{N1} | | | |
|-----------------|-------------------|---|-----------------------------------|--------|--------|--------|
| | | | 2021 | 2026 | 2031 | 2036 |
| 500L/EP/d | 40.959 | 32.767 | 8.962 | 5.846 | 2.556 | -0.558 |
| 450L/EP/d | | | 11.815 | 9.006 | 6.005 | 3.197 |
| 350L/EP/d | | | 17.342 | 15.276 | 12.903 | 10.707 |

^{N1} Spare overall network storage capacity based on total reservoir volumes

9.1.3 PUMP AUGMENTATION REQUIREMENTS

9.1.3.1 WPPS01

The pump station fed via the Proserpine LL reservoir and servicing the Proserpine HL Water Tower has been assessed following its associated pipe capacity augmentation, for comparison of current performance against required performance as per WRC DSS. The pipe capacity augmentation in question consists of a replacement of the existing DN225 AC class C main from WPPS01 pump station, with a proposed DN375 (ID386mm) mPVC main for the majority of the underground section of the pipeline and an OD419 (ID383mm) MSCL main for the above ground segments up to the tower, noting pipe material selection is preliminary only and may be changed according to detail design requirements.

Following this pipe replacement, performance of the existing Grundfos HS 200x150-380 pumpset is able to do the following, based on average performance at tank ≈halfway through filling:

- Existing pump performance ≈180.5L/s @ 35.5m

Based on WRC DSS, Table 9-4 below lists the required capacity of treated water pumps feeding an elevated reservoir across the three assessed demand scenarios.

Table 9-4 WPPS01 Required Capacity

| WPPS01 | 2021 | 2026 | 2031 | 2036 |
|---------------|-------|-------|-------|-------|
| A – 500L/EP/d | 220.0 | 226.2 | 232.5 | 237.7 |
| B – 450L/EP/d | 196.5 | 202.1 | 207.7 | 212.4 |
| C – 350L/EP/d | 149.4 | 153.7 | 158.1 | 161.8 |

A deficiency in capacity is seen in scenarios A & B, whilst existing capacity remains adequate throughout demand scenario C. As such, pump capacity augmentations have been specified for the non-compliant demand scenarios, with an indicative pump selection provided below based on the most conservative requirement being the 2036 planning horizon under base 500L/EP/d demand. A preliminary selection is included below for information consisting of a 1+1 pumpset configuration based on the existing pump model HS in Figure 9-2 below.

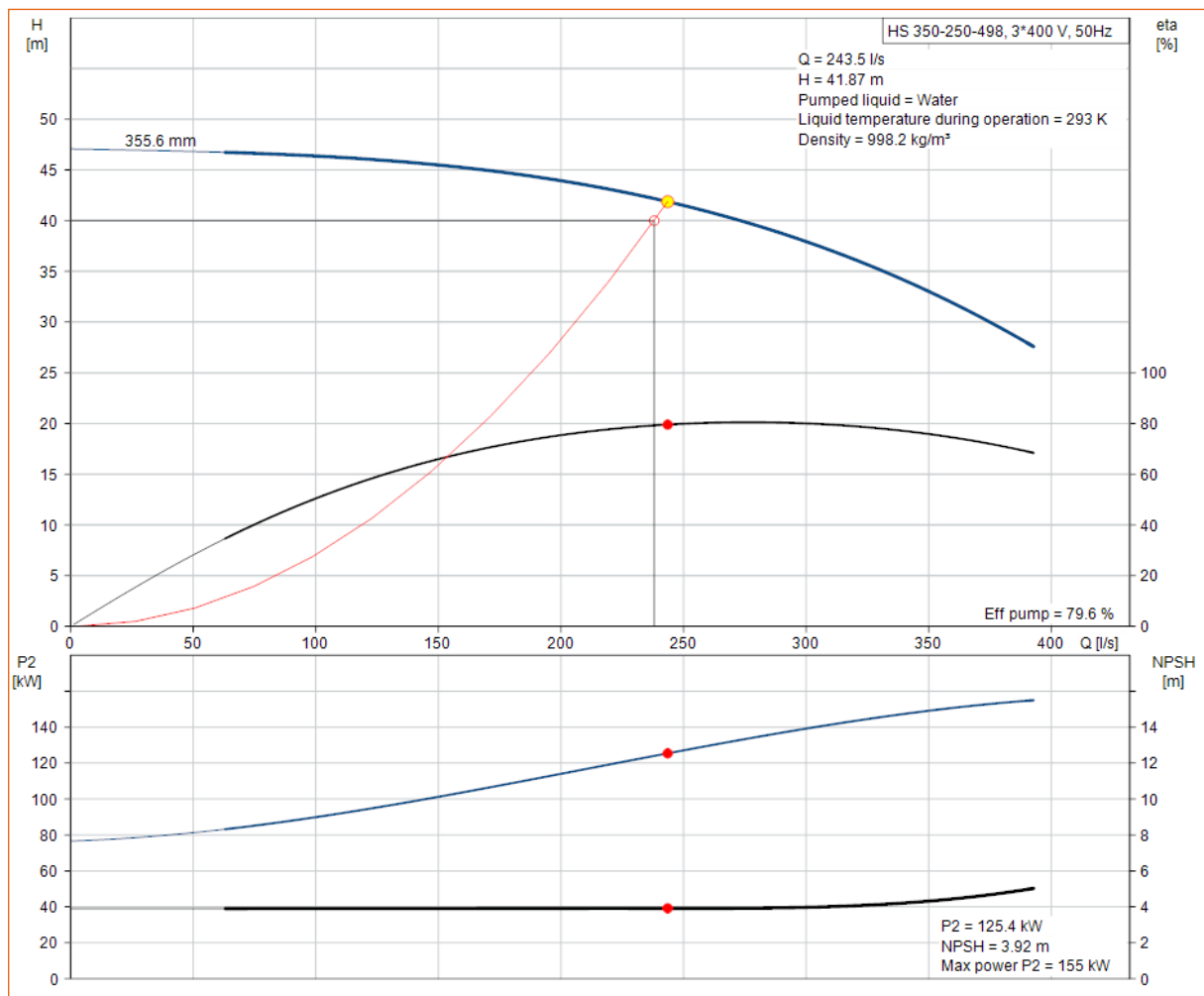


Figure 9-2 Grundfos HS 350-250-498 Indicative Proserpine HL Supply Pump Curve – 500L/EP/d 2036 Requirement

9.1.3.2 Bulk Water Pipeline Pumpset

An augmentation to the existing pumpset supplying the Whitsundays bulk water pipeline from Proserpine WTP to the trunk water supply line downstream of Booster 2 pump station has been specified, for implementation in the 2031 500L/EP/d scenario or the 2036 450L/EP/d scenario. The existing pumpset consists of three parallel Flowserve Hydro-Titan 150x125-315 pumps. The augmentation specified consists of increasing impeller size of the existing pumpset from the current 301mm impellers to 307mm whilst still allowing the pumps to function off the currently installed motors. Doing so provides a modest increase in design performance from 97.5L/s @ 109m per pump to 98L/s @ 114m, allowing for better performance from the future Cannon Valley reservoir in maintaining adequate levels during a three consecutive peak day demand event. The augmented performance curve of the Bulk Water Pipeline Pumpset following impeller size increase is included in Figure 9-3 below.

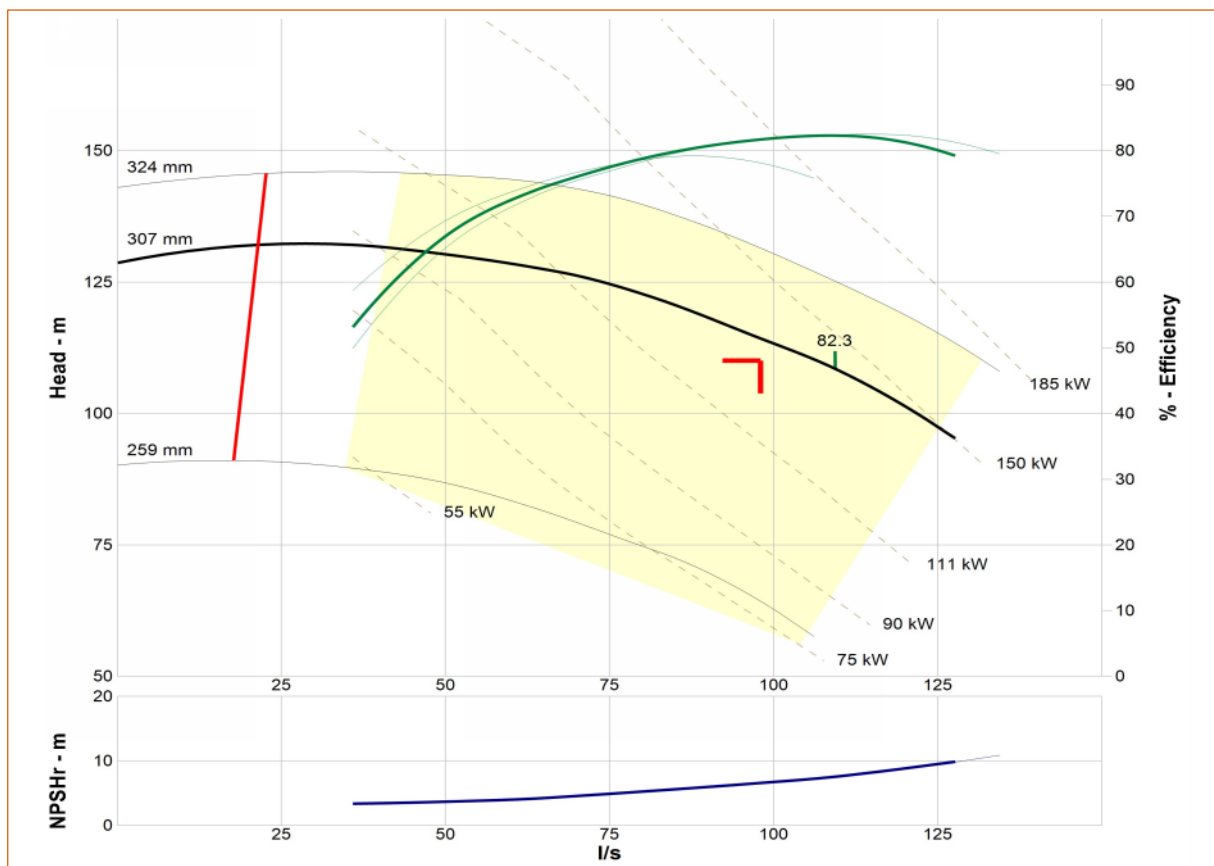


Figure 9-3 Flowserve Hydro-Titan 150x125-315 - 307mm Impeller

9.1.4 PROSERPINE WATER SUPPLY SECURITY

An opportunity has been identified to supplement water supply capability to the Proserpine area following the notable increase in projected population from the updated Norling 2018 study. With the majority of Proserpine being serviced via the Proserpine HL water tower, the supply pumps from the Proserpine GL reservoir feeding the HL water tower are a key asset in maintaining supply to the Proserpine catchment.

To avoid sole reliance on this pumpset and to provide some redundancy to the supply network servicing Proserpine, it is recommended to introduce a gravity supply option from the Cannon Valley reservoir. This would likely be achieved via the use of the supply line previously used by the Booster 1 pump station to transfer water up to the north-eastern part of the network i.e. Cannonvale and beyond, which

is to be decommissioned following the introduction of the Bulk Water Pipeline. Use of this Booster 1 pipeline as a gravity main fed via direct connection to the Cannon Valley reservoir allows for reliable gravity service to the Mt Julian area and would also allow for this gravity supply to extend to the Proserpine catchment as an alternate supply source in case of any issues with the Proserpine HL water tower and associated supply pumpset. This option would require the use of PRVs to limit service pressure from direct gravity feed via Cannon Valley reservoir.

9.1.5 NETWORK AUGMENTATION SUMMARY

Refer to Appendix A for detailed breakdown of pipe & pump augmentations required for network performance to WRC DSS for the Whitsundays potable water network.

Option A – 500L/EP/d

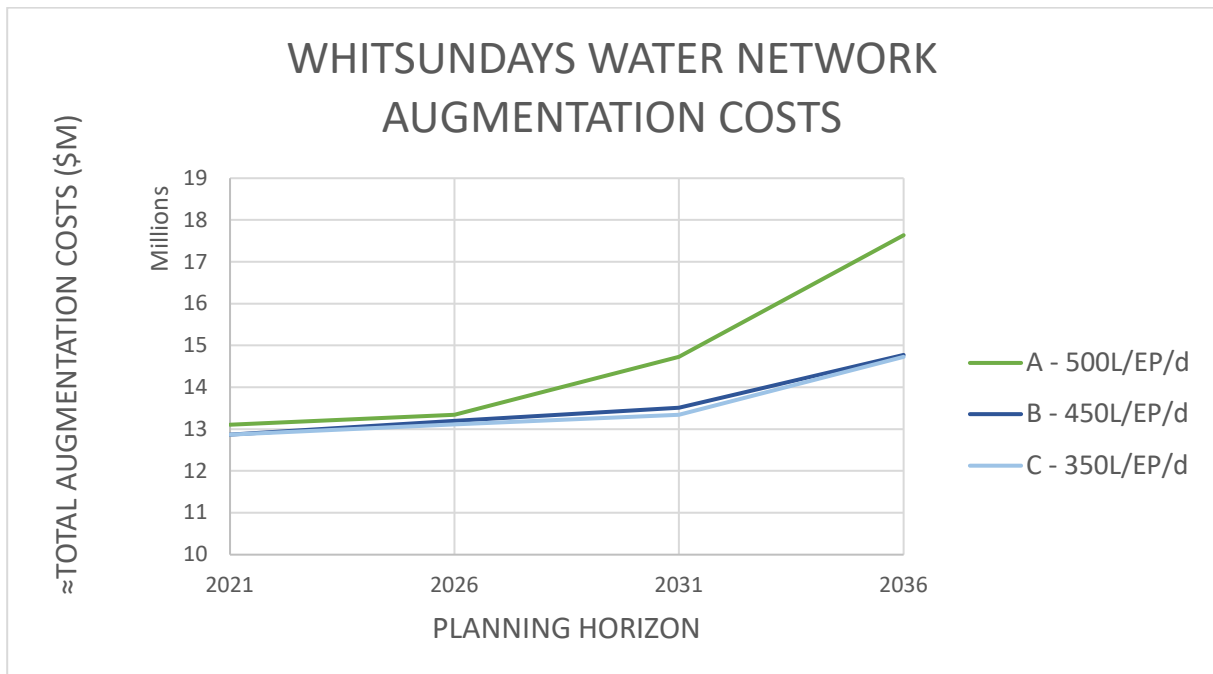
| | 2021 | 2026 | 2031 | 2036 |
|--------------------|--|--|--|--|
| Pipe Augmentations | 131 pipes ≈18,584m total pipe length ≈\$13,111,315 | 137 pipes ≈19,070m total pipe length ≈\$13,340,206 | 148 pipes ≈21,374m total pipe length ≈\$14,730,410 | 197 pipes ≈26,575m total pipe length ≈\$17,639,716 |
| Pump Augmentations | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps Proserpine WTP Bulk Water Pipeline pumps | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps Proserpine WTP Bulk Water Pipeline pumps |

Option B – 450L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|--|--|--|--|
| Pipe Augmentations | 126 pipes ≈18,027m total pipe length ≈\$12,867,797 | 133 pipes ≈18,767m total pipe length ≈\$13,196,108 | 139 pipes ≈19,379m total pipe length ≈\$13,509,301 | 149 pipes ≈21,476m total pipe length ≈\$14,777,616 |
| Pump Augmentations | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps | <ul style="list-style-type: none"> Proserpine HL reservoir supply pumps Proserpine WTP Bulk Water Pipeline pumps |

Option C – 350L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|--|--|--|--|
| Pipe Augmentations | 126 pipes ≈18,027m total pipe length ≈\$12,867,797 | 131 pipes ≈18,584m total pipe length ≈\$13,111,315 | 137 pipes ≈19,070m total pipe length ≈\$13,340,207 | 148 pipes ≈21,374m total pipe length ≈\$14,730,410 |
| Pump Augmentations | | | | |



9.2 BOWEN WATER NETWORK

9.2.1 RESERVOIR IMPACTS

The service catchment of all active reservoirs within the Bowen catchment have been isolated in the 2021 horizon and factored accordingly based on Norling 2018 population projections to determine reservoir requirements across the 2021-2036 planning cycle. The service catchment demand of each reservoir is summarised in Table 9-5 below, with a summary of the total spare capacity available within the Bowen network included in Table 9-6 based on total reservoir volumes.

Refer to the detailed reservoir calculation summary located in Appendix B for further information.

Table 9-5 Bowen Network Reservoir Summary

| Reservoir | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Service Catchment Demand (EPs) ^{N1} | | | |
|-------------------|-------------------|---|--|---------|----------|----------|
| | | | 2021 | 2026 | 2031 | 2036 |
| Bowen Central Res | 17.091 | 13.673 | 9019.32 | 9513.51 | 10265.41 | 10790.84 |
| Heronvale (NEW) | 0.884 | 0.707 | 197.10 | 199.96 | 207.77 | 210.72 |

^{N1} Some reservoir supply areas vary depending on loading scenario, above factored based on 2021 supply catchments

Table 9-6 Bowen Network Reservoir Assessment Summary

| Demand Scenario | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Spare Capacity (ML) ^{N1} | | | |
|-----------------|-------------------|---|-----------------------------------|-------|-------|-------|
| | | | 2021 | 2026 | 2031 | 2036 |
| 500L/EP/d | 17.091 | 13.673 | 5.817 | 5.199 | 4.259 | 3.602 |
| 450L/EP/d | | | 6.944 | 6.388 | 5.542 | 4.951 |
| 350L/EP/d | | | 9.199 | 8.767 | 8.109 | 7.649 |

^{N1} Spare overall network storage capacity based on total reservoir volumes

^{N2} New Heronvale reservoir omitted from above summary

9.2.2 NETWORK AUGMENTATION SUMMARY

Refer to Appendix A for detailed breakdown of pipe & pump augmentations required for network performance to WRC DSS for the Bowen potable water network.

Option A – 500L/EP/d

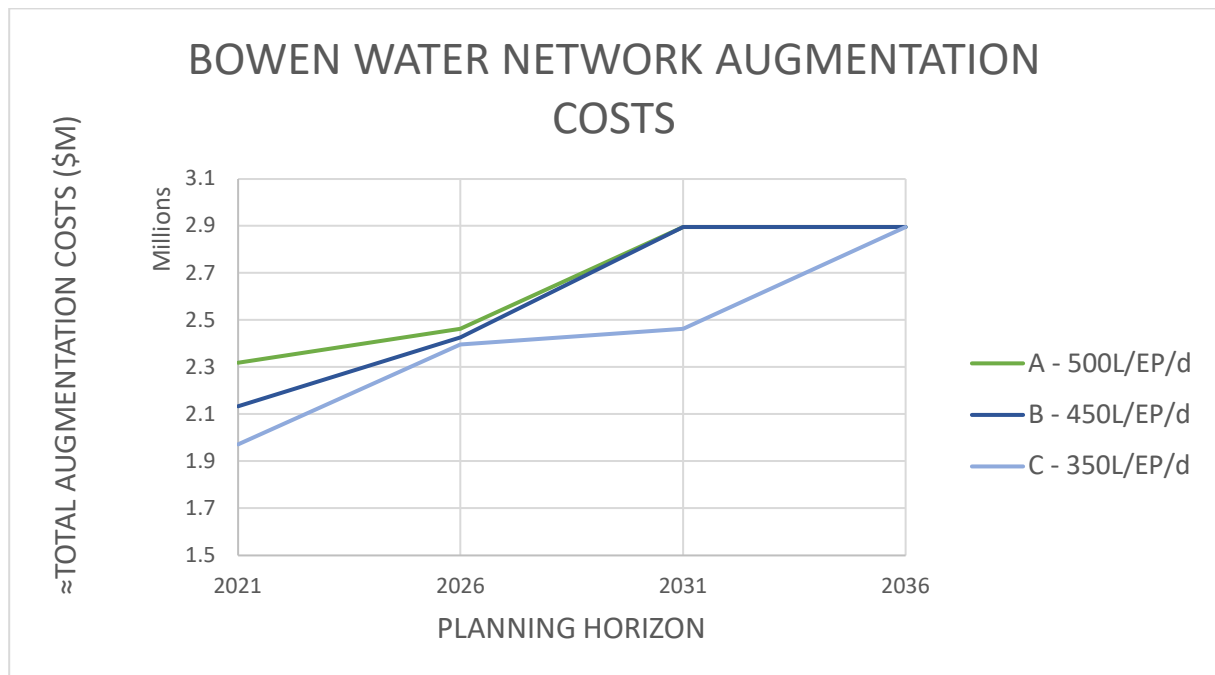
| | 2021 | 2026 | 2031 | 2036 |
|--------------------|---|---|---|---|
| Pipe Augmentations | 31 pipes ≈4,843m total pipe length ≈\$2,318,354 | 34 pipes ≈5,210m total pipe length ≈\$2,461,970 | 43 pipes ≈6,123m total pipe length ≈\$2,894,765 | 43 pipes ≈6,123m total pipe length ≈\$2,894,765 |
| Pump Augmentations | | | | |

Option B – 450L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|---|---|---|---|
| Pipe Augmentations | 28 pipes ≈4,433m total pipe length ≈\$2,133,198 | 33 pipes ≈5,129m total pipe length ≈\$2,425,393 | 43 pipes ≈6,123m total pipe length ≈\$2,894,765 | 43 pipes ≈6,123m total pipe length ≈\$2,894,765 |
| Pump Augmentations | | | | |

Option C – 350L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|---|---|---|---|
| Pipe Augmentations | 26 pipes ≈4,119m total pipe length ≈\$1,971,431 | 32 pipes ≈5,058m total pipe length ≈\$2,396,101 | 34 pipes ≈5,210m total pipe length ≈\$2,461,970 | 43 pipes ≈6,123m total pipe length ≈\$2,894,765 |
| Pump Augmentations | | | | |



9.3 COLLINSVILLE WATER NETWORK

9.3.1 RESERVOIR IMPACTS

The service catchment of all active reservoirs within the Collinsville catchment have been isolated in the 2021 horizon and factored accordingly based on Norling 2018 population projections to determine reservoir requirements across the 2021-2036 planning cycle. The service catchment demand of each reservoir is summarised in Table 9-7 below, with a summary of the total spare capacity available within the Collinsville network included in Table 9-8 based on total reservoir volumes.

The total spare network capacity assessed in Table 9-8 includes the total demand and volumes of both reservoirs within the network. This is only possible due to the Peter Delemothe Rd reservoir spare capacity being able to be utilised by the Miller St reservoirs via gravity feed.

Refer to the detailed reservoir calculation summary located in Appendix B for further information.

Table 9-7 Collinsville Network Reservoir Summary

| Reservoir | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Service Catchment Demand (EPs) ^{N1} | | | |
|----------------------------------|-------------------|---|--|---------|---------|---------|
| | | | 2021 | 2026 | 2031 | 2036 |
| Peter Delemothe Rd ^{N2} | 6.107 | 4.886 | 2015.78 | 2002.90 | 2067.26 | 2155.79 |
| Miller St | 1.615 | 1.292 | 1674.20 | 1707.05 | 1762.71 | 1784.16 |

^{N1} Some reservoir supply areas vary depending on loading scenario, above factored based on 2021 supply catchments

^{N2} EPs supplied = Dedicated supply from Peter Delemothe Rd res – does not include catchments of reservoirs supplied via Peter Delemothe Rd res gravity feed

Table 9-8 Collinsville Network Reservoir Assessment Summary

| Demand Scenario | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Spare Capacity (ML) ^{N1} | | | |
|-----------------|-------------------|---|-----------------------------------|-------|-------|-------|
| | | | 2021 | 2026 | 2031 | 2036 |
| 500L/EP/d | 7.722 | 6.178 | 2.639 | 2.616 | 2.481 | 2.357 |
| 450L/EP/d | | | 3.054 | 3.034 | 2.912 | 2.801 |
| 350L/EP/d | | | 3.884 | 3.868 | 3.774 | 3.687 |

^{N1} Spare overall network storage capacity based on total reservoir volumes

9.3.2 NETWORK AUGMENTATION SUMMARY

Refer to Appendix A for detailed breakdown of pipe & pump augmentations required for network performance to WRC DSS for the Collinsville potable water network.

Option A – 500L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|---|---|---|---|
| Pipe Augmentations | 17 pipes ≈2,576m total pipe length ≈\$1,175,785 | 18 pipes ≈2,677m total pipe length ≈\$1,222,465 | 18 pipes ≈2,677m total pipe length ≈\$1,222,465 | 18 pipes ≈2,677m total pipe length ≈\$1,222,465 |
| Pump Augmentations | | | | |

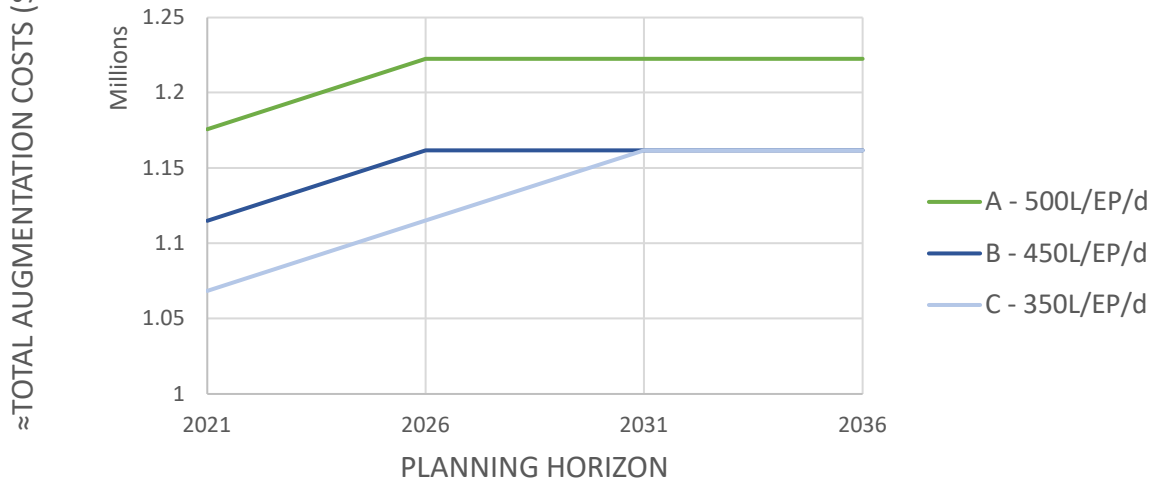
Option B – 450L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|---|---|---|---|
| Pipe Augmentations | 16 pipes ≈2,458m total pipe length ≈\$1,114,994 | 17 pipes ≈2,559m total pipe length ≈\$1,161,673 | 17 pipes ≈2,559m total pipe length ≈\$1,161,673 | 17 pipes ≈2,559m total pipe length ≈\$1,161,673 |
| Pump Augmentations | | | | |

Option C – 350L/EP/d

| | 2021 | 2026 | 2031 | 2036 |
|--------------------|---|---|---|---|
| Pipe Augmentations | 15 pipes ≈2,329m total pipe length ≈\$1,068,345 | 16 pipes ≈2,458m total pipe length ≈\$1,114,994 | 17 pipes ≈2,559m total pipe length ≈\$1,161,673 | 17 pipes ≈2,559m total pipe length ≈\$1,161,673 |
| Pump Augmentations | | | | |

COLLINSVILLE WATER NETWORK AUGMENTATION COSTS



10 SEWERAGE SCENARIO OUTCOMES

Following updating of the sewerage models, the increased flows outlined in the LGIP through the 2021 to 2036 horizons were assessed against the DSS. The assessment determined large augmentations of the pipe network, operational storage, and sewer pump capacities were necessary. The determined augments are discussed in detail below. Refer to Appendix C for a breakdown of sewer network augmentations and mapping.

10.1.1 PIPE AUGMENTATIONS

Pipe augmentations were undertaken with the following points noted:

- All new augmentations were sized using internal diameters for PVC-U pipelines from AS1477;
- All AC pipes 150mm in diameter or smaller were replaced and not duplicated;
- Replacement was also taken on AC pipes known to be un-serviceable and in areas where failure may lead to unacceptable risks; and
- New gravity pipes were sized to provide a max depth of flow of 75% in line with WRC guidelines.

Table 10-1 Pipe Augmentations Summary 2021 – 2036 horizons

| Horizon | 2021 | 2026 | 2031 | 2036 |
|---------------------------------|----------------------------------|-----------------------------------|-----------------------------------|-----------------------------------|
| Cannonvale Pipe Augmentations | 7 pipes ≈1386m total pipe length | 10 pipes ≈1556m total pipe length | 12 pipes ≈1664m total pipe length | 24 pipes ≈4274m total pipe length |
| Bowen Pipe Augmentations | 1 pipe ≈723m total pipe length | 2 pipes ≈1988m total pipe length | 2 pipes ≈1988m total pipe length | 2 pipes ≈1988m total pipe length |
| Proserpine Pipe Augmentations | 5 pipes ≈2948m total pipe length | 5 pipes ≈2948m total pipe length | 5 pipes ≈2948m total pipe length | 5 pipes ≈2948m total pipe length |
| Collinsville Pipe Augmentations | 1 pipe ≈175m total pipe length | 1 pipe ≈175m total pipe length | 1 pipe ≈175m total pipe length | 1 pipe ≈175m total pipe length |

10.1.2 PUMP CAPACITY & OPERATIONAL STORAGE AUGMENTATIONS

Table 10-2 Number of Pump Capacity & Operational Storage Upgrades Required - Cumulative

| Horizon | 2021 | 2026 | 2031 | 2036 |
|--|------|------|------|------|
| Cannonvale Operational and Storage Augmentations | 3 | 1 | 1 | 0 |
| Bowen Operational and Storage Augmentations | 1 | 0 | 0 | 0 |

| Horizon | 2021 | 2026 | 2031 | 2036 |
|--|------|------|------|------|
| Storage Augmentations | | | | |
| Proserpine Operational and Storage Augmentations | 6 | 0 | 0 | 0 |
| Collinsville Operational and Storage Augmentations | 1 | 0 | 0 | 0 |
| Number of Total Augmentations | 11 | 1 | 0 | 0 |

10.1.3 PUMP STATION DETAILS

A summary of pump station loading information has been included in Table 10-3 through to Table 10-6 below. Note existing pump capacity has been sourced via model curve extracts and data provided by WRC where possible, assessing available head of the pump against the required flow rate of its respective catchment in the 2036 horizon. Refer to Appendix D for a summary of WRC pump station performance characteristics and requirements.

Table 10-3 Cannonvale Pump Station Details

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|------|------|------|
| | | | 2021 | 2026 | 2031 | 2036 |
| PUMP-CANN1.2 | 15.00 | 59 L/s @ 35m | 2404 | 2786 | 3144 | 3533 |
| PUMP-CANN11.1 | 2.60 | | 970 | 1124 | 1269 | 1426 |
| PUMP-CANN12.1 | 1.80 | 52 L/s @ 12m | 2221 | 2574 | 2905 | 3264 |
| PUMP-CANN14.2 | 1.23 | | 106 | 123 | 139 | 156 |
| PUMP-CANN15.2 | 0.43 | N/A | 0 | 0 | 0 | 0 |
| PUMP-CANN17.1 | 16.00 | N/A | 14 | 16 | 18 | 20 |
| PUMP-CANN18.1 | 16.00 | N/A | 146 | 169 | 191 | 215 |

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|------|------|------|
| | | | 2021 | 2026 | 2031 | 2036 |
| PUMP-CANN2.2 | 2.04 | 25.5L/s @ 3.9m | 314 | 364 | 410 | 461 |
| PUMP-CANN3.1 | 15.00 | 33L/s @ 30m | 1774 | 2057 | 2321 | 2608 |
| PUMP-CANN4.1 | 8.68 | N/A | 116 | 135 | 152 | 171 |
| PUMP-CANN5.2 | 6.51 | 50L/s @ 51m | 2177 | 2523 | 2848 | 3200 |
| PUMP-CANN51.1 | 2.00 | N/A | 23 | 26 | 30 | 33 |
| PUMP-CANN6.2 | 39.00 | N/A | 680 | 789 | 890 | 1000 |
| PUMP-JUBI1.1 | 8.68 | | 3871 | 4487 | 5064 | 5690 |
| PUMP-JUBI2.2 | 4.98 | 55L/s @ 6m | 1537 | 1781 | 2011 | 2259 |
| PUMP-JUBI3.2 | 15.00 | N/A | 323 | 374 | 422 | 475 |
| PUMP-JUBI4.2 | 1.33 | 10.5L/s @ 6.7m | 398 | 461 | 520 | 584 |
| PUMP-SHUT1.2 | 2.54 | 24L/s @ 2.5m | 310 | 360 | 406 | 456 |
| PUMP-SHUT2.1 | 4.06 | N/A | 148 | 171 | 193 | 217 |
| PUMP-SHUT3.1 | 16.00 | N/A | 86 | 99 | 112 | 126 |

Table 10-4 Bowen Pump Station Details

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|-------|-------|-------|
| | | | 2021 | 2026 | 2031 | 2036 |
| 1-PUMPS | 61.65 | 300L/s @ 22m | 9289 | 10274 | 10871 | 11200 |
| 10-PUMPS | 1.52 | 22.5L/s @ 7.9m | 67 | 74 | 78 | 80 |
| 13-PUMPS | 1.52 | 40L/s @ 4.4m | 173 | 192 | 203 | 209 |

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|------|------|------|
| | | | 2021 | 2026 | 2031 | 2036 |
| 15-PUMPS | 1.52 | 4L/s @ 5m | 53 | 59 | 62 | 64 |
| 1A-PUMPS | 1.52 | 4L/s @ 2.7m | 6 | 6 | 7 | 7 |
| 1C-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| 2-PUMPS | 12.26 | 50L/s @ 16m | 1430 | 1582 | 1674 | 1724 |
| 2A-PUMPS | 1.52 | 8L/s @ 9m | 109 | 121 | 128 | 132 |
| 2B-PUMPS | 1.52 | 10.5L/s @ 9m5.4 | 213 | 236 | 250 | 257 |
| 2D-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| 2E-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| 2F-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| 2G-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| 3-PUMPS | 17.53 | 81L/s @ 34m | 2785 | 3080 | 3259 | 3358 |
| 4-PUMPS | 12.26 | 49L/s @ 28.5m | 1334 | 1475 | 1561 | 1608 |
| 4A-PUMPS | 1.52 | 32L/s @ 3.5m | 74 | 81 | 86 | 89 |
| A-PUMPS | 1.77 | 13L/s @ 14m | 248 | 275 | 291 | 300 |
| B-PUMPS | 2.14 | 11L/s @ 12m | 305 | 338 | 357 | 368 |
| C-PUMPS | 2.14 | 4L/s @ 10m | 351 | 388 | 411 | 423 |
| D-PUMPS | 1.78 | 10L/s @ 12m | 170 | 188 | 199 | 205 |
| E-PUMPS | 6.80 | 19L/s @ 6.7m | 1260 | 1393 | 1474 | 1519 |
| F-PUMPS | 1.41 | 10L/s @ 4m | 30 | 33 | 35 | 36 |
| G-PUMPS | 1.42 | 2L/s @ 4.5m | 194 | 215 | 227 | 234 |
| H-PUMPS | 6.80 | 64L/s @ 37m | 2214 | 2449 | 2592 | 2670 |
| J-PUMPS | 2.49 | 45L/s @ 13.5m | 1097 | 1213 | 1283 | 1322 |
| KING_ST | 29.22 | 4L/s @ 6m | 23 | 25 | 27 | 28 |

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|------|------|------|
| | | | 2021 | 2026 | 2031 | 2036 |
| L-PUMPS | 1.73 | 20L/s @ 7m | 633 | 700 | 741 | 763 |
| M-PUMPS | 2.71 | 3.5L/s @ 6m | 39 | 43 | 45 | 47 |
| N-PUMPS | 1.22 | 50L/s @ 9m | 6517 | 7208 | 7627 | 259 |
| O-PUMPS | 1.22 | 4.5L/s @ 5.2m | 86 | 95 | 100 | 103 |
| P-PUMPS | 0.92 | 22L/s @ 7m | 265 | 293 | 310 | 319 |
| Q1-PUMPS | 1.52 | 27L/s @ 7.5m | 109 | 121 | 128 | 132 |
| Q3-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| Q4-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| R-PUMPS | 2.49 | 40L/s @ 4m | 362 | 401 | 424 | 437 |
| S-PUMPS | 1.52 | N/A | 0 | 0 | 0 | 0 |
| T-PUMPS | 1.52 | 4L/s @ 6m | 13 | 15 | 16 | 16 |
| WS-PUMPS | 1.52 | N/A | 4123 | 0 | 0 | 0 |
| Z-PUMPS | 22.68 | 90L/s @ 25m | 4123 | 4560 | 4825 | 4971 |

Table 10-5 Collinsville Pump Station Details

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|------|------|------|
| | | | 2021 | 2026 | 2031 | 2036 |
| 1-C_PUMPS | 18.012 | 100L/s @ 28m | 2843 | 2858 | 2951 | 2958 |
| 2-C_PUMPS | 5.7096 | | 583 | 586 | 605 | 606 |
| B-C_PUMP | 1.524 | N/A | 19 | 19 | 19 | 20 |
| A-C_PUMPS | 1.524 | 4L/s @ 5m | 60 | 61 | 63 | 63 |

Table 10-6 Proserpine Pump Station Details

| Pump Station ID | Operational Storage Volume | Existing Pump Capacity | Catchment Loading (EPs) | | | |
|-----------------|----------------------------|------------------------|-------------------------|------|------|------|
| | | | 2021 | 2026 | 2031 | 2036 |
| PROS_6 | 1.59 | | 782 | 803 | 824 | 841 |
| PROS_PS1 | 10.97 | | 3365 | 3454 | 3543 | 3618 |
| PROS_PS10 | 0.80 | 15L/s @ 2m | 88 | 90 | 92 | 94 |
| PROS_PS11 | 0.80 | 15L/s @ 1.5m | 87 | 90 | 92 | 94 |
| PROS_PS12 | 0.80 | | 881 | 905 | 928 | 948 |
| PROS_PS2 | 5.85 | | 3170 | 3255 | 3339 | 3409 |
| PROS_PS3 | 2.92 | | 1568 | 1610 | 1651 | 1686 |
| PROS_PS4A | 1.33 | N/A | 0 | 0 | 0 | 0 |
| PROS_PS5 | 1.20 | 16.5L/s @ 4.5m | 478 | 491 | 503 | 514 |
| PROS_PS9 | 0.93 | | 471 | 483 | 496 | 506 |
| PUMP-11 | 0.80 | 35L/s @ 16m | 650 | 667 | 684 | 699 |

10.2 SEWERAGE COST ANALYSIS

A breakdown of the sewerage augmentation costs for the four models are provided below and include a 10% regional increase and 30% contingency as outlined in Section 5.2 – Cost Analysis Basis:

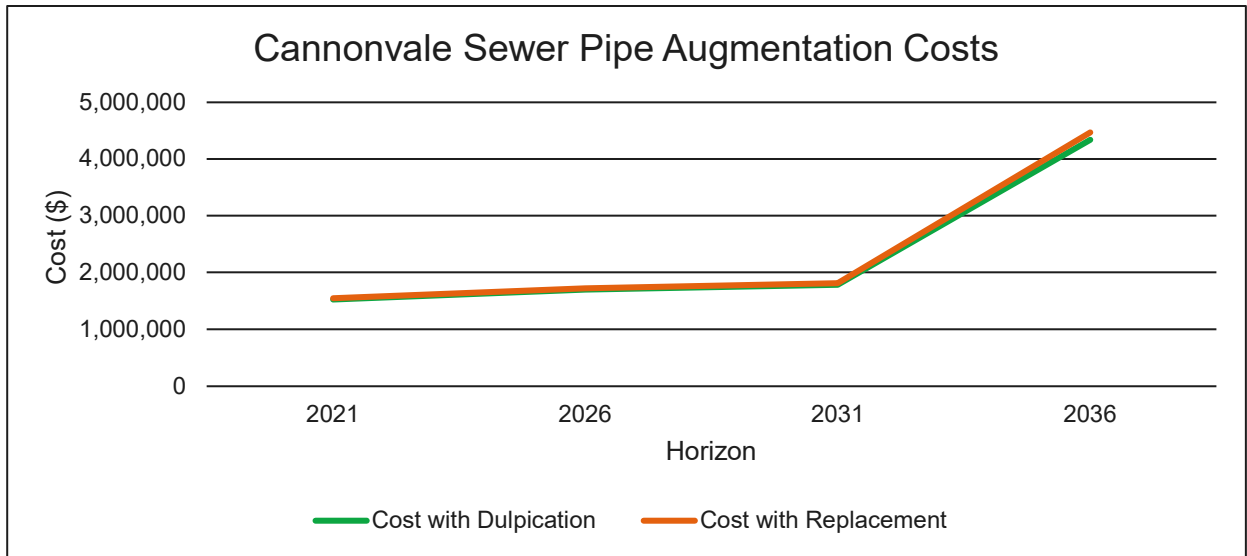
10.2.1 PIPE AUGMENTATIONS

10.2.1.1 CANNONVALE SEWER PIPE AUGMENTATION COSTS

Table 10-7 Cannonvale Sewer Pipe Augmentation Costs

| Horizon | 2021 | 2026 | 2031 | 2036 |
|--------------------|----------------------------------|-----------------------------------|-----------------------------------|-----------------------------------|
| Pipes | 7 pipes ≈1386m total pipe length | 10 pipes ≈1556m total pipe length | 12 pipes ≈1664m total pipe length | 24 pipes ≈4274m total pipe length |
| Cost – Duplication | \$1,524,149 | \$1,696,571 | \$1,782,641 | \$4,340,048 |
| Cost - Replacement | \$1,547,415 | \$1,719,837 | \$1,813,657 | \$4,469,596 |

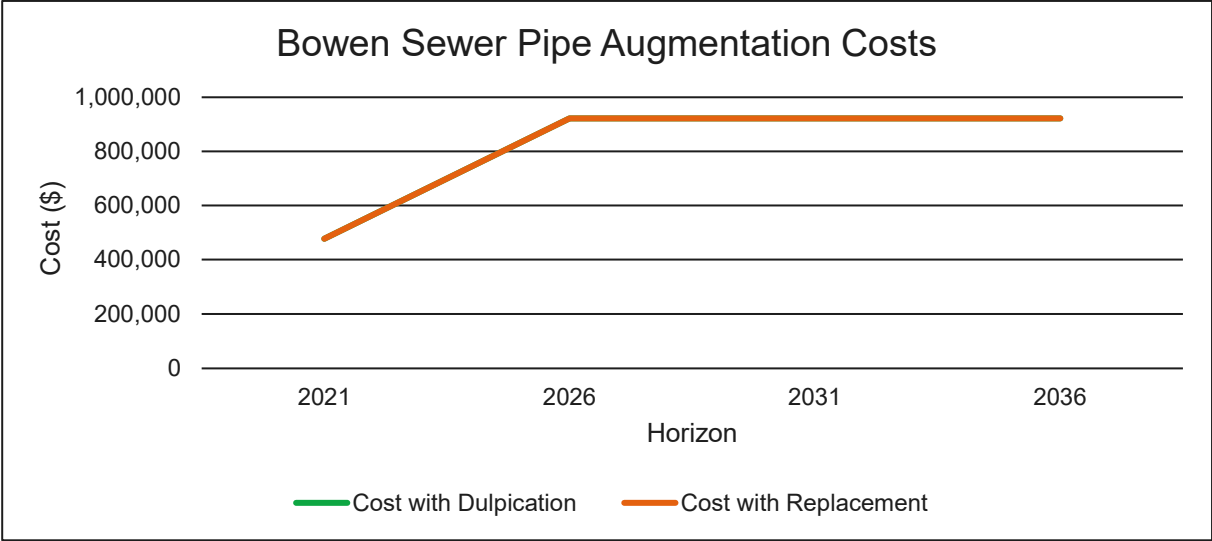
| Horizon | 2021 | 2026 | 2031 | 2036 |
|---------------------------|----------|----------|----------|-----------|
| Cost Replacement Increase | \$23,266 | \$23,266 | \$31,016 | \$129,549 |



10.2.1.2 BOWEN SEWER PIPE AUGMENTATIONS COST

Table 10-8 Bowen Sewer Pipe Augmentation Costs

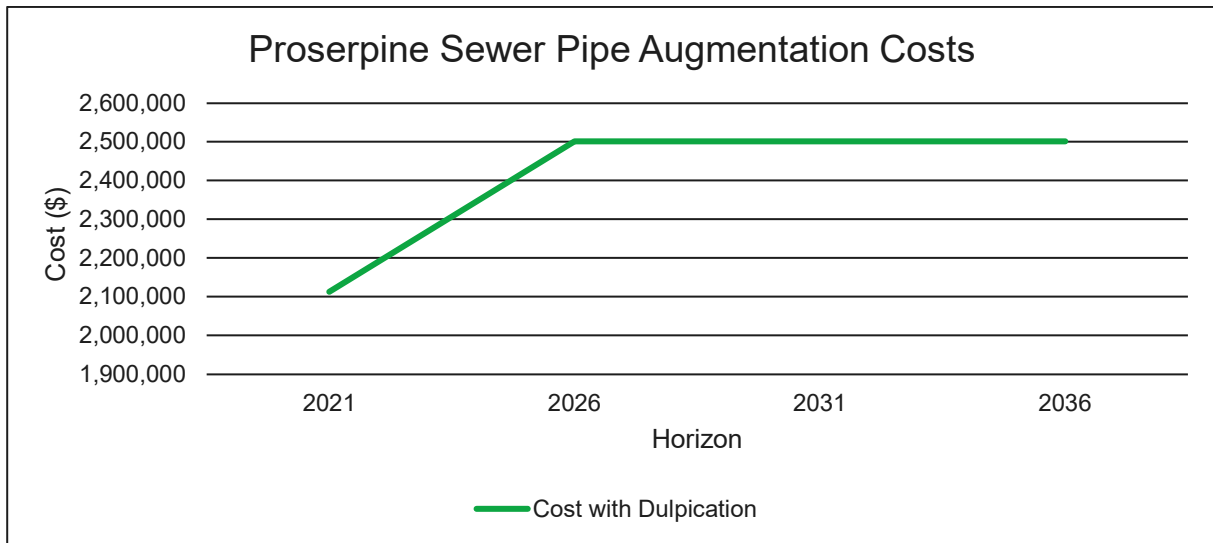
| Horizon | 2021 | 2026 | 2031 | 2036 |
|---------------------------|--------------------------------|----------------------------------|----------------------------------|----------------------------------|
| Pipes | 1 pipe ≈723m total pipe length | 2 pipes ≈1988m total pipe length | 2 pipes ≈1988m total pipe length | 2 pipes ≈1988m total pipe length |
| Cost – Duplication | \$477,387 | \$921,564 | \$921,564 | \$921,564 |
| Cost - Replacement | \$477,386.56 | \$921,564 | \$921,564 | \$921,564 |
| Cost Replacement Increase | - | - | - | - |



10.2.1.3 PROSERPINE SEWER PIPE AUGMENTATION COSTS

Table 10-9 Proserpine Sewer Pipe Augmentation Costs

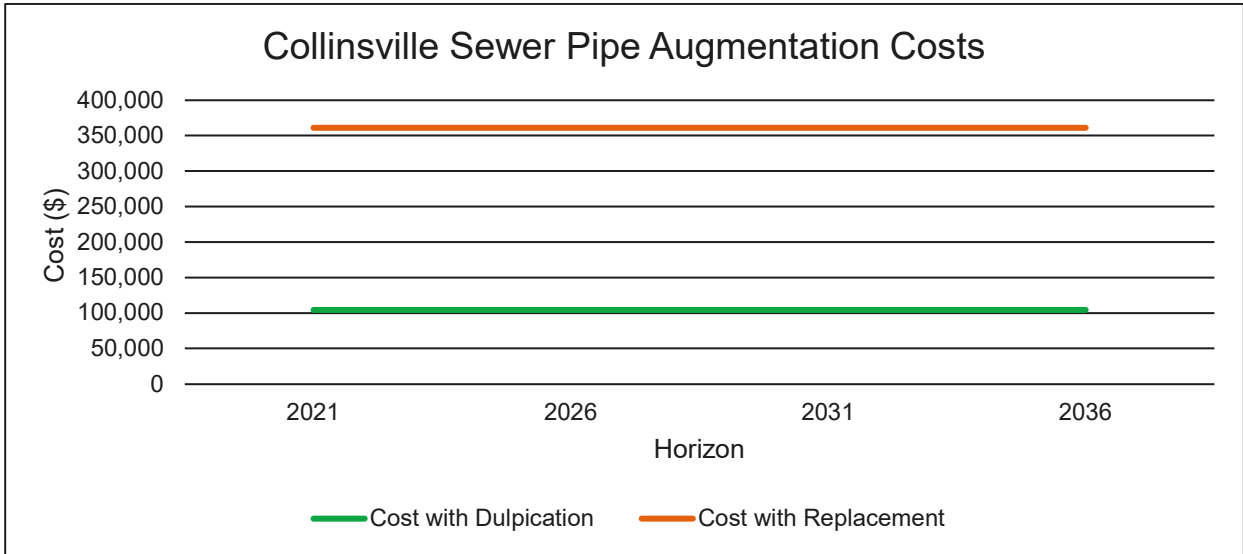
| Horizon | 2021 | 2026 | 2031 | 2036 |
|---------------------------|----------------------------------|----------------------------------|----------------------------------|----------------------------------|
| Pipes | 5 pipes ≈2948m total pipe length | 5 pipes ≈2948m total pipe length | 5 pipes ≈2948m total pipe length | 5 pipes ≈2948m total pipe length |
| Cost – Duplication | \$1,778,347 | \$1,778,347 | \$1,778,347 | \$1,778,347 |
| Cost - Replacement | - | - | - | - |
| Cost Replacement Increase | - | - | - | - |



10.2.1.4 COLLINSVILLE SEWER PIPE AUGMENTATION COSTS

Table 10-10 Collinsville Sewer Pipe Augmentation Costs

| Horizon | 2021 | 2026 | 2031 | 2036 |
|---------------------------|--------------------------------|--------------------------------|--------------------------------|--------------------------------|
| Pipes | 1 pipe ≈175m total pipe length | 1 pipe ≈175m total pipe length | 1 pipe ≈175m total pipe length | 1 pipe ≈175m total pipe length |
| Cost – Duplication | \$97,511 | \$97,511 | \$97,511 | \$97,511 |
| Cost - Replacement | \$239,554 | \$239,554 | \$239,554 | \$239,554 |
| Cost Replacement Increase | \$142,043 | \$142,043 | \$142,043 | \$142,043 |



10.2.1.5 TOTAL SEWER PIPE AUGMENTATION COSTS

Table 10-11 Total Sewer Pipe Augmentation Costs

| Horizon | 2021 | 2026 | 2031 | 2036 |
|--------------------|-------------|-------------|-------------|-------------|
| Cost – Duplication | \$4,117,211 | \$4,733,811 | \$4,819,880 | \$7,377,287 |
| Cost - Replacement | \$4,282,520 | \$4,899,119 | \$4,992,939 | \$7,648,878 |

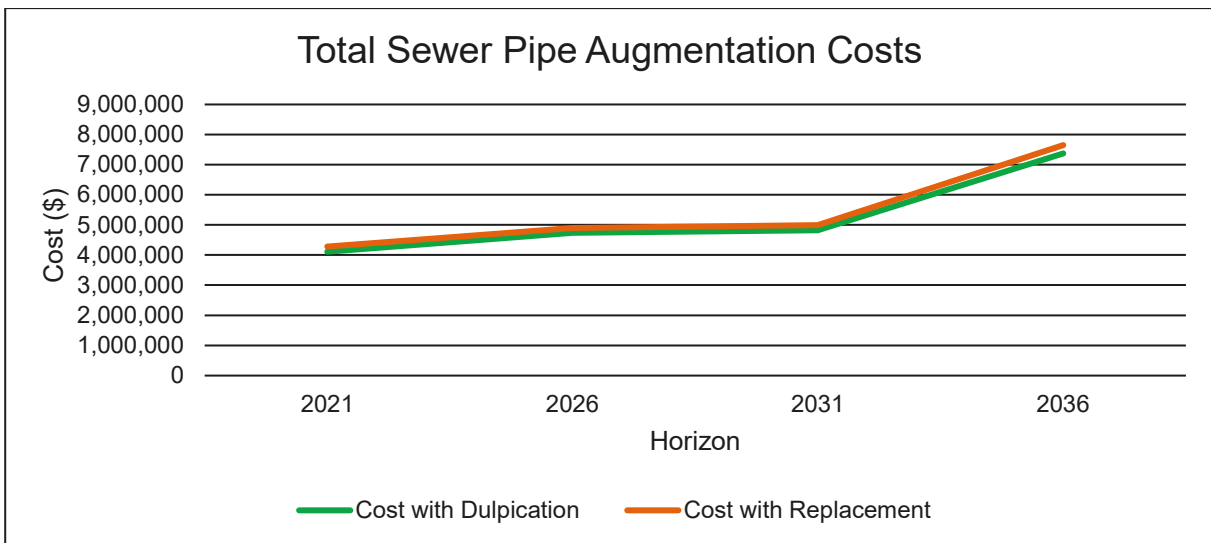


Figure 10-1 Total Sewer Pipe Augmentation Duplication/Replacement Comparison

10.2.2 SEWERAGE PUMP CAPACITY AND STORAGE UPGRADE COSTS – 2036 HORIZON

The costs of pump capacity upgrades and storage augments for the four locations through the horizons are represented below:

Table 10-12 Pump Capacity and Storage Upgrade Costs – 2036 Horizon

| | Cannonvale | Bowen | Proserpine | Collinsville | TOTAL |
|-----------------------------------|-----------------------|---------------------|---------------------|---------------------|-----------------------|
| Pump Capacity Upgrade Costs | \$1,036,699.66 | \$287,242.01 | \$740,071.59 | \$174,906.69 | \$2,238,919.95 |
| Operational Storage Upgrade Costs | \$21,556.70 | \$102,352.02 | \$2,624.99 | \$- | \$126,533.70 |
| TOTAL | \$1,058,256.35 | \$389,594.03 | \$742,696.57 | \$174,906.69 | \$2,365,453.65 |

11 DISCUSSION

11.1 INDIRECT IMPACTS

Beyond the consideration of the direct water network costs, Council should also consider the indirect implications of demand management on other Council systems as this would represent additional, albeit more modest benefits, that can be more fully explored, should Council consider this way forward desirable.

11.1.1 SEWAGE COLLECTION SYSTEM

Reducing per person water consumption not only impacts water supply systems, but also impacts sewage collection systems, reducing overall hydraulic loads. If a 150 L/EP/d reduction was achieved from 500L/EP/d, this equates to a 30% reduction in potable water demands and may also come with a 10% reduction in sewer flows, delaying augmentations within this system as well. Conversely increasing demands will bring forward augmentations to both the water networks (quantified) and the sewer collection and treatment system (not quantified).

11.1.2 STP

The impacts of altering the system demand do not impact the sewage treatment plants as much as they are often limited by their process load and not their hydraulic load. There will be some savings in particular sizing of primary and secondary tanks within as the total flows maybe reduced (from the base scenario) but the process vessels that are sized to nutrient loads may not be altered significantly.

There is potential for the concentration of the process load to be impacted (lower flows, similar quality loads) if a demand reduction was applied quickly. This in turn may impact the treatment ability of any biological nutrient removal due to influent concentration increase, however this would only be a short term impact and managed through pre-dosing of bacteria or ongoing operations management. In all likelihood, the water demand would occur over a longer period softening the impact.

Reflecting on the concentration increase and given Proserpine STP is approaching hydraulic limits and is facing a modest growth forecast, but has a higher process capacity. There may be an opportunity for demand management (and infiltration management) to defer spend on this facility.

11.1.3 RECYCLED WATER

In using recycled water (or alternative sources of raw water) to offset potable water use, demand on the potable system can be reduced and allow the existing potable water network to service additional customers as the per person potable water demand is reduced. With a higher percentage of effluent recycled it may be possible into the future for some form of environmental licence relaxation that could in turn lead to opportunities for additional cost savings.

11.2 POLICY POSITION OPTIONS

There are four policy positions investigated for Council's consideration with regard to urban water demand management. These options are:

- 1) Staged demand reduction – recommended;

- 2) Support current design demands;
- 3) Target a 450 L/p/d demand;
- 4) Target a 350 L/p/d demand;

In supporting each of these scenarios it is acknowledged that not all water users are the same. For this study three basic categories and sub-categories of water users are considered:

COUNCIL CONSUMPTION AND LOSSES

- Parks and gardens irrigation rates and source;
- Council facilities consumption rates;
- Pipe leaks, losses and bursts;
- Stand-pipe and fill point meter management; and
- Water theft management.

COMMERCIAL WATER CONSUMPTION

- Typically, savings come from structural efficiency improvement opportunities typical, over discretionary improvements;
- Influenced by total water cycle costs (charging trade waste, removes a current subsidy, from residents to business, and encourages efficiency). A clear price path may encourage the water savings to be put in place before full charges applied;
- Overall sector performance important, rather than individual business (as long as equitable policies enforced);
- Typically, lower percentage savings achievable, as water consumption is often viewed as an input cost and actively reduced by many businesses already; and
- Severe restrictions can impact the economy as hospitality and food service industries in particular trade on a reputation / standard.

RESIDENTIAL WATER CONSUMPTION

- Both structural and discretionary improvements typically achievable;
- Overall performance of region / town is important not individual performance business (as long as equitable policies enforced);
- Typically, higher percentage savings have been observed as response to information and pricing can lead to change in the way water is consumed; and
- Historically price sensitive

For demand management to be effective, their needs to be a focus on all three groups of water users. It is recommend that Council continue to lead by example in pursuit of “intelligent” water efficiency, and advocate for residents and businesses to work with us to achieve this community goal.

11.2.1.1 STAGED DEMAND REDUCTION – RECOMMENDED

By adopting a staged demand reduction program the strategy will target only the necessary demand reduction needed to defer new capital work expenditure by a rolling 5 years. This will then allow for demand management to be approached in a cost effective and non-disruptive way by avoiding the need to engage is subsidies and large scale compliance action.

In adopting this strategy the demands have been modelled in a step down approach with the following adopted:

- achieving 450 L/EP/day for each town from 2017 to 2026;
- reducing to 350 L/EP/day beyond this period.

To achieve it is recommended that target savings from each of our water using groups be proposed as outlined below:

ALL GROUPS

Recommend setting a regional water restrictions policy inclusive of when WRC is able to offer an allocation tariff.

- When Peter Faust Dam is below 60% at the start of any financial year, allocation tariff will not be available for customers to purchase. In doing this we will be straight up with our customers in being clear we will not pre-sell water when there is a risk we cannot supply the full volume, through applying water restrictions; and
- Set Water restrictions regionally by source, with Bowen Airlie / Cannonvale and Proserpine on common water restrictions set from the Peter Faust Dam level or in response to network constraints. Set Collinsville's water restrictions on Eungella Dam or in response to network constraints.

COUNCIL/WRC CONSUMPTION

- Continue to optimise our irrigation water consumption with Parks and Gardens with recycled and raw water sources where viable and through the use of efficient irrigation technology that Parks and Gardens are now deploying. This is a key component of WRC reducing it's own demands, through the Greening and Growing Bowen Program and Queens Beach Bore Concept. In using recycled water, we can maintain a level of amenity in the community with a lower reliance on potable water supplies;
- Continue to work with WRC/Council facilities to reduce water consumption and inefficiency when they are undertaking renewal activities;
- Continue to address leakage and losses through:
 - Reviewing and expanding our district meter program to target areas with higher losses;
 - Continuing to respond promptly to observed leaks;
 - Active investigation of "unusual greenery" as a potential sign of leakage;
 - Continued roll out of the Bowen Cast Iron Main Replacement program; and
 - Focused renewal of water meters with appropriately sized meters (reducing under-read of water consumption).
- More actively manage standpipe losses by working with our technology provider and looking for data gaps and anomalies and promptly following up; and
- Recommend Waste and water continues with their investigation into and compliance activities in reducing water theft and water losses that has resulted in reduced losses of over 500 ML last financial year.

BUSINESS CONSUMPTION

- Recommend that trade waste charging begin for all water users (on a 3 year price path or similar). In engaging with business customers noting to them the costs the Council faces in sourcing, treating water and collecting treating and disposing of / reusing their sewage. This would be done in the hope that this knowledge will lead to changes in water consumption where economic. The

Council should engage with them and advise that it is Council's preference that they look at their business and improve their water efficiency where they can and save them the charges and the Council, the cost of supplying additional capacity for our growing community. It is acknowledged that the region relies upon food manufacturing and services and hospitality, both are industries that need sufficient water to operate safely and for which the image as a clean and healthy place is essential. To support these and other industries in the mid-term it is suggested that the Council work with local businesses rather than to overtly seek to restrict their water use (from a common base).

RESIDENTIAL CONSUMPTION

- Recommend the volumetric set point associated with the allocation tariff to align with the design parameters and be set to 500kL / property / year (at the same price point, inflation adjusted). By doing this the Council can be confident in the near term that we can supply water that people may have pre-paid for. As the design set point is reduced, it is suggested that the allocation tariff is further reduced in line with the revised design set point;
- Recommend a low-cost water efficiency program based on school education, Facebook, Twitter and Instagram posting of what Council is doing to conserve water, and asking the community to work with the Council to do their part; and
- Suggest that adjustment of pricing to reduce the incentive of the current allocation tariff by raising the allocation tariff at inflation plus 2% for example to both the allocation charge and excess consumption charge.

11.2.1.2 500L TARGET DEMAND (CURRENT DESIGN BASIS)

By adopting the 500 L/EP strategy Council will define a standard for the community to achieve and adhere to. This has both positive negative implications:

- This is an expansionary target and may lead the community to believe that water and sewerage services are in fact cheap to supply and a "right", rather than a service that could be delivered efficiently;
- The expansionary target will result require all LGIP infrastructure to be provided (and cost more than \$30m over the next 15 years in additional capital expenditure should introduction of a new water source be required in the later planning horizons;
 - After each piece of infrastructure / source augmentation / allocation purchase is committed to, the costs to supply needs to be met, regardless of water demand reduction achieved after this date. There is a clear risk of a structural revenue shortfall by adopting this pathway, requiring Council to bail out the Water and Sewage business for an extended period rather than be a source of a modest dividend.
- In 2036, WRC may need to consider a growth cap / look at a new source (with all the associated costs) that would not be needed for another 10 or 20 years had moderate demand management been successful. Should the resultant price increase drive demand reduction after the new source is built Whitsunday water financial position may be severely challenged even with a doubling of retail water prices;
- In defining a numerical target user groups may splinter as the heavy lifting is born by one group vs another etc; and
- Residents and businesses may view the target as the long-term goal, rather than a journey to efficiency as we work together to deliver the necessary water and sewerage services as efficiently as possible.

It is proposed that adoption of similar types of policies to what the Council already has, but will require the allocation tariff be set at 500 kL / household per annum.

Only 12 of the 1067 residents that opted into allocation tariff last FY used over 500kL/a, this represents 1.12% of those who took up allocation tariffs or 0.08% of ratepayers more generally were allocation tariff holder who used over 500kL. In refining the allocation to this lower amount, the allocation tariff would be in alignment with our policy setting and infrastructure build program. Additionally, it is recommended that the draft water restrictions policy be reviewed and adopted.

11.2.1.3 450L TARGET DEMAND

By adopting the 450 L/EP strategy, WRC will define a target for the community to achieve. This has both positive negative implications:

- The modest target will be relatively easy to achieve;
- The modest target will result in modest drop in demand / revenue;
- In defining a numerical target user groups may splinter as the heavy lifting is born by one group vs another etc; and
- Residents and businesses may view the modest target as the long-term goal, rather than a journey to efficiency as we work together to deliver the necessary water and sewerage services as efficiently as possible.
- It is proposed that adoption of similar types of policies to what the Council already has, but will require the allocation tariff be set at 450 kL / household per annum.

11.2.1.4 350L TARGET DEMAND

By adopting the 350 L/EP strategy Council will define a target for the community to achieve. This has both positive negative implications:

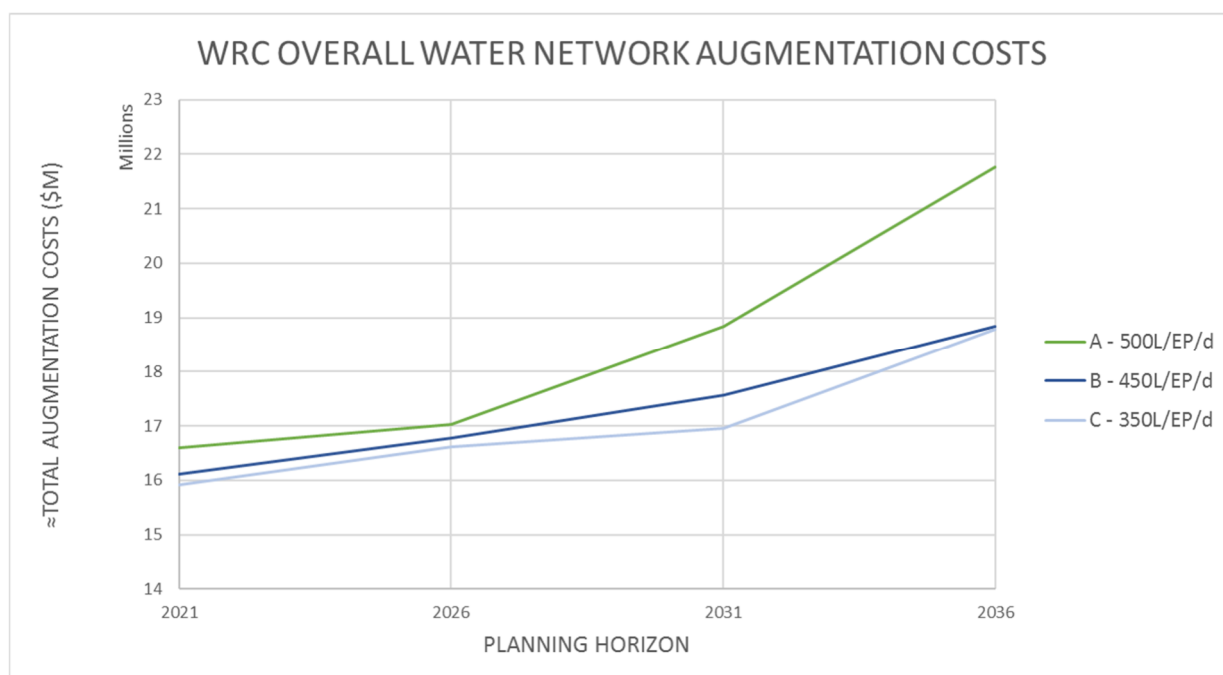
- The target will be relatively hard to achieve in a short time period;
- The target will result in drop in demand / revenue;
- In defining a numerical target user groups may splinter as the heavy lifting is born by one group vs another etc; and
- Residents and businesses may view the target as the long-term goal, rather than a journey to efficiency as we work together to deliver the necessary water and sewerage services as efficiently as possible

This option will more realistically involve a gradual progression to this target over time due to the above constraints and progression required to incorporate and integrate this target with the community.

12 CONCLUSION AND RECOMMENDATION

The above study has identified augmentations that may be required to the WRC water supply and sewer networks including assessment of varying water consumption profiles, yielding an updated 2020 LGIP set for all water and sewer networks. Water consumption option assessments serve to demonstrate the large impact that consumption assumptions have on network performance, infrastructure development and capital expenditure.

Augmentation requirements are observed to increase in direct proportion to the assigned demand per EP, with the following graph illustrating the overall water network augmentation costs per assessment option.

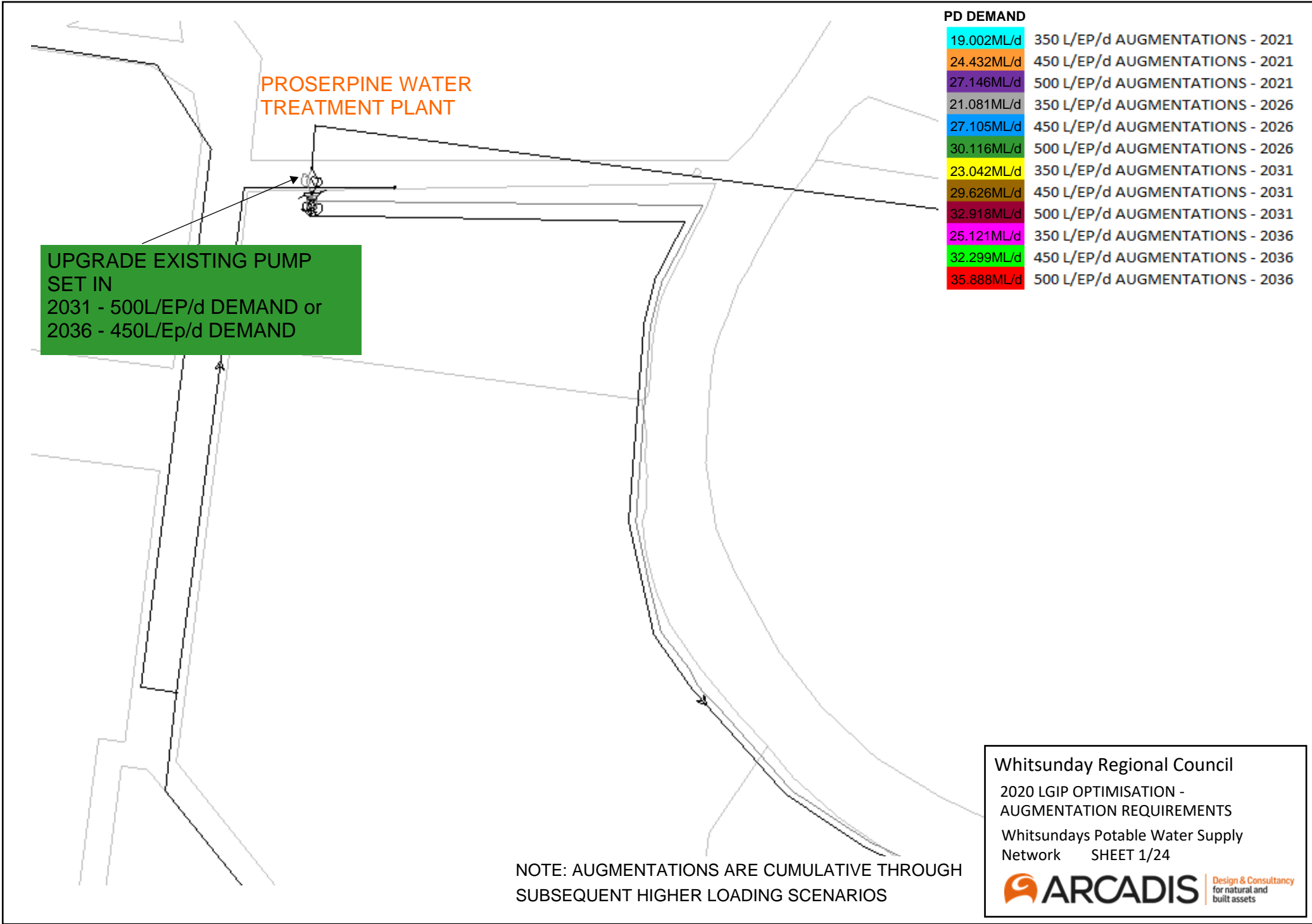


Assessment of the lower demand options demonstrated that the most notable augmentations required to the network may be implemented during later horizons or even omitted depending on the extent of the reduction, with progressively decreasing network augmentation requirements as demand decreases in the assessed option B (450L/EP/d) and option C (350L/EP/d).

When considering the substantial augmentation requirements, it is recommended that Council further investigate the opportunity to establish a water efficiency strategy which would allow for Council to delay a large number of augmentations to the network to later planning horizons. Depending on the adoption rate by the WRC community of the requested water demand targets, it may be possible to gradually decrease daily demand per person over a number of years, potentially achieving lower than 350L/EP/d. WRC note the potential saving of up to ≈\$1M of pipework infrastructure CAPEX for the 2021 horizon if water consumption per EP is dropped by 150L/EP/D from 500 down to 350 L/EP/Day as an indication of effectiveness of water demand management, with increasing savings in augmentation requirements and CAPEX in future planning horizons

APPENDIX A

POTABLE WATER NETWORK AUGMENTATION MAPPING AND SUMMARY



PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
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| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
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| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

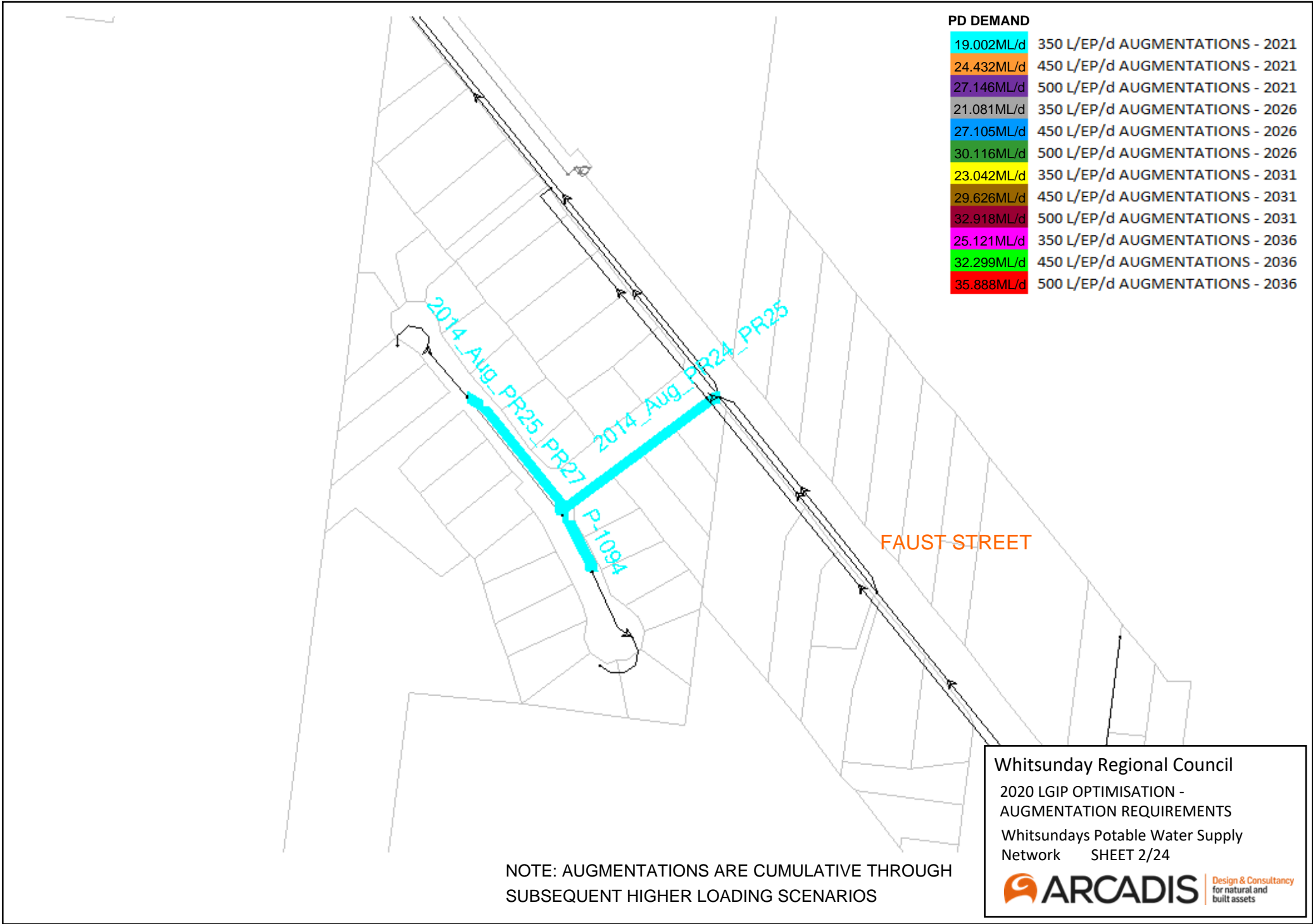
UPGRADE EXISTING PUMP SET IN
 2031 - 500L/EP/d DEMAND or
 2036 - 450L/EP/d DEMAND

PROSERPINE WATER TREATMENT PLANT

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
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| PD DEMAND | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
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 Whitsundays Potable Water Supply
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PD DEMAND

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| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

**UPGRADE EXISTING
PUMP SET IN
2021 - 350L/EP/d**

**PROSERPINE RAILWAY
STATION**

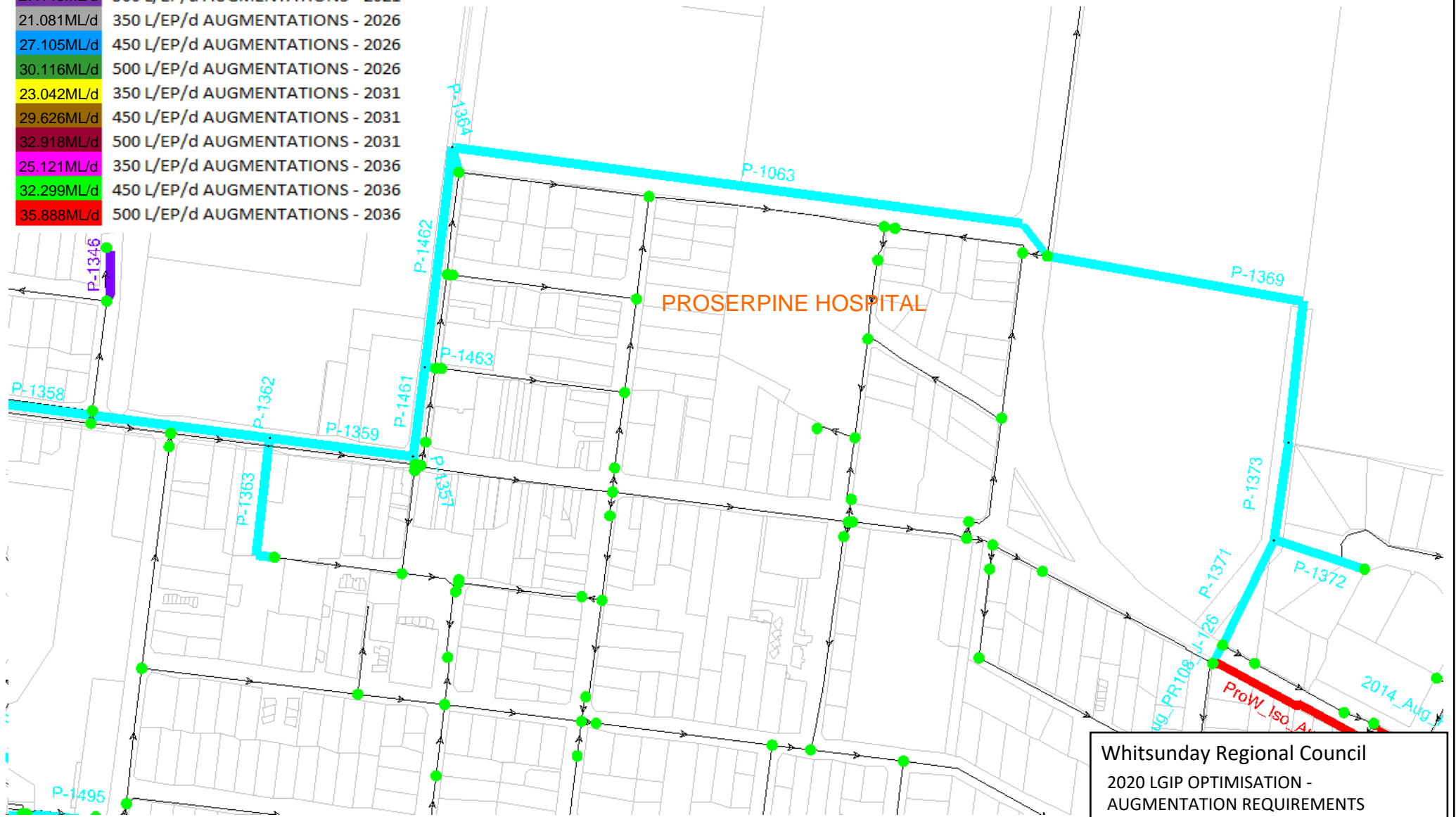
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

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2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS
Whitsundays Potable Water Supply
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PD DEMAND

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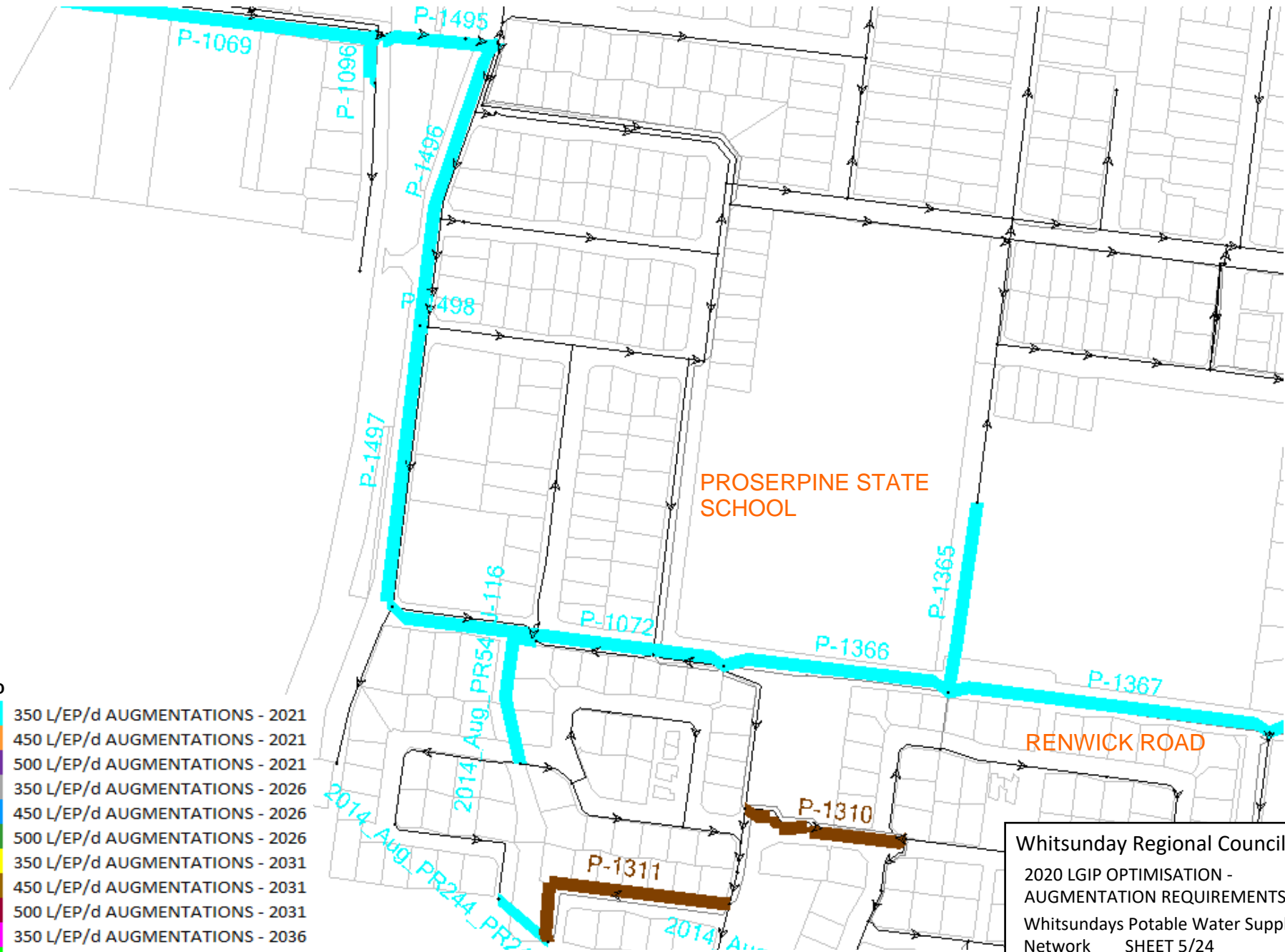


NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 4/24



Design & Consultancy for natural and built assets




PD DEMAND

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|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
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SUBSEQUENT HIGHER LOADING SCENARIOS

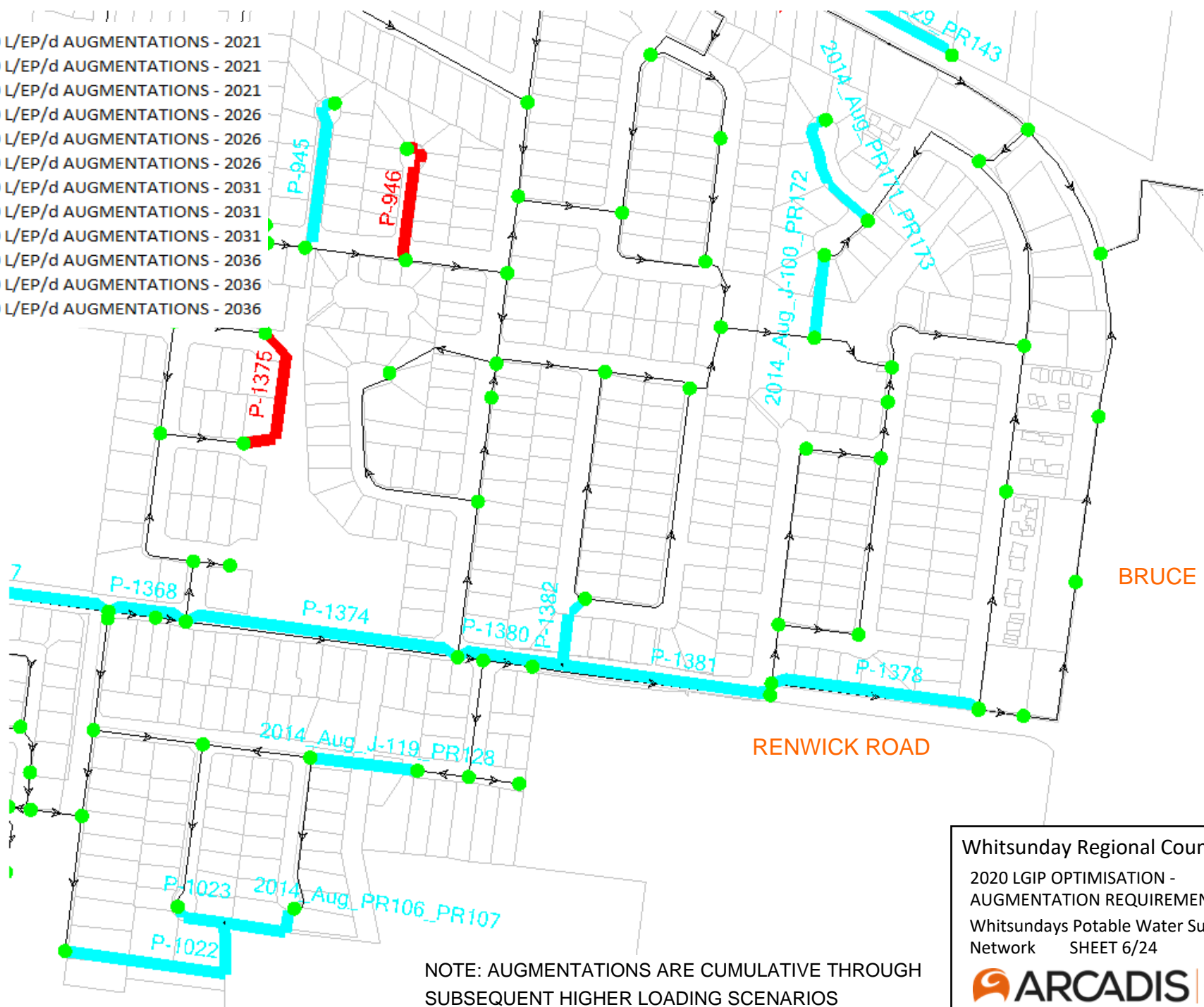
Whitsunday Regional Council
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 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
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Design & Consultancy
for natural and
built assets


PD DEMAND

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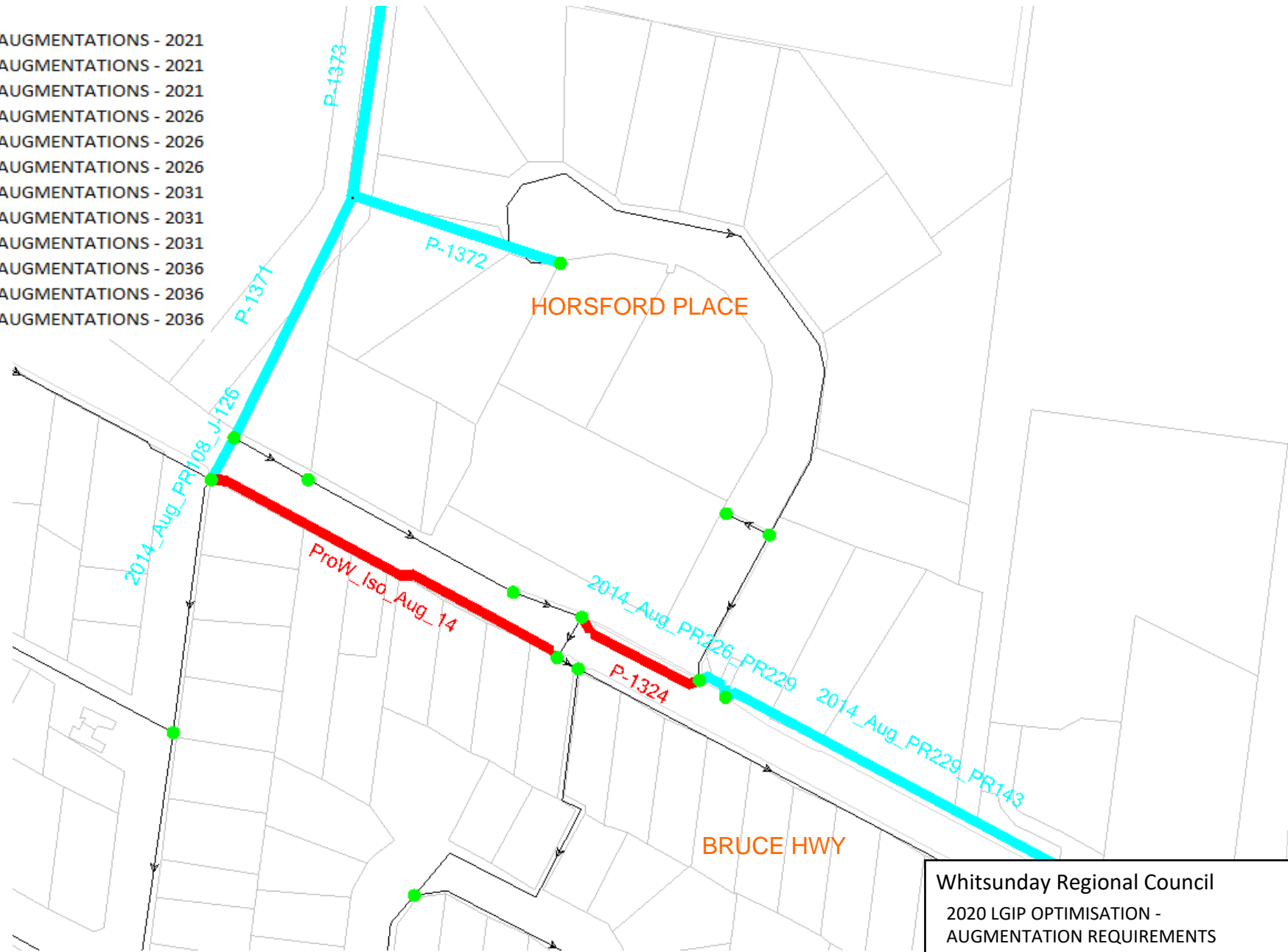
Whitsunday Regional Council
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 6/24



Design & Consultancy for natural and built assets

PD DEMAND

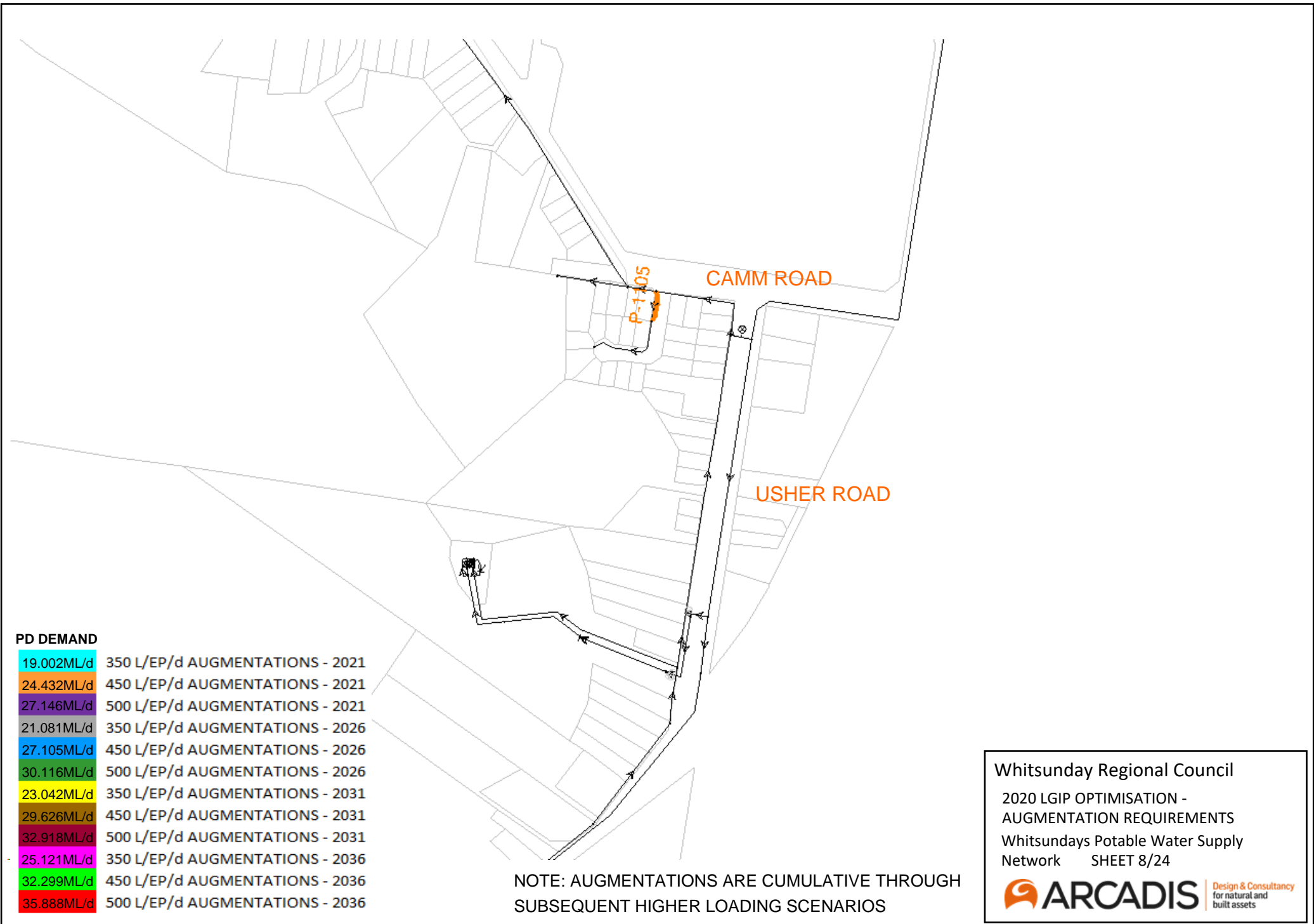
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 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
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PD DEMAND

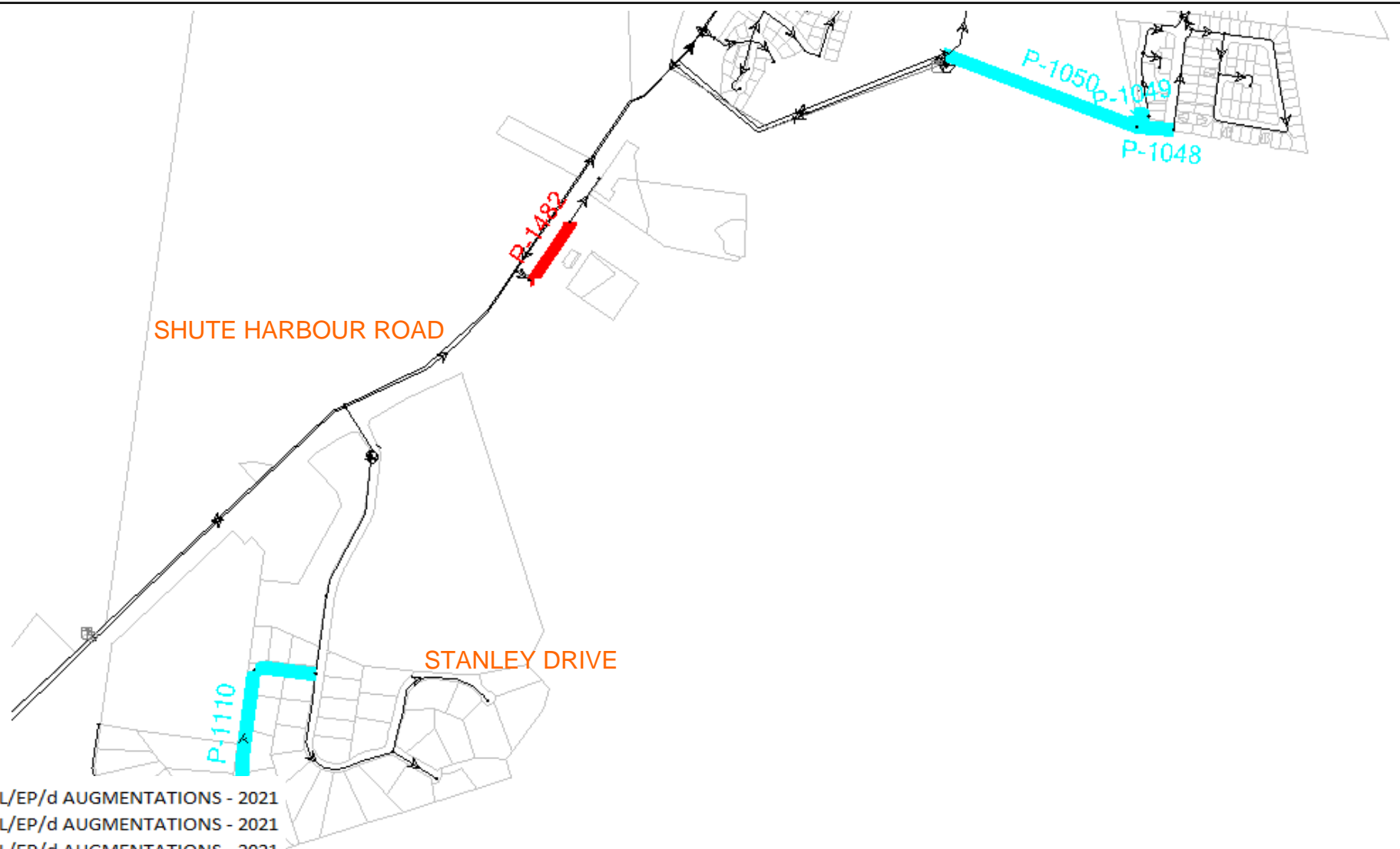
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 2020 LGIP OPTIMISATION -
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 Whitsundays Potable Water Supply
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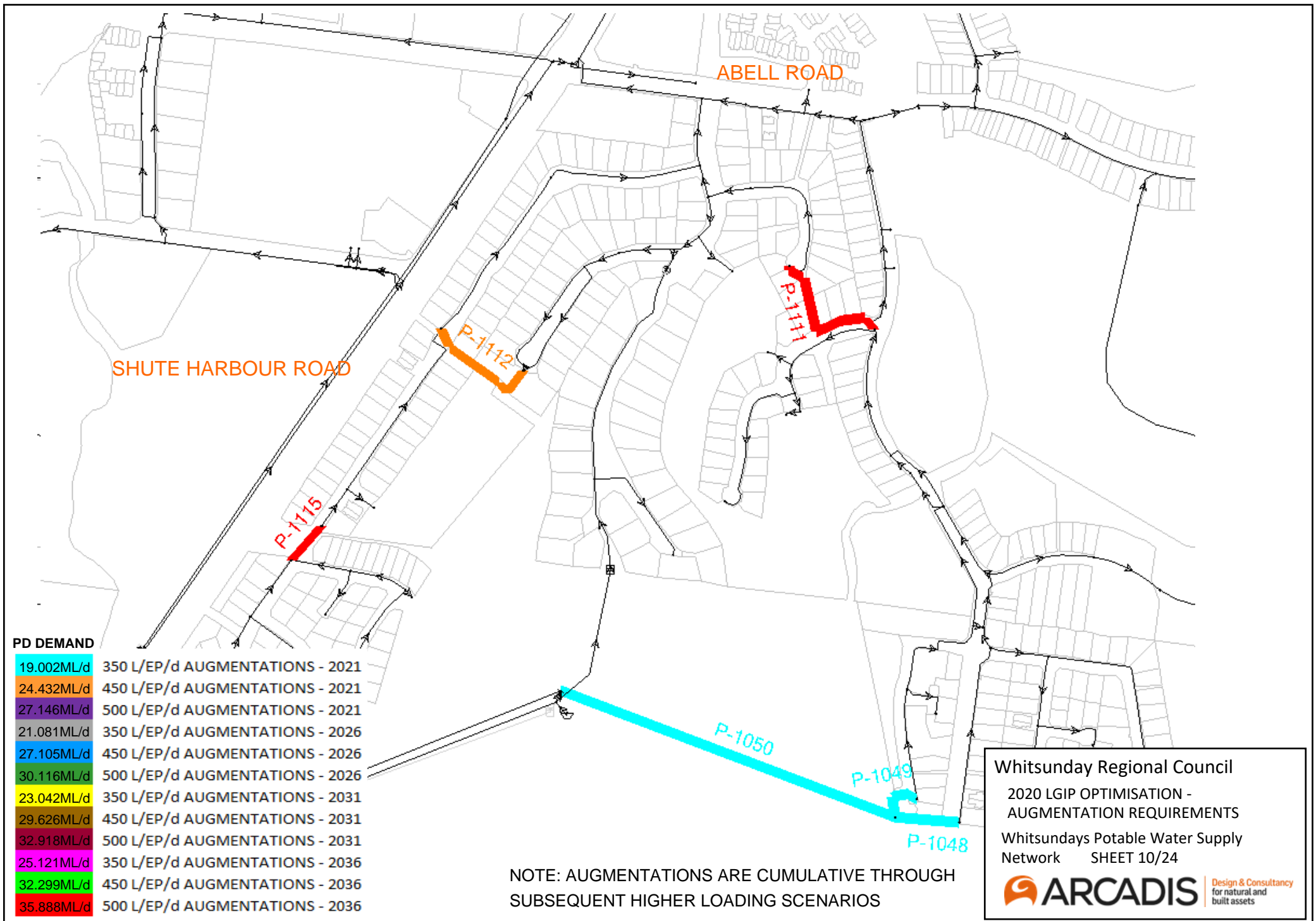
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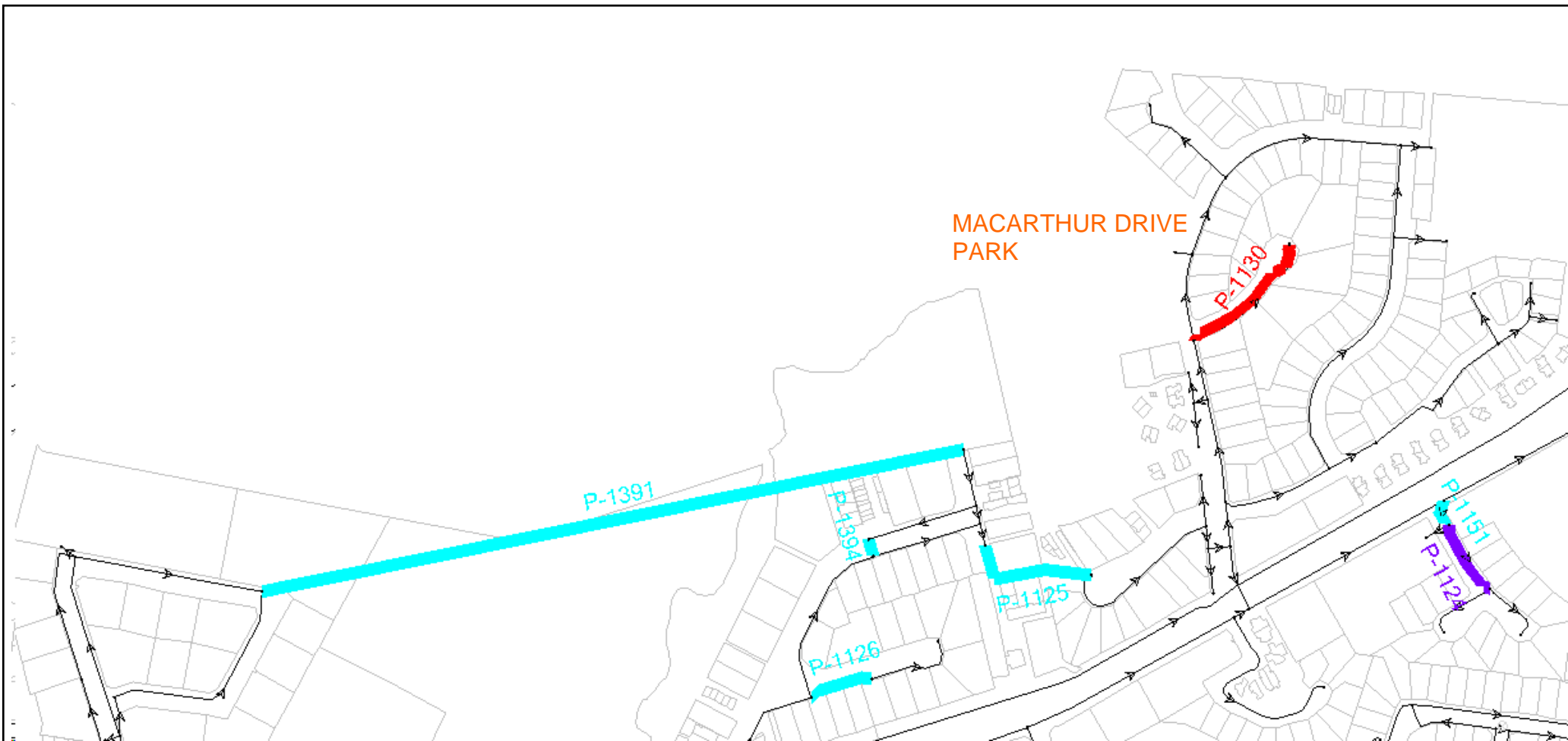
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Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
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PD DEMAND

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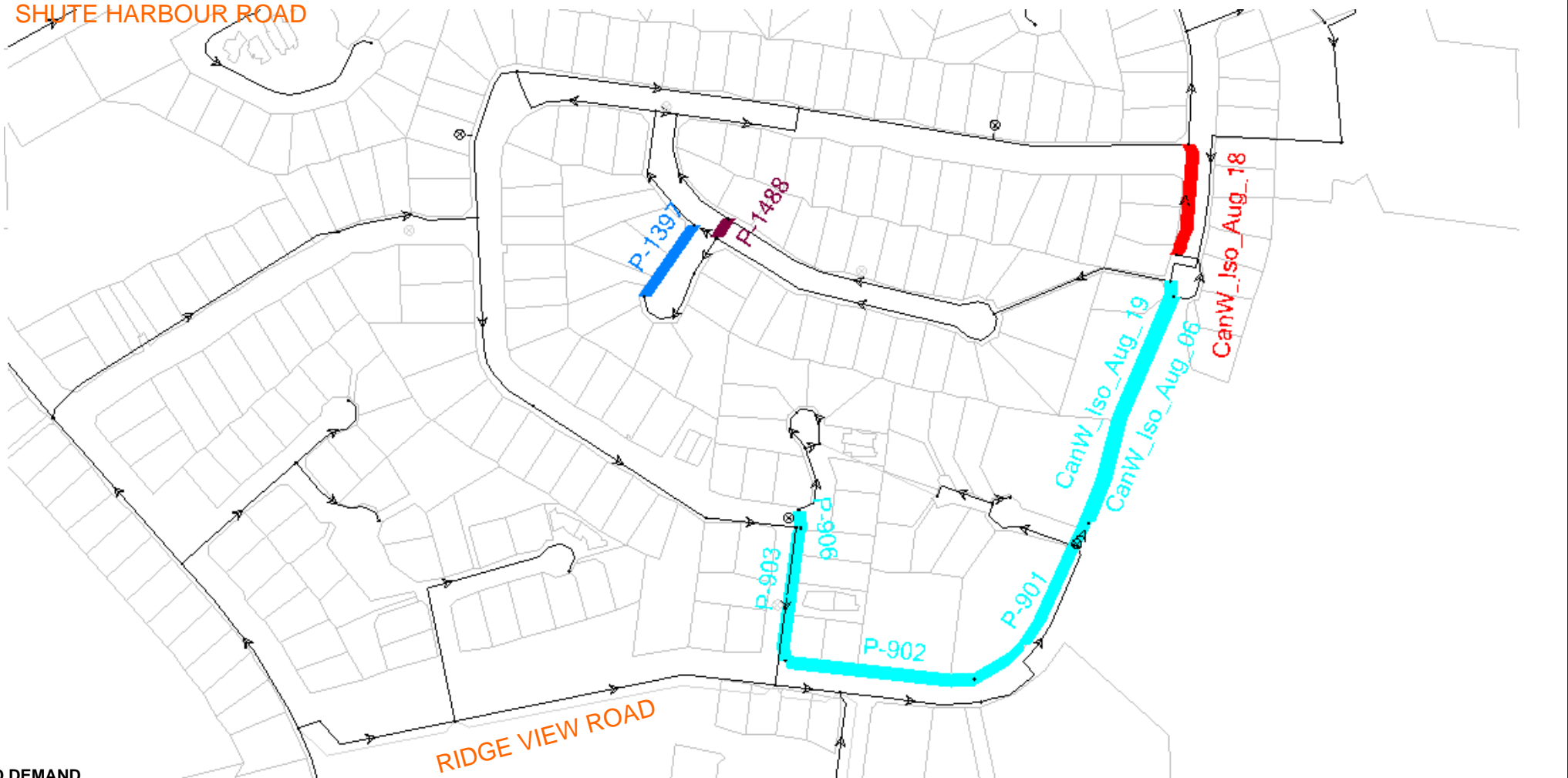
SHUTE HARBOUR ROAD

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
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 Network SHEET 11/24



SHUTE HARBOUR ROAD




RIDGE VIEW ROAD

PD DEMAND

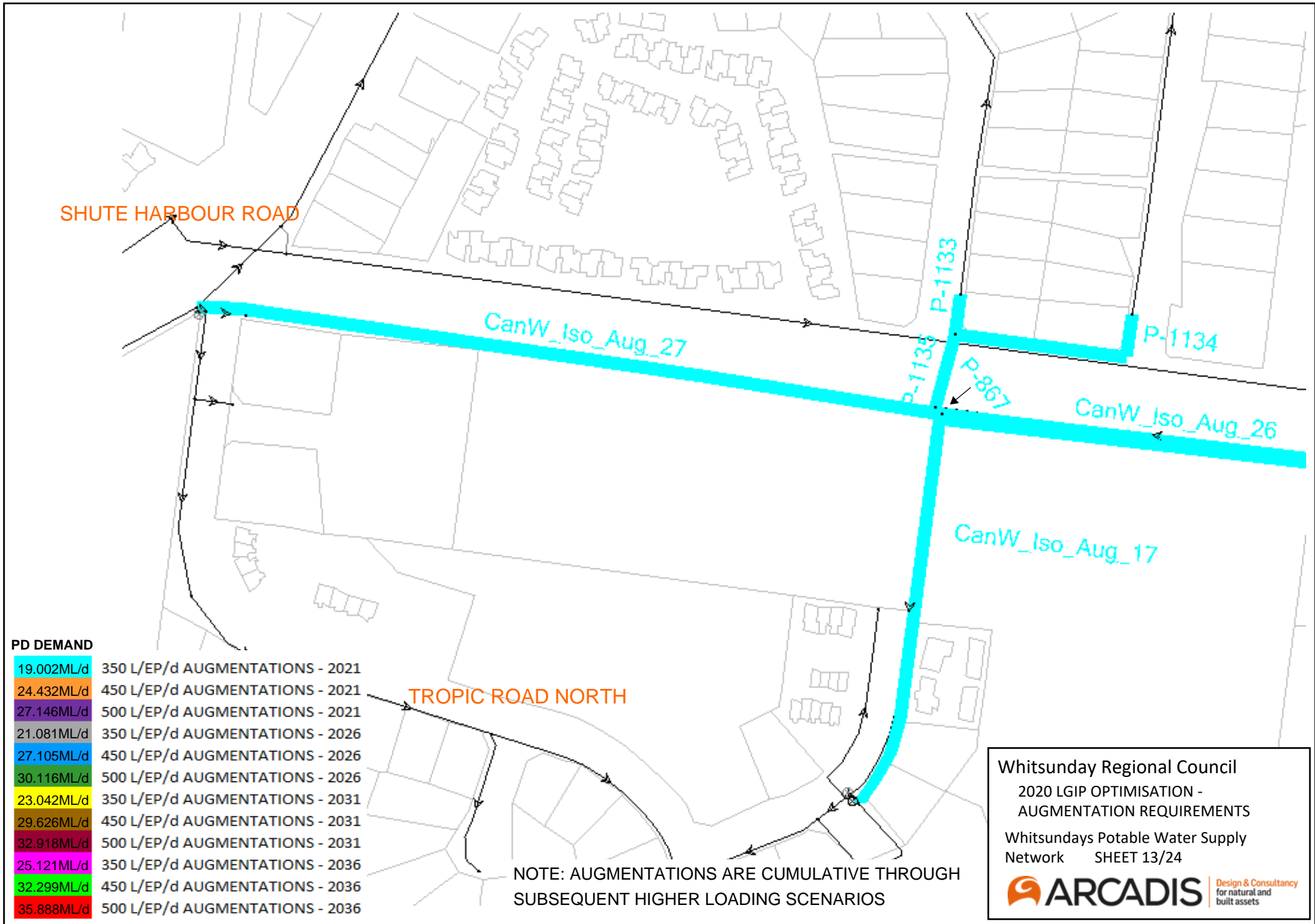
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
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Design & Consultancy
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built assets



PD DEMAND

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| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 13/24



COYNE ROAD HL PUMP
STATION - Fed via connection to
Trunk Supply Line

P-1487
CanW76a
CanW377b
CanW76b

P-1486

CanW76

P-1076

PD DEMAND

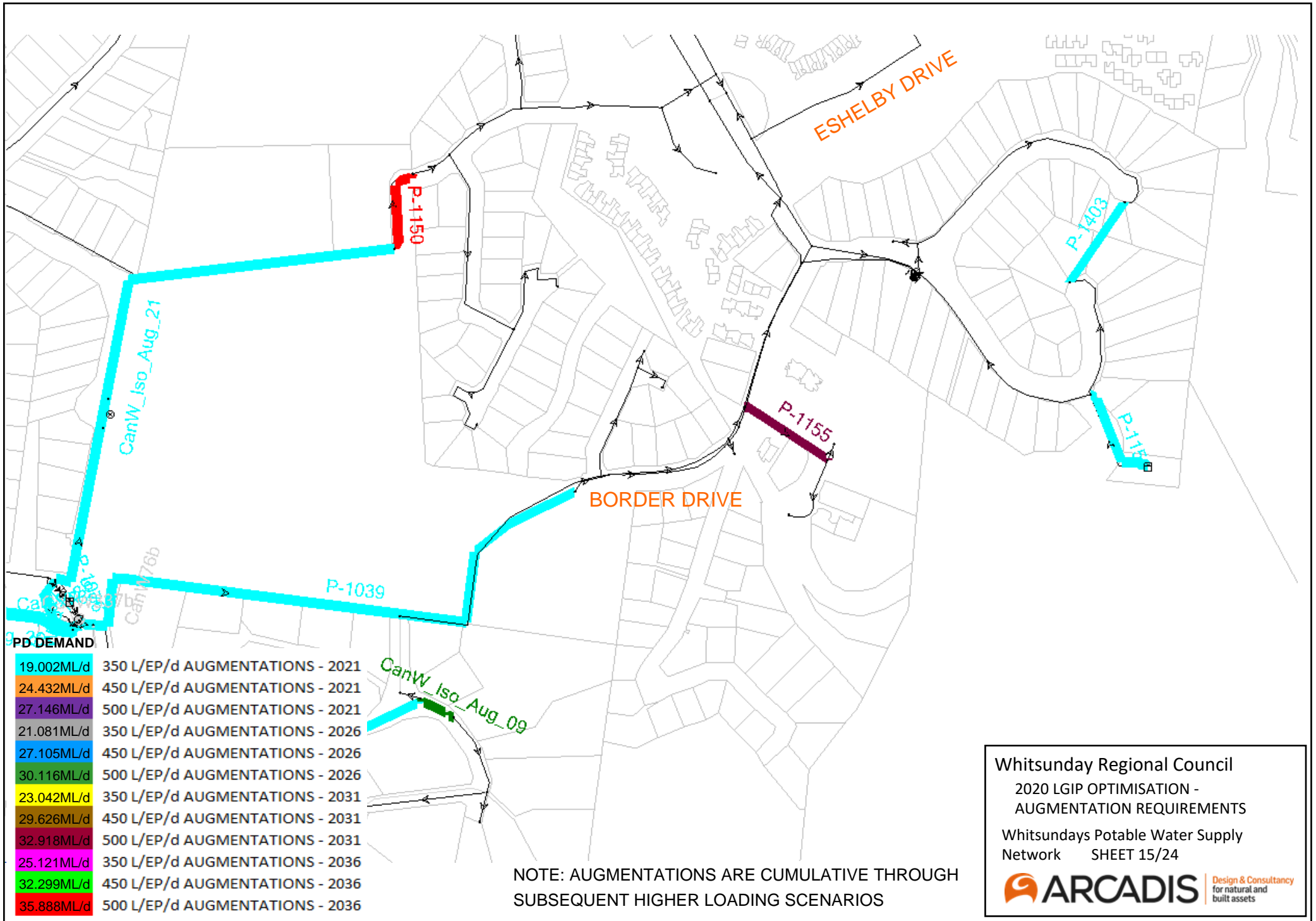
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Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

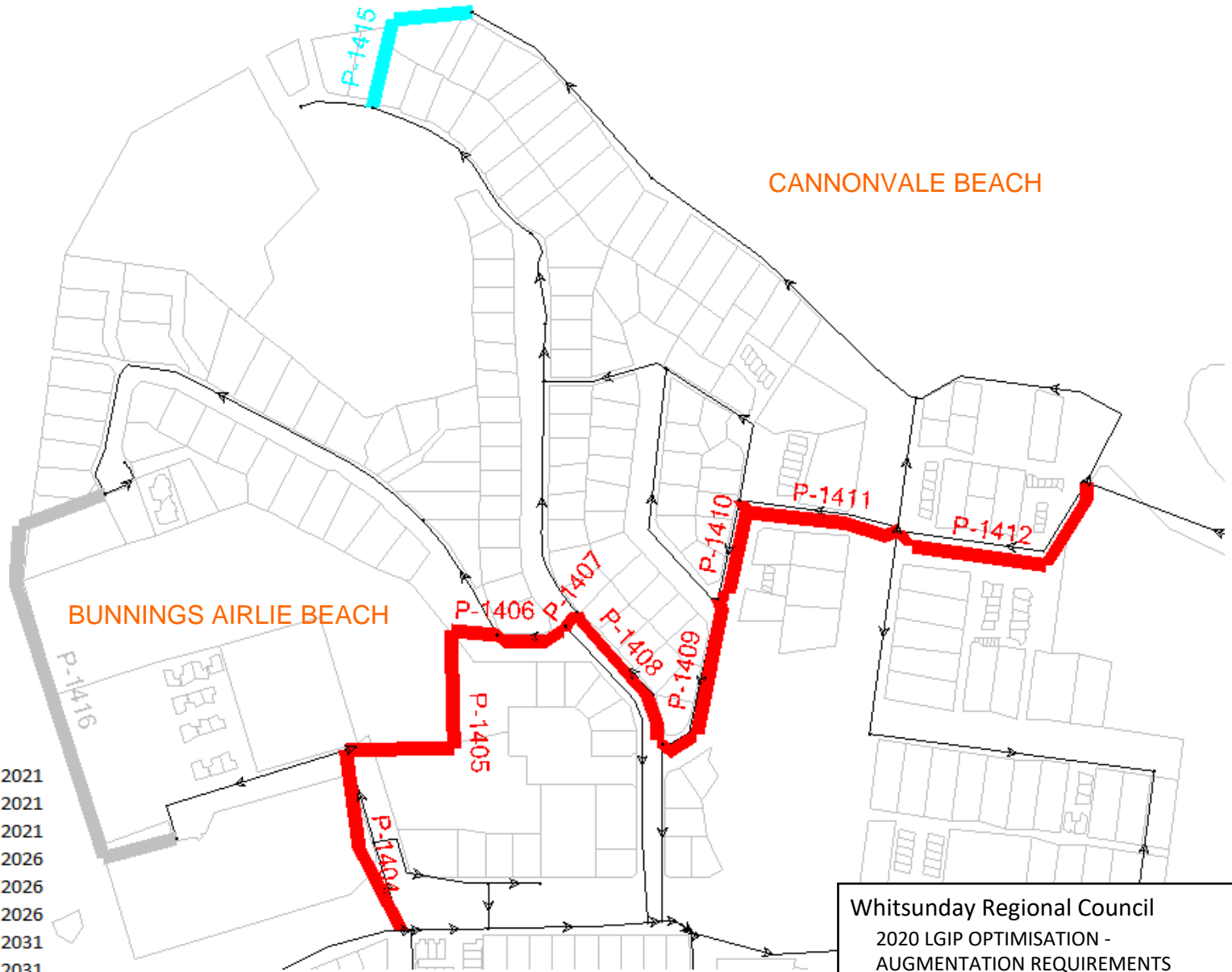
Whitsundays Potable Water Supply
Network SHEET 14/24





Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 15/24

ARCADIS Design & Consultancy
 for natural and
 built assets



PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

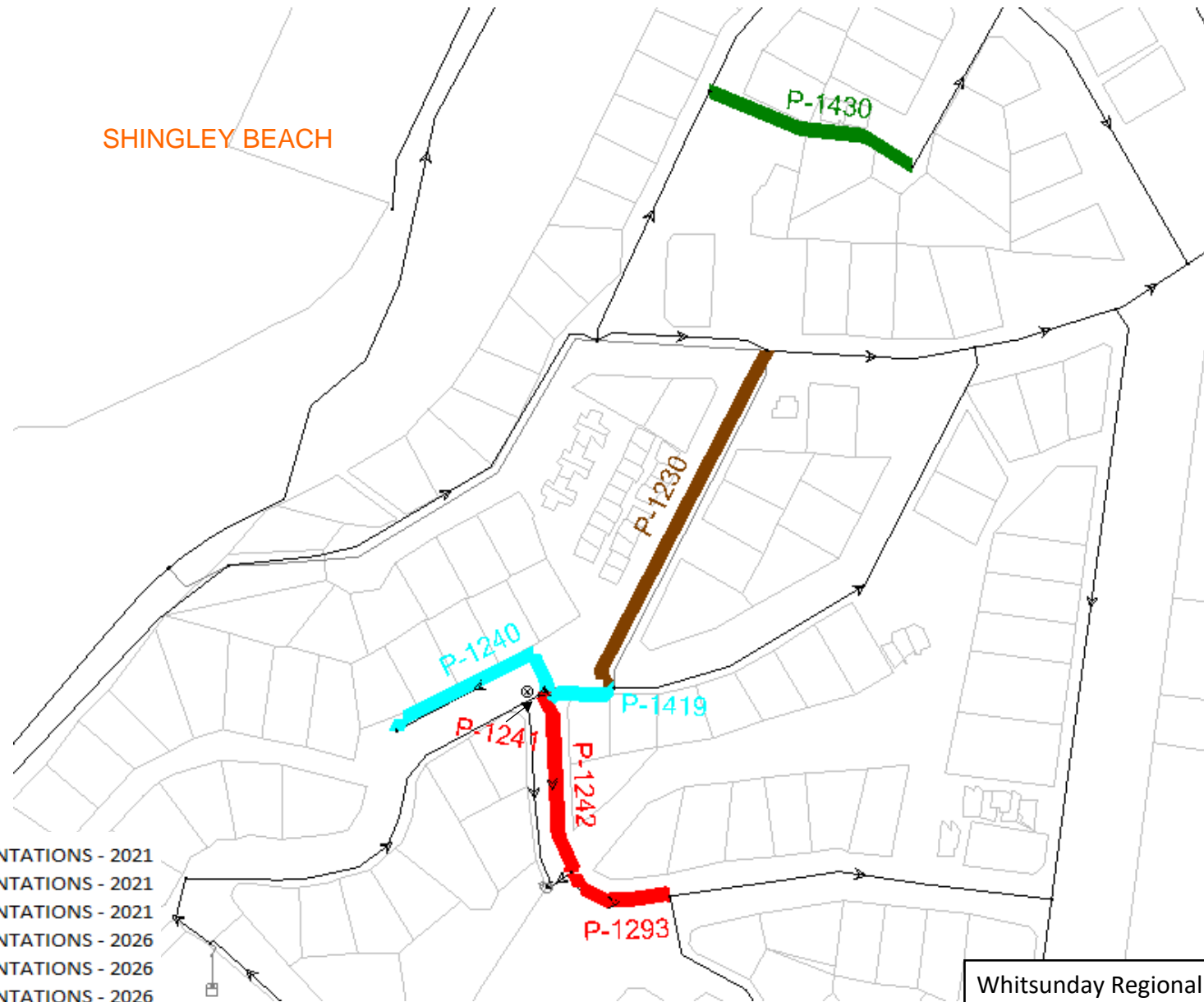
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
 Network SHEET 16/24



SHINGLEY BEACH



PD DEMAND

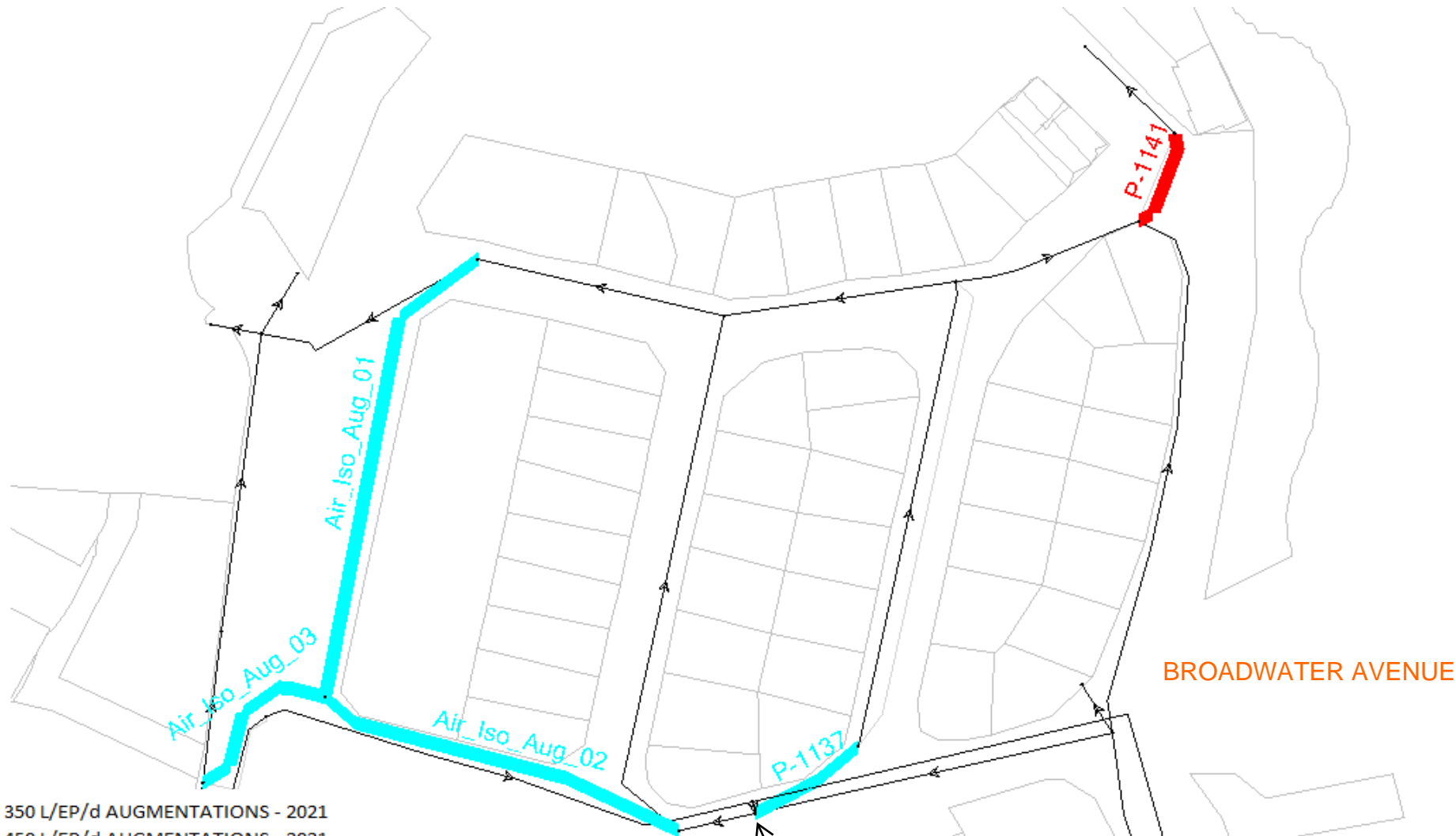
| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
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| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 17/24





PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
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| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

SHUTE HARBOUR ROAD

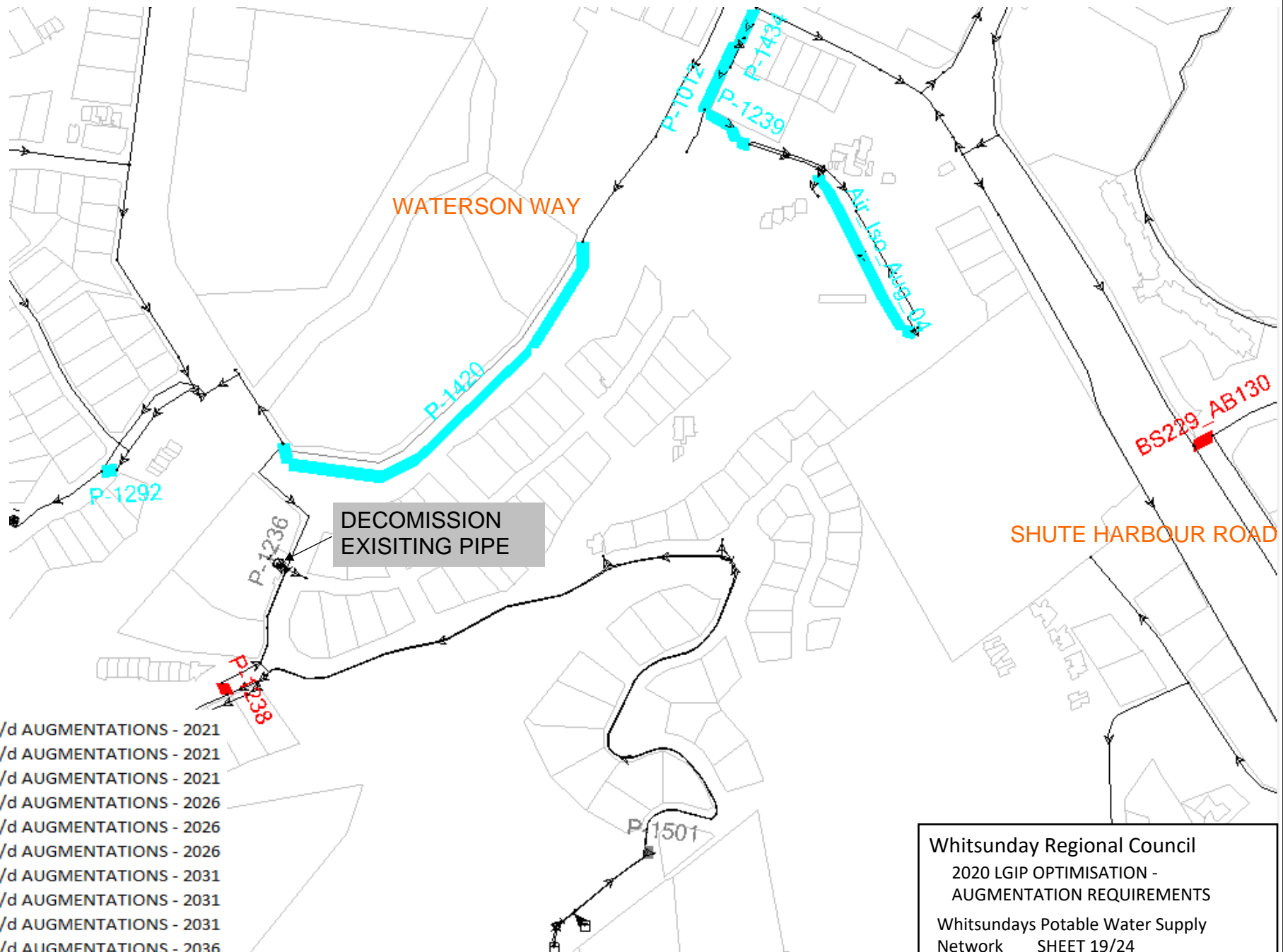
MAINTAIN
CROSS-CONNECTION TO
TRUNK SUPPLY

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 18/24






PD DEMAND

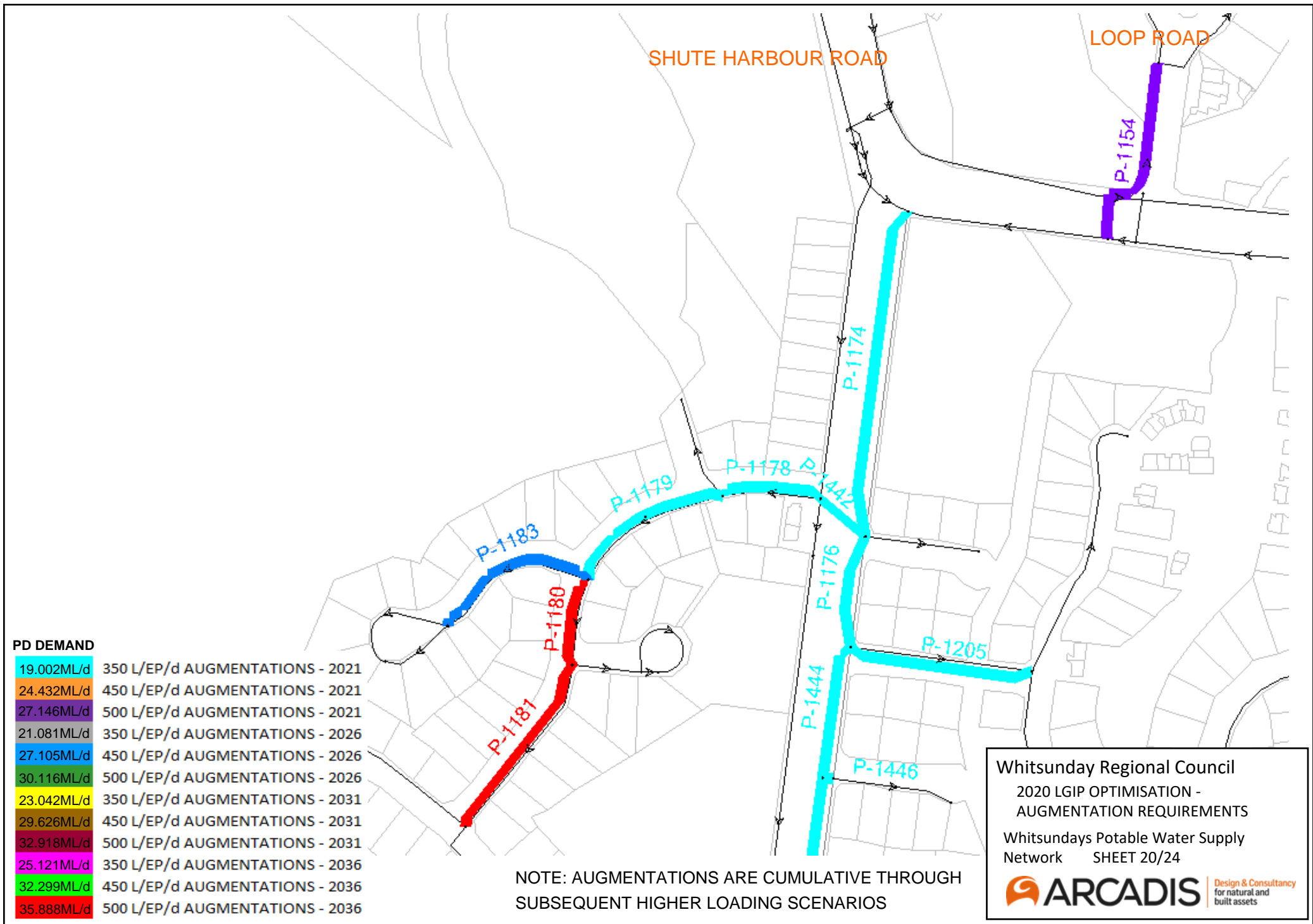
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|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
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| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 19/24



Design & Consultancy
for natural and built assets




PD DEMAND

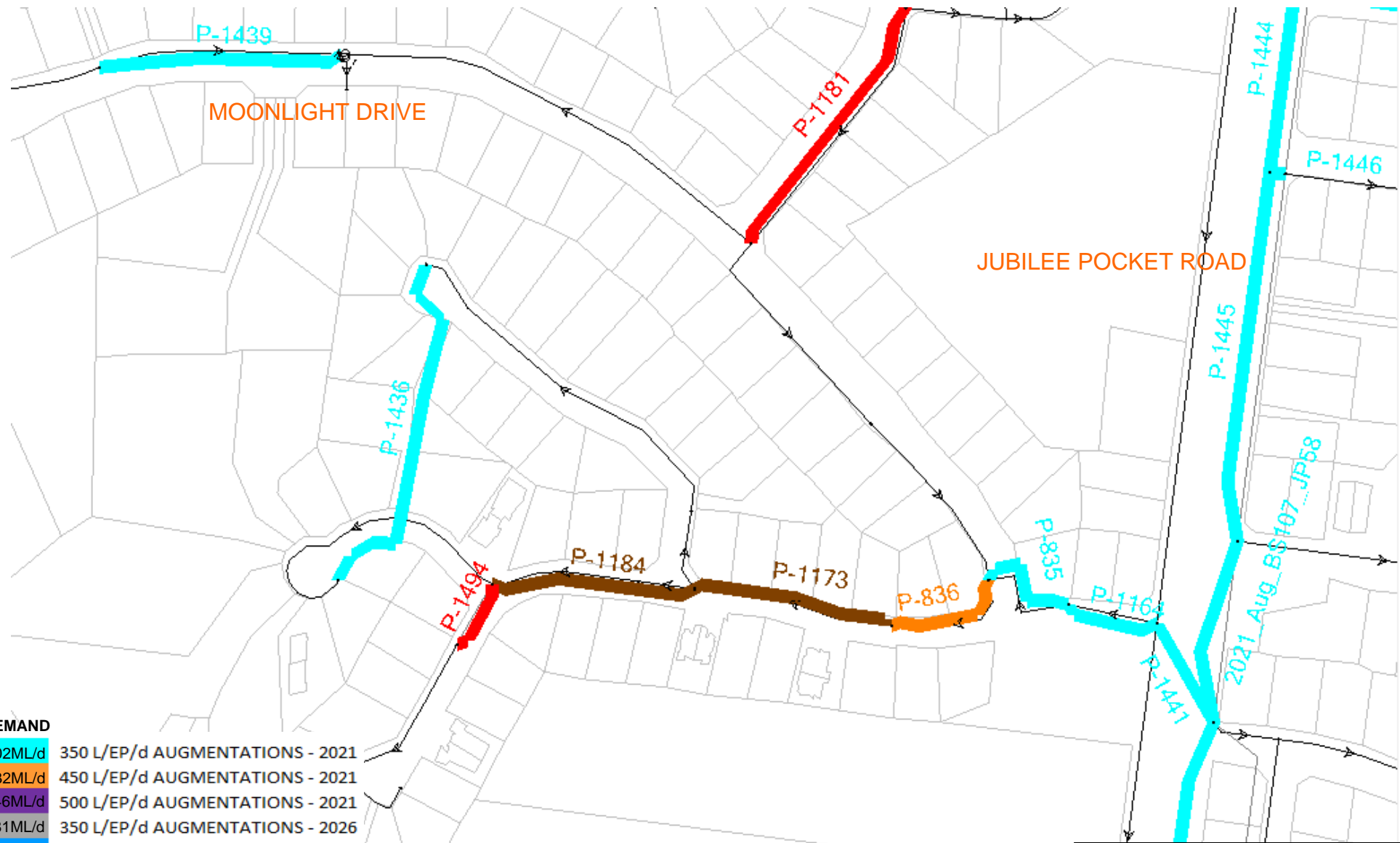
| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
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| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 20/24



Design & Consultancy for natural and built assets



PD DEMAND

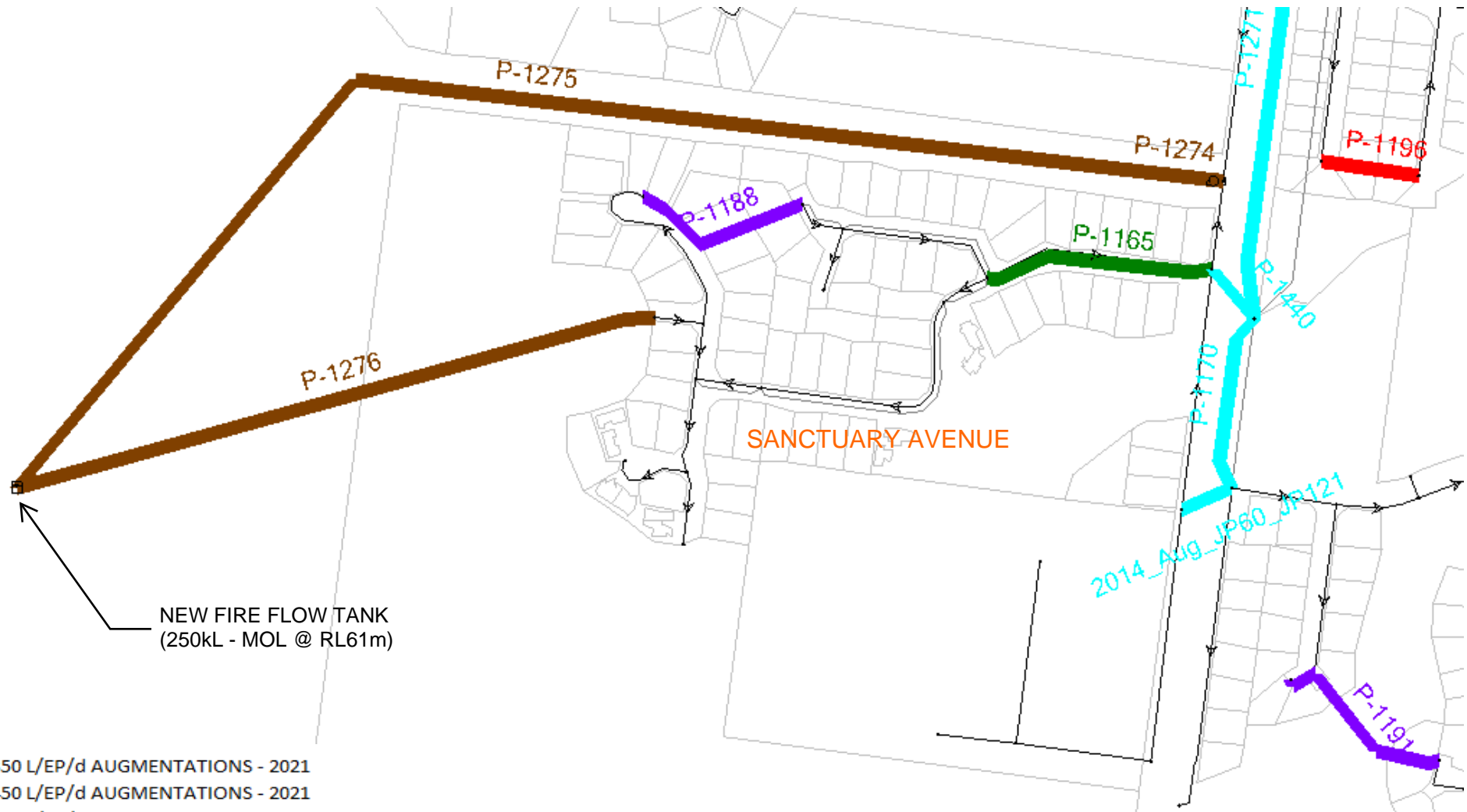
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|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 21/24





NEW FIRE FLOW TANK
(250kL - MOL @ RL61m)

PD DEMAND

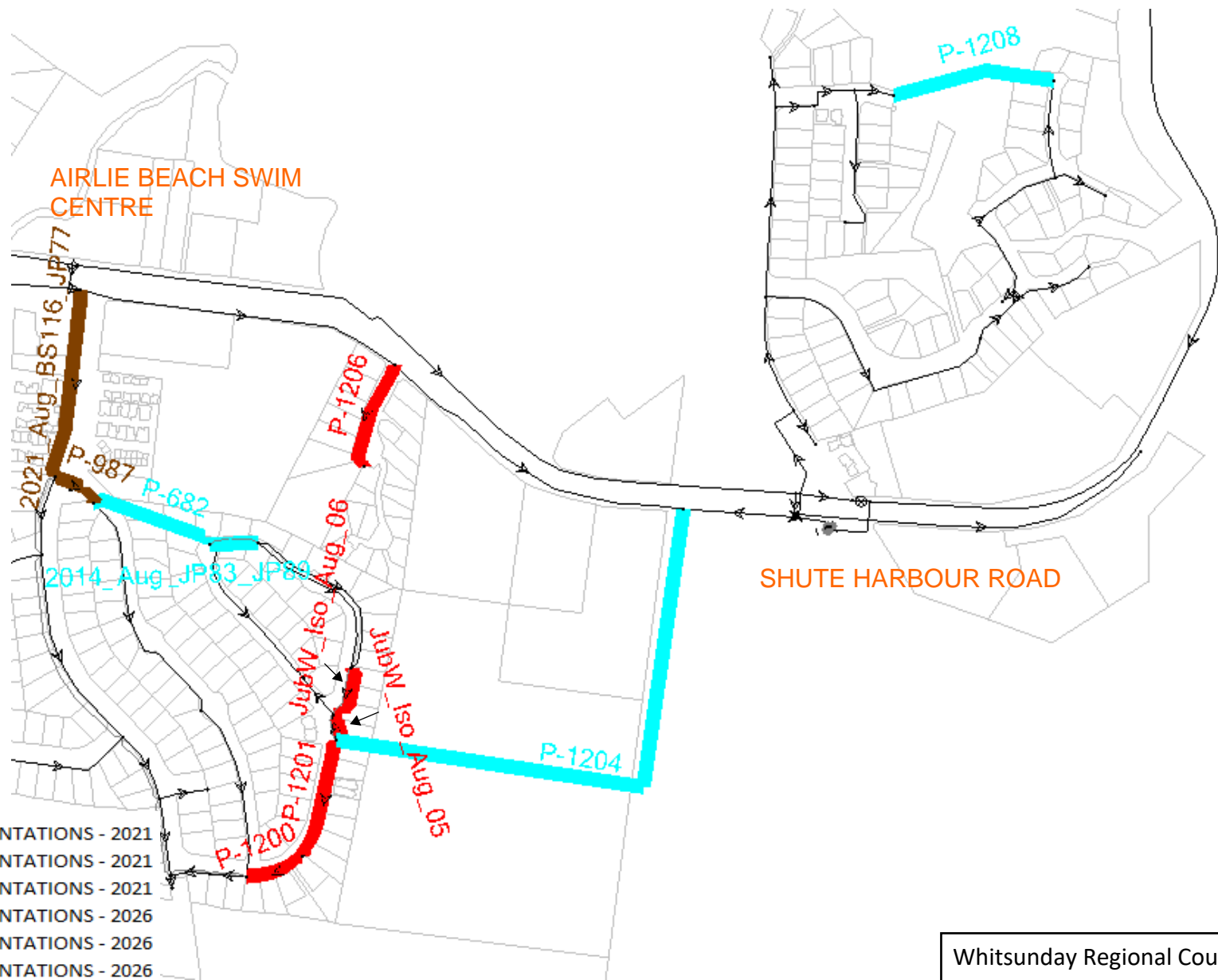
| | |
|------------|---------------------------------|
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 22/24






PD DEMAND

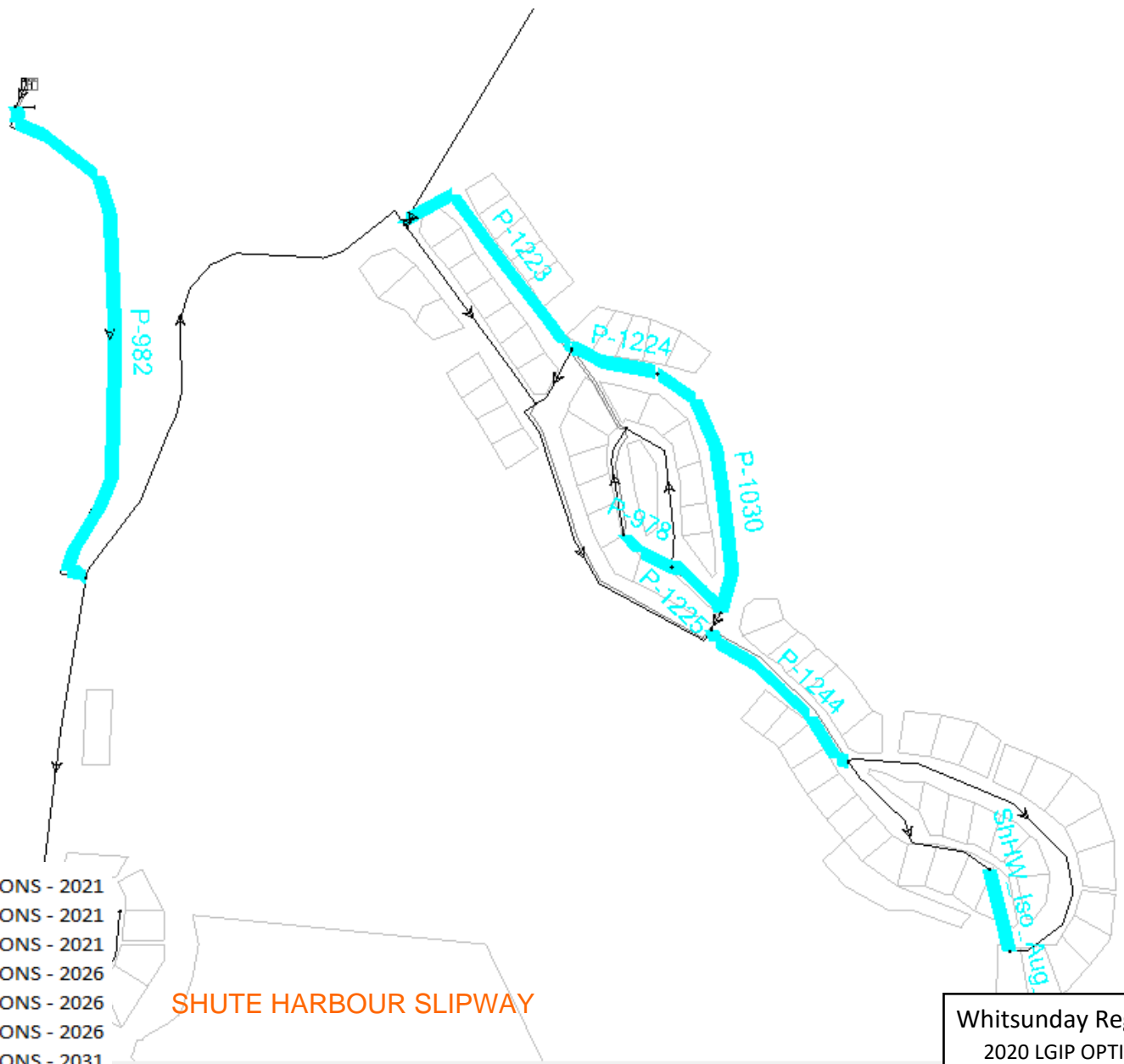
| | |
|------------|---------------------------------|
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 23/24



Design & Consultancy
for natural and
built assets




PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
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SHUTE HARBOUR SLIPWAY

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 24/24



Design & Consultancy
for natural and built assets

PROJECT: Whitsundays Potable Water Network Modelling
 DOCUMENT NUMBER: D005-10027536-AAC-04
 DATE: 25.02.2020

Project Engineer: M.C.
 Software: WaterCad v8i



**WHITSUNDAYS POTABLE WATER NETWORK
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENT SUMMARY - DATA**

| AUGMENTATION ID | LENGTH (m) | EXISTING PIPE SEGMENT | | ID (mm) | COST - \$/m | ADJUSTMENT FACTOR FOR SOIL | 10% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW/DUPLICATION | NON-LGIP AUGMENTATIONS | LGIP AUGMENTATIONS |
|----------------------|------------|-----------------------|----------|---------|-------------|----------------------------|-------------------------------|-----------------|-------------------------|------------------------|--------------------|
| | | START NODE | END NODE | | | | | | | | |
| 2014_Aug_J100_PR172 | 77.3 | J100 | PR172 | 160 | 262 | 1.26 | 10% | 30% | \$ 35,725.59 | \$ | \$ 35,725.59 |
| 2014_Aug_J119_PR126 | 99.98 | J119 | PR126 | 150 | 262 | 1.26 | 10% | 30% | \$ 46,207.56 | \$ | \$ 46,207.56 |
| 2014_Aug_JP60_JP121 | 38.82 | JP60 | JP121 | 313 | 459 | 1.26 | 10% | 30% | \$ 31,431.62 | \$ | \$ 31,431.62 |
| 2014_Aug_JP83_JP89 | 52.57 | JP83 | JP89 | 157 | 262 | 1.26 | 10% | 30% | \$ 24,296.17 | \$ | \$ 24,296.17 |
| 2014_Aug_PR106_PR107 | 82.95 | J-216 | PR107 | 160 | 262 | 1.26 | 10% | 30% | \$ 38,336.84 | \$ | \$ 38,336.84 |
| 2014_Aug_PR108_J-126 | 19.5 | PR108 | J-126 | 212 | 310 | 1.26 | 10% | 30% | \$ 10,663.38 | \$ | \$ 10,663.38 |
| 2014_Aug_PR171_PR173 | 120.09 | PR171 | PR173 | 160 | 262 | 1.26 | 10% | 30% | \$ 55,501.76 | \$ | \$ 55,501.76 |
| 2014_Aug_PR226_PR229 | 16.41 | PR226 | PR229 | 160 | 262 | 1.26 | 10% | 30% | \$ 7,584.18 | \$ | \$ 7,584.18 |
| 2014_Aug_PR229_PR143 | 175.58 | PR229 | PR143 | 160 | 262 | 1.26 | 10% | 30% | \$ 81,147.46 | \$ | \$ 81,147.46 |
| 2014_Aug_PR242_PR237 | 104.53 | PR242 | PR237 | 100 | 205 | 1.26 | 10% | 30% | \$ 37,800.14 | \$ | \$ 37,800.14 |
| 2014_Aug_PR244_PR243 | 51.04 | PR244 | PR243 | 100 | 205 | 1.26 | 10% | 30% | \$ 16,457.08 | \$ | \$ 16,457.08 |
| 2014_Aug_PR244_PR25 | 104.98 | PR244 | PR25 | 160 | 262 | 1.26 | 10% | 30% | \$ 48,518.40 | \$ | \$ 48,518.40 |
| 2014_Aug_PR25_PR27 | 81.6 | PR25 | J-315 | 160 | 262 | 1.26 | 10% | 30% | \$ 37,712.91 | \$ | \$ 37,712.91 |
| 2014_Aug_PR54_J-116 | 118.46 | PR54 | J-116 | 150 | 262 | 1.26 | 10% | 30% | \$ 54,748.42 | \$ | \$ 54,748.42 |
| 2021_Aug_BS107_JP58 | 92.75 | JP17 | JP70 | 313 | 459 | 1.26 | 10% | 30% | \$ 75,097.45 | \$ | \$ 75,097.45 |
| 2021_Aug_BS116_JP77 | 208.4 | BS116 | JP77 | 212 | 310 | 1.26 | 10% | 30% | \$ 113,961.46 | \$ | \$ 113,961.46 |
| Air_Iso_Aug_01 | 155.69 | AB86 | J-176 | 102 | 205 | 1.26 | 10% | 30% | \$ 56,300.62 | \$ | \$ 56,300.62 |
| Air_Iso_Aug_02 | 122.69 | J-176 | AB5 | 102 | 205 | 1.26 | 10% | 30% | \$ 44,367.16 | \$ | \$ 44,367.16 |
| Air_Iso_Aug_03 | 57.29 | J-176 | AB33 | 102 | 205 | 1.26 | 10% | 30% | \$ 20,717.21 | \$ | \$ 20,717.21 |
| Air_Iso_Aug_04 | 154.31 | AB97 | AB68 | 200 | 310 | 1.26 | 10% | 30% | \$ 84,382.88 | \$ | \$ 84,382.88 |
| BS229_AB130 | 17.48 | BS229 | BS229 | 200 | 310 | 1.26 | 10% | 30% | \$ 9,558.76 | \$ | \$ 9,558.76 |
| CanW76 | 2.8 | BS200 | BS195 | 175.6 | 272 | 1.26 | 10% | 30% | \$ 1,343.46 | \$ | \$ 1,343.46 |
| CanW76a | 0.47 | WCPS03/P2 | BS200 | 175.6 | 272 | 1.26 | 10% | 30% | \$ 225.51 | \$ | \$ 225.51 |
| CanW76b | 0.42 | WCPS03/P1 | BS200 | 175.6 | 272 | 1.26 | 10% | 30% | \$ 201.52 | \$ | \$ 201.52 |
| CanW337b | 0.51 | WCPS03/P1 | J-322 | 157 | 262 | 1.26 | 10% | 30% | \$ 235.71 | \$ | \$ 235.71 |
| CanW_Iso_Aug_06 | 159.11 | CA221 | CA276 | 157 | 262 | 1.26 | 10% | 30% | \$ 73,535.55 | \$ | \$ 73,535.55 |
| CanW_Iso_Aug_09 | 33.02 | J-168 | J-319 | 160 | 262 | 1.26 | 10% | 30% | \$ 15,260.79 | \$ | \$ 15,260.79 |
| CanW_Iso_Aug_17 | 427.54 | J-188 | J-186 | 160 | 384 | 1.26 | 10% | 30% | \$ 289,605.34 | \$ | \$ 289,605.34 |
| CanW_Iso_Aug_18 | 74.77 | CA51 | CA52 | 260 | 262 | 1.26 | 10% | 30% | \$ 34,556.30 | \$ | \$ 34,556.30 |
| CanW_Iso_Aug_19 | 185.49 | J-191 | J-195 | 157 | 262 | 1.26 | 10% | 30% | \$ 85,727.54 | \$ | \$ 85,727.54 |
| CanW_Iso_Aug_21 | 561 | CA243 | CA189 | 250 | 384 | 1.26 | 10% | 30% | \$ 380,007.94 | \$ | \$ 380,007.94 |
| CanW_Iso_Aug_26 | 219.43 | CA243 | J-192 | 300 | 459 | 1.26 | 10% | 30% | \$ 177,667.20 | \$ | \$ 177,667.20 |
| CanW_Iso_Aug_27 | 373.92 | J-192 | BS66 | 300 | 459 | 1.26 | 10% | 30% | \$ 302,754.05 | \$ | \$ 302,754.05 |
| JubW_Iso_Aug_05 | 32.62 | J-199 | JP80 | 200 | 310 | 1.26 | 10% | 30% | \$ 17,837.92 | \$ | \$ 17,837.92 |
| JubW_Iso_Aug_06 | 64.79 | JP80 | JP82 | 200 | 310 | 1.26 | 10% | 30% | \$ 35,429.76 | \$ | \$ 35,429.76 |
| P-682 | 139.44 | JP33 | JP83 | 157 | 262 | 1.26 | 10% | 30% | \$ 64,444.71 | \$ | \$ 64,444.71 |
| P-835 | 57.87 | JP69 | JP67 | 210 | 310 | 1.26 | 10% | 30% | \$ 31,645.63 | \$ | \$ 31,645.63 |
| P-836 | 62.46 | JP67 | JP68 | 210 | 310 | 1.26 | 10% | 30% | \$ 34,155.63 | \$ | \$ 34,155.63 |
| P-867 | 4.67 | J-192 | J-193 | 250 | 384 | 1.26 | 10% | 30% | \$ 3,163.35 | \$ | \$ 3,163.35 |
| P-901 | 112.89 | J-203 | J-208 | 157 | 262 | 1.26 | 10% | 30% | \$ 52,174.15 | \$ | \$ 52,174.15 |
| P-902 | 124.94 | J-203 | J-204 | 157 | 262 | 1.26 | 10% | 30% | \$ 57,743.27 | \$ | \$ 57,743.27 |
| P-903 | 88.03 | J-204 | J-205 | 157 | 262 | 1.26 | 10% | 30% | \$ 40,684.65 | \$ | \$ 40,684.65 |
| P-906 | 11.82 | J-205 | J-206 | 157 | 262 | 1.26 | 10% | 30% | \$ 5,462.83 | \$ | \$ 5,462.83 |
| P-945 | 147.96 | PR112 | PR113 | 150 | 262 | 1.26 | 10% | 30% | \$ 68,382.38 | \$ | \$ 68,382.38 |
| P-946 | 118.5 | PR114 | pr115 | 150 | 262 | 1.26 | 10% | 30% | \$ 54,766.91 | \$ | \$ 54,766.91 |
| P-978 | 42.58 | SH13 | SH10 | 212 | 310 | 1.26 | 10% | 30% | \$ 23,284.45 | \$ | \$ 23,284.45 |
| P-982 | 388.7 | BS129 | BS128 | 300 | 459 | 1.26 | 10% | 30% | \$ 314,721.06 | \$ | \$ 314,721.06 |
| P-987 | 55.85 | JP77 | JP33 | 157 | 262 | 1.26 | 10% | 30% | \$ 25,812.08 | \$ | \$ 25,812.08 |
| P-1012 | 65.45 | AB94 | AB62 | 150 | 262 | 1.26 | 10% | 30% | \$ 30,248.90 | \$ | \$ 30,248.90 |
| P-1022 | 197.91 | PR104 | J-216 | 160 | 262 | 1.26 | 10% | 30% | \$ 91,467.67 | \$ | \$ 91,467.67 |
| P-1023 | 49.62 | J-216 | PR106 | 160 | 262 | 1.26 | 10% | 30% | \$ 22,927.78 | \$ | \$ 22,927.78 |
| P-1030 | 190.25 | J-216 | J-218 | 212 | 310 | 1.26 | 10% | 30% | \$ 104,036.31 | \$ | \$ 104,036.31 |
| P-1039 | 530.92 | BS201 | CA196 | 200 | 310 | 1.26 | 10% | 30% | \$ 290,328.29 | \$ | \$ 290,328.29 |
| P-1045 | 318.43 | BS287 | WPWT | 386.9 | 878 | 1.26 | 10% | 30% | \$ 493,181.84 | \$ | \$ 493,181.84 |
| P-1048 | 82.77 | J-223 | CA359 | 200 | 310 | 1.26 | 10% | 30% | \$ 45,261.95 | \$ | \$ 45,261.95 |
| P-1049 | 60.75 | J-223 | CA356 | 200 | 310 | 1.26 | 10% | 30% | \$ 33,220.53 | \$ | \$ 33,220.53 |
| P-1050 | 457.74 | PRV-8 | J-223 | 300 | 459 | 1.26 | 10% | 30% | \$ 370,621.09 | \$ | \$ 370,621.09 |
| P-1059 | 23.36 | WPWT | J-227 | 523 | 1278 | 1.26 | 10% | 30% | \$ 52,662.60 | \$ | \$ 52,662.60 |
| P-1060 | 61.02 | J-227 | PR1 | 523 | 1278 | 1.26 | 10% | 30% | \$ 137,563.00 | \$ | \$ 137,563.00 |
| P-1063 | 578.87 | J-224 | PR3 | 250 | 384 | 1.26 | 10% | 30% | \$ 391,570.74 | \$ | \$ 391,570.74 |
| P-1066 | 316.48 | J-227 | BS329 | 523 | 1278 | 1.26 | 10% | 30% | \$ 713,409.98 | \$ | \$ 713,409.98 |
| P-1069 | 279 | BS329 | PR35 | 375 | 878 | 1.26 | 10% | 30% | \$ 432,128.46 | \$ | \$ 432,128.46 |
| P-1072 | 266.01 | PR48 | PR208 | 375 | 878 | 1.26 | 10% | 30% | \$ 411,994.16 | \$ | \$ 411,994.16 |
| P-1076 | 1.51 | BS195 | J-188 | 250 | 384 | 1.26 | 10% | 30% | \$ 1,022.84 | \$ | \$ 1,022.84 |
| P-1088 | 13.19 | J-226 | PR1 | 523 | 1278 | 1.26 | 10% | 30% | \$ 29,735.43 | \$ | \$ 29,735.43 |
| P-1090 | 211.45 | BS196 | J-168 | 160 | 262 | 1.26 | 10% | 30% | \$ 97,725.42 | \$ | \$ 97,725.42 |
| P-1092 | 37.15 | PR18 | J-312 | 160 | 262 | 1.26 | 10% | 30% | \$ 17,169.54 | \$ | \$ 17,169.54 |
| P-1094 | 34.23 | PR25 | J-316 | 160 | 262 | 1.26 | 10% | 30% | \$ 15,820.01 | \$ | \$ 15,820.01 |
| P-1096 | 41.38 | PR35 | J-309 | 160 | 262 | 1.26 | 10% | 30% | \$ 19,124.51 | \$ | \$ 19,124.51 |
| P-1105 | 40.03 | MJ5 | J-327 | 160 | 262 | 1.26 | 10% | 30% | \$ 18,500.59 | \$ | \$ 18,500.59 |
| P-1110 | 584.37 | CA169 | J-238 | 160 | 262 | 1.26 | 10% | 30% | \$ 270,077.11 | \$ | \$ 270,077.11 |
| P-1111 | 184.41 | CA181 | CA322 | 150 | 262 | 1.26 | 10% | 30% | \$ 85,228.40 | \$ | \$ 85,228.40 |
| P-1112 | 155.83 | CA177 | CA180 | 150 | 262 | 1.26 | 10% | 30% | \$ 72,019.64 | \$ | \$ 72,019.64 |
| P-1115 | 58.58 | J-239 | CA340 | 100 | 205 | 1.26 | 10% | 30% | \$ 21,183.70 | \$ | \$ 21,183.70 |
| P-1124 | 85.43 | CA241 | CA222 | 150 | 262 | 1.26 | 10% | 30% | \$ 39,483.01 | \$ | \$ 39,483.01 |
| P-1125 | 144.67 | CA22 | CA25 | 150 | 262 | 1.26 | 10% | 30% | \$ 66,861.84 | \$ | \$ 66,861.84 |
| P-1126 | 70.8 | CA17 | J-321 | 160 | 262 | 1.26 | 10% | 30% | \$ 32,721.49 | \$ | \$ 32,721.49 |
| P-1130 | 155.38 | CA35 | CA36 | 160 | 262 | 1.26 | 10% | 30% | \$ 71,811.66 | \$ | \$ 71,811.66 |
| P-1133 | 19.66 | CA86 | J-241 | 150 | 262 | 1.26 | 10% | 30% | \$ 9,086.22 | \$ | \$ 9,086.22 |
| P-1134 | 107.5 | J-241 | CA287 | 160 | 262 | 1.26 | 10% | 30% | \$ 49,683.06 | \$ | \$ 49,683.06 |
| P-1135 | 37.61 | J-241 | J-193 | 160 | 262 | 1.26 | 10% | 30% | \$ 17,382.14 | \$ | \$ 17,382.14 |
| P-1137 | 39.85 | AB10 | AB76 | 160 | 262 | 1.26 | 10% | 30% | \$ 18,417.39 | \$ | \$ 18,417.39 |
| P-1141 | 32.75 | AB28 | AB29 | 160 | 262 | 1.26 | 10% | 30% | \$ 15,136.00 | \$ | \$ 15,136.00 |
| P-1149 | 52.17 | CA92 | CA281 | 150 | 262 | 1.26 | 10% | 30% | \$ 24,111.30 | \$ | \$ 24,111.30 |
| P-1150 | 73.62 | CA189 | CA245 | 200 | 310 | 1.26 | 10% | 30% | \$ 40,258.36 | \$ | \$ 40,258.36 |
| P-1151 | 31.27 | BS65 | CA241 | 150 | 262 | 1.26 | 10% | 30% | \$ 14,451.99 | \$ | \$ 14,451.99 |
| P-1154 | 146.01 | BS231 | JP55 | 160 | 262 | 1.26 | 10% | 30% | \$ 67,481.15 | \$ | \$ 67,481.15 |
| P-1155 | 85.07 | CA97 | CA99 | 150 | 262 | 1.26 | 10% | 30% | \$ 39,316.63 | \$ | \$ 39,316.63 |
| P-1156 | 88.21 | WCGR07 | BS84 | 200 | 310 | 1.26 | 10% | 30% | \$ 48,236.76 | \$ | \$ 48,236.76 |
| P-1162 | 349.99 | CA213 | CA402 | 200 | 310 | 1.26 | 10% | 30% | \$ 191,388.53 | \$ | \$ 191,388.53 |
| P-1164 | 48.29 | JP58 | JP69 | 210 | 310 | 1.26 | 10% | 30% | \$ 26,406.90 | \$ | \$ 26,406.90 |
| P-1165 | 165.4 | JP59 | JP61 | 200 | 310 | 1.26 | 10% | 30% | \$ 90,447.34 | \$ | \$ 90,447.34 |
| P-1170 | 128.77 | JP19 | JP121 | 313 | 459 | 1.26 | 10% | 30% | \$ 104,261.98 | \$ | \$ 104,261.98 |
| P-1173 | 100.82 | JP68 | JP84 | 210 | 310 | 1.26 | 10% | 30% | \$ 55,132.41 | \$ | \$ 55,132.41 |
| P-1174 | 239.16 | BS115 | JP10 | 313 | 459 | 1.26 | 10% | 30% | \$ 193,642.11 | \$ | \$ 193,642.11 |
| P-1176 | 82.24 | JP10 | JP12 | 313 | 459 | 1.26 | 10% | 30% | \$ 66,587.75 | \$ | \$ 66,587.75 |
| P-1178 | 75.13 | BS108 | JP133 | 200 | 310 | 1.26 | 10% | 30% | \$ 41,084.09 | \$ | \$ 41,084.09 |
| P-1179 | 123.26 | JP133 | BS109 | 200 | 310 | 1.26 | 10% | 30% | \$ 67,403.50 | \$ | \$ 67,403.50 |
| P-1180 | 65.97 | BS109 | BS110 | 150 | 262 | 1.26 | 10% | 30% | \$ 30,489.22 | \$ | \$ 30,489.22 |
| P-1181 | 141.12 | BS110 | BS111 | 150 | 262 | 1.26 | 10% | 30% | \$ 65,221.15 | \$ | \$ 65,221.15 |
| P-1183 | 125.73 | BS109 | JP6 | 150 | 2 | | | | | | |

REFERENCE

ACID SULFATE SOILS (ASS) ON RELATIVELY UNDISTURBED LAND

| Depth Code | Depth to Actual Acid Sulfate Soil (m) | Depth to Potential Acid Sulfate Soil (m) |
|------------|---------------------------------------|--|
| 0 | 0-0.5m | 0 |
| 1 | 0.5-1m | 1 |
| 2 | 1-2m | 2 |
| 3 | 2-3m | 3 |
| 4 | 3-4m | 4 |
| 5 | 4-5m | 5 |
| >5m | >5m | >5m |

NOTE:

- The depth codes above imply that a predominance of profiles in the map unit fall within the nominated depth range.
- Actual acid sulfate soils (designated with a color) often occur in the potential acid sulfate soil layer (designated with an S code). Where this occurs, the ASS map unit is colored according to the depth of the water table or the actual water table and overlaid with the S code. In the absence of a water table, the soil layer with the highest acid sulfate soil content is designated as the dominant soil. Where the depth code is shown on the map, no color is assigned to it.
- In areas where there is varying depth to an ASS layer that cannot be separately mapped at the operative scale, two colors are used to designate the dominant depth. This appears as equal width vertical columns e.g. S0/S1.
- P as a subscript indicates sediments of Pleistocene age¹, W as a subscript indicates sediments of Holocene age².
- W as a subscript indicates areas associated with Melaleuca sp. wetlands and occasionally Casuarina glauca communities. Occasional sulfur % in surface layers may be highly variable and does not exceed the Holocene limit. This may include soils from organic materials and recent accretion of sulfides in the soil organic soil environment. ASS spatially occurs at depth. Where this occurs e.g. S_{0W} or S_{1W}, the map is colored as per the actual or potential category and is overlaid with W pattern.
- In a subset of ASS areas with variable sulfur values that exceed the color codes³, but contain varying amounts of carbonate materials that may compensate for the potential acidity. Carbonates in the soil profile are indicated by carbonate nodules, cone fragments or fragments. Such codes are shown as a potential acid sulfate soil patch in carbonate occurring at a 1 to 2m depth is designated S0_C. The map unit is colored as S0 and overlaid with green data.

S_{0L} Limited field assessment but occurs in a landscape position where there is a reasonable probability of ASS occurrence. This is usually land where the present use precludes any disturbance e.g. National Parks, Reserves etc., or land where accessibility is severely restricted.

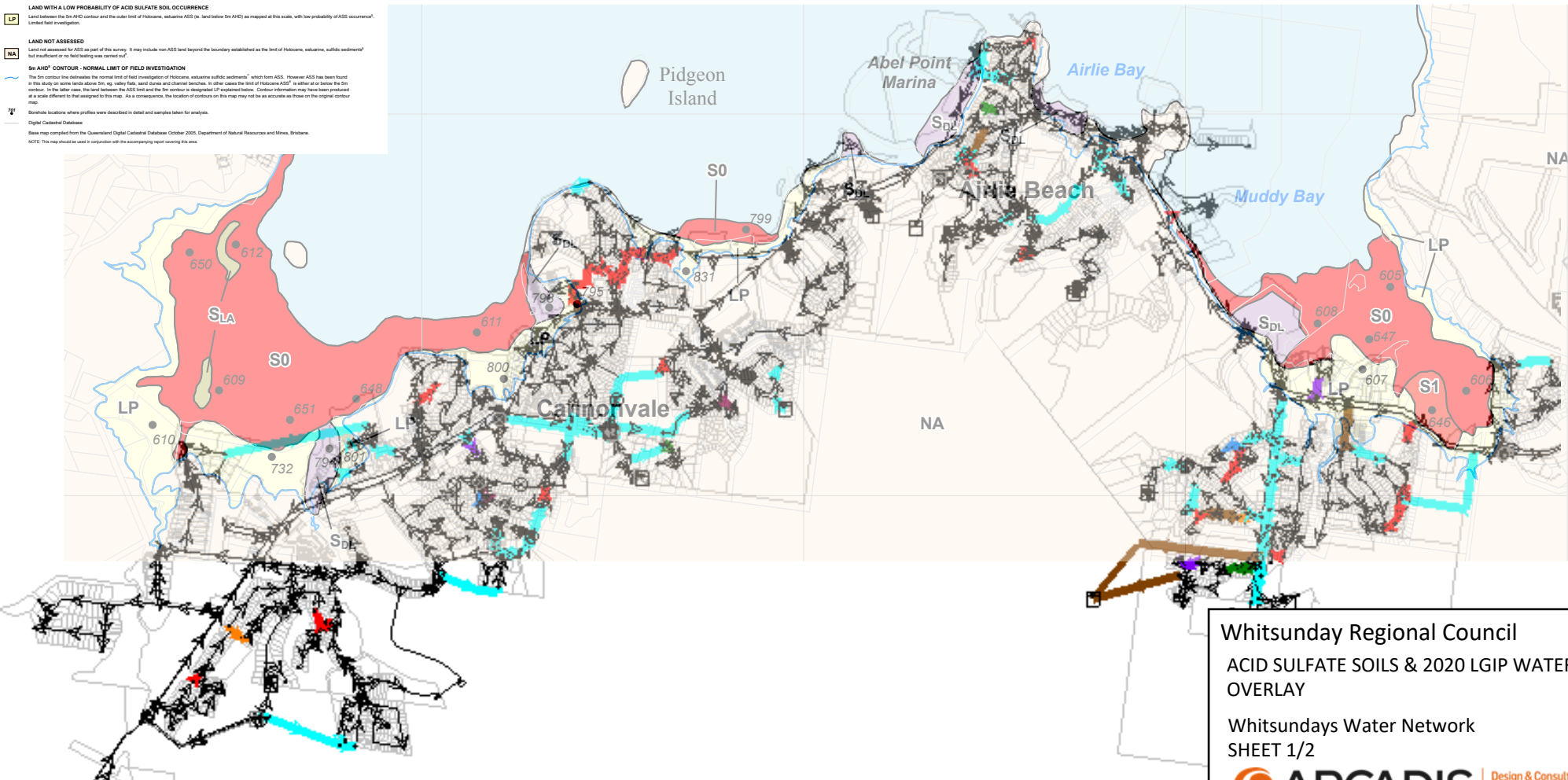
ACID SULFATE ON DISTURBED LAND⁴
S_D Disturbed land, e.g. Canal sediments, Marine, Aquaculture, Quarry, Urban, Industrial likely to contain ASS. (In some cases partial or full treatment may have been undertaken).

LAND WITH A LOW PROBABILITY OF ACID SULFATE SOIL OCCURRENCE
LP Land between the 5m AHD contour and the outer limit of Holocene, estuarine ASS (ie. land below 5m AHD) as mapped at this scale, with low probability of ASS occurrence⁵. Limited field investigation.

LAND NOT ASSESSED
NA Land not assessed for ASS as part of this survey. It may include non-ASS land beyond the boundary established as the limit of Holocene, estuarine, sulfidic sediments⁶ but significant or no field testing was carried out.

5m AHD⁷ CONTOUR - NORMAL LIMIT OF FIELD INVESTIGATION
 The 5m contour line determines the normal limit of field investigation of Holocene, estuarine sulfidic sediments⁸, which form ASS. However ASS has been found in the study or some lands above 5m AHD, valley floor, near dunes and dune benches. In other cases the limit of Holocene ASS⁹ is either at or below the 5m contour. In the latter case, the land between the ASS limit and the 5m contour is designated LP as explained below. Contour information may have been produced at a scale different to that applied to the map. As a consequence, the location of contours on the map may not be as accurate as those on the original contour map.

¹ Borehole locations where profiles were described in detail and samples taken for analysis.
² Digital Cadastral Database.
³ Based map compiled from the Queensland Digital Cadastral Database October 2005. Department of Natural Resources and Mines, Brisbane.
 NOTE: This map should be used in conjunction with the accompanying report covering the area.



Whitsunday Regional Council
 ACID SULFATE SOILS & 2020 LGIP WATER
 OVERLAY
 Whitsundays Water Network
 SHEET 1/2
 Design & Consultancy
 for natural and
 built assets

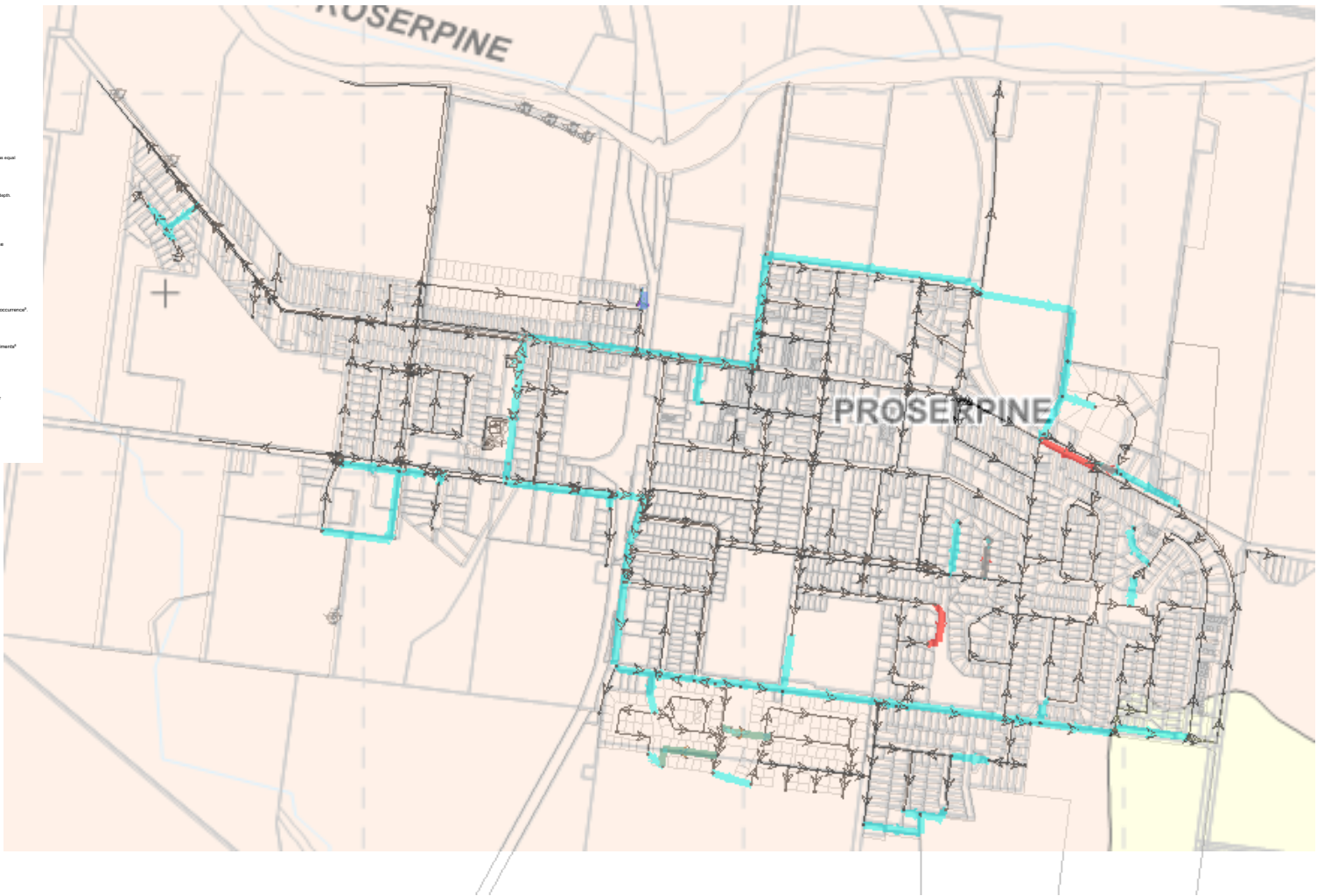
ACID SULFATE SOILS OVERLAY AND LEGEND SOURCED FROM QUEENSLAND GOVERNMENT

REFERENCE

ACID SULFATE SOILS (ASS) ON RELATIVELY UNDISTURBED LAND

| Depth Code | Depth | Depth to Actual Acid Sulfate Soil (m) | Depth to Potential Acid Sulfate Soil (m) |
|------------|--------|---------------------------------------|--|
| 0 | 0-0.5m | A0 | A0 |
| 1 | 0.5-1m | A1 | A1 |
| 2 | 1-2m | A2 | A2 |
| 3 | 2-3m | A3 | A3 |
| 4 | 3-4m | A4 | A4 |
| 5 | 4-5m | A5 | A5 |
| 6 | >5m | A6+ | A6+ |

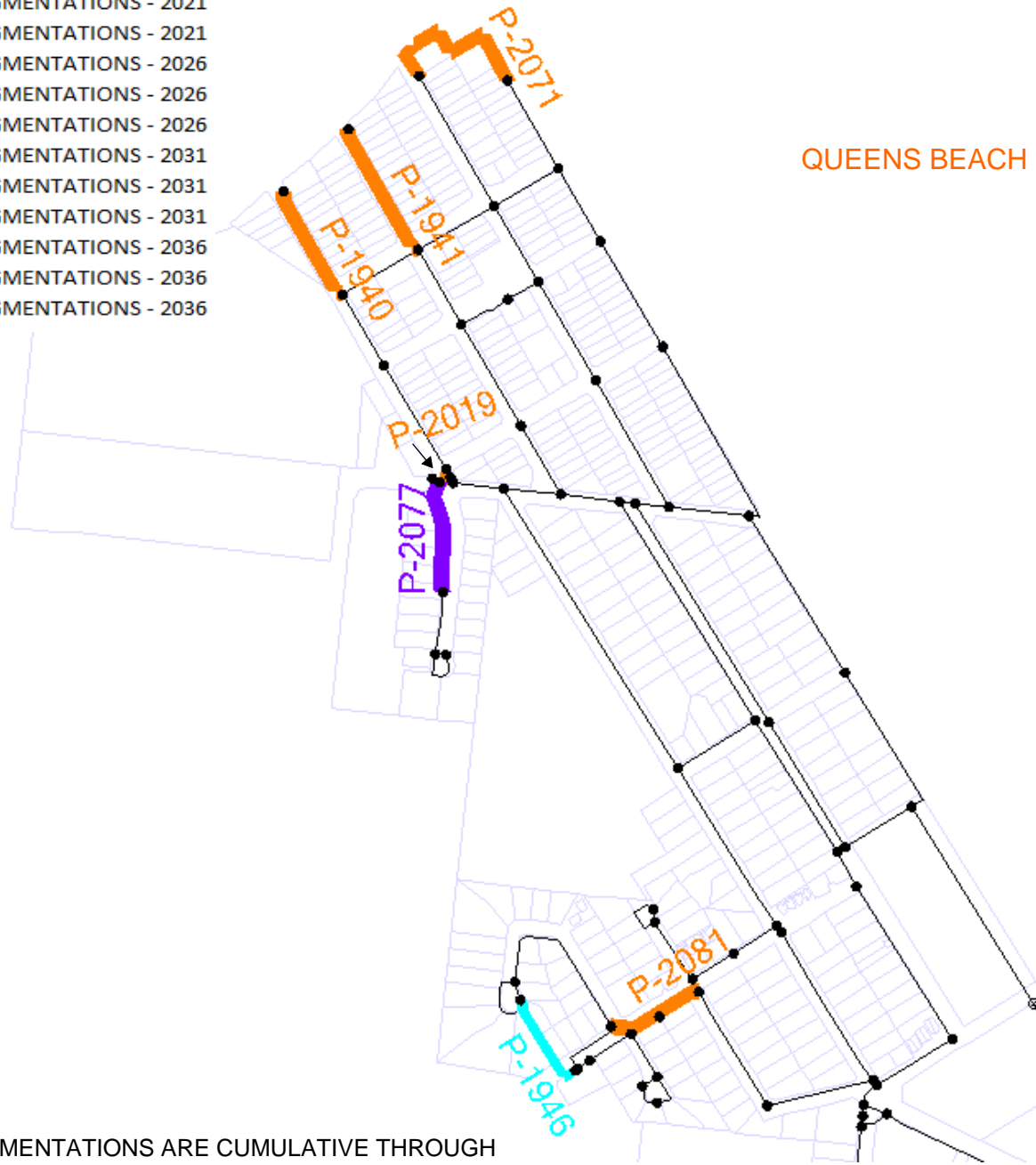
- NOTE:**
- The depth codes above imply that a predominance of profiles in the map unit fall within the nominated depth range.
 - Actual acid sulfate soils (designated with a A code) often occur in potential acid sulfate soil layers (designated with an S code). Where this occurs, the ASS map unit is coloured according to the depth of the outer code or the 'actual' map (AS) and overlaid with yellow dots. An 'S' preceding the soil depth code indicates a strong acid soil layer with field pH ranging from 1.4 to 5.5. This may or may not be a result of sulfate oxidation. Where 'S' depth code is shown on the map, no colour is assigned to it.
 - In areas where there is varying depth to an ASS layer that cannot be separately mapped at the operative scale, two colours are used to designate the dominant depths. This appears as equal width diagonal colours (e.g. S2/S3).
 - P as a subscript indicates sediments of Pleistocene age¹, while P¹ indicates sulfidic sediments (of Pleistocene age) deeper than 5m.
 - W as a subscript indicates areas associated with Melaleuca sp. wetlands and occasionally Casuarina glauca communities. Oxidizable sulfur % in surface layers may be highly variable and often exceeds the 'Actual Code'. This may indicate sulfur from organic materials and reduced oxidation of sulfides in the soil organic soil environment. ASS typically occurs at depth. Where this occurs (e.g. S₁₀ or S₂₀ or A₁₀), the map is coloured as per the actual or potential depth category and is overlaid with W₁₀ pattern.
 - W as a subscript indicates areas with oxidizable sulfur values that exceed the 'Actual Code'² but contain varying amounts of carbonate materials that may compensate for the potential acidity. Contours the carbonate response are indicated occurring with topographic contour lines of 5m intervals. Depth codes are as above (e.g. a potential acid sulfate soil with carbonate occurring at 1 to 2m depth is designated S2₁₀). The map unit is coloured as S2 and overlaid with green data.
- LA** Limited field assessment but occurs in a landscape position where there is a reasonable probability of ASS occurrence. This is usually land where the present use precludes any disturbance (e.g. National Parks, Reserves etc.) or land where accessibility is severely restricted.
- AS** ACID SULFATE ON DISTURBED LAND³
Disturbed land, e.g. Canal estates, Marine, Aquaculture, Quarry, Urban, Industrial likely to contain ASS. (In some cases partial or full treatment may have been undertaken).
- LP** LAND WITH A LOW PROBABILITY OF ACID SULFATE SOIL OCCURRENCE
Land between the 5m AHD contour and the outer limit of Holocene, estuarine ASS (ie. land below 5m AHD) as mapped at this scale, with low probability of ASS occurrence⁴. Limited field investigation.
- NA** LAND NOT ASSESSED
Land not assessed for ASS as part of this survey. It may include non ASS land beyond the boundary established as the limit of Holocene, estuarine, sulfidic sediments⁵ but insufficient or no field testing was carried out⁶.
- 5m AHD CONTOUR - NORMAL LIMIT OF FIELD INVESTIGATION**
The 5m contour line determines the normal limit of field investigation of Holocene, estuarine sulfidic sediments⁷, which form ASS. However ASS has been found in the field on some lands above the 5m topographic limit, such as canal and channel benches. In other cases the limit of Holocene ASS⁸ is either at or below the 5m contour. In the latter case, the land between the ASS limit and the 5m contour is designated LP explained below. Contour information may have been produced at a scale different to that applied to this map. As a consequence, the location of contours on this map may not be as accurate as those on the original contour map.
- ?** Specific locations where profiles were described in detail and samples taken for analysis.
- Digital Cadastral Database
Base map compiled from the Queensland Digital Cadastral Database October 2005. Department of Natural Resources and Mines, Brisbane.
NOTE: This map should be used in conjunction with the accompanying report covering the area.



Whitsunday Regional Council
 ACID SULFATE SOILS & 2020 LGIP WATER
 OVERLAY
 Whitsundays Water Network
 SHEET 2/2

PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |



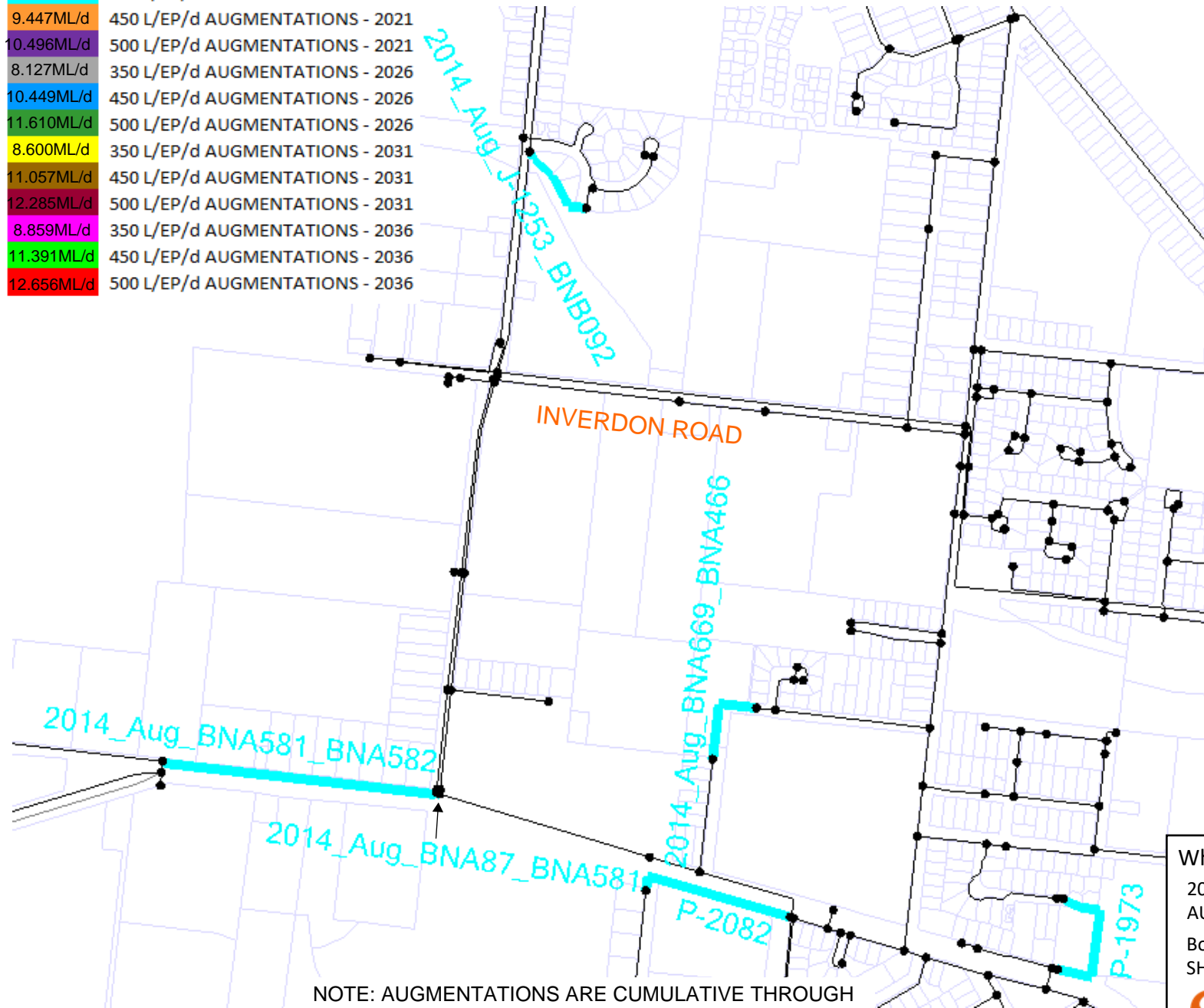
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 1/9



PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
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| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

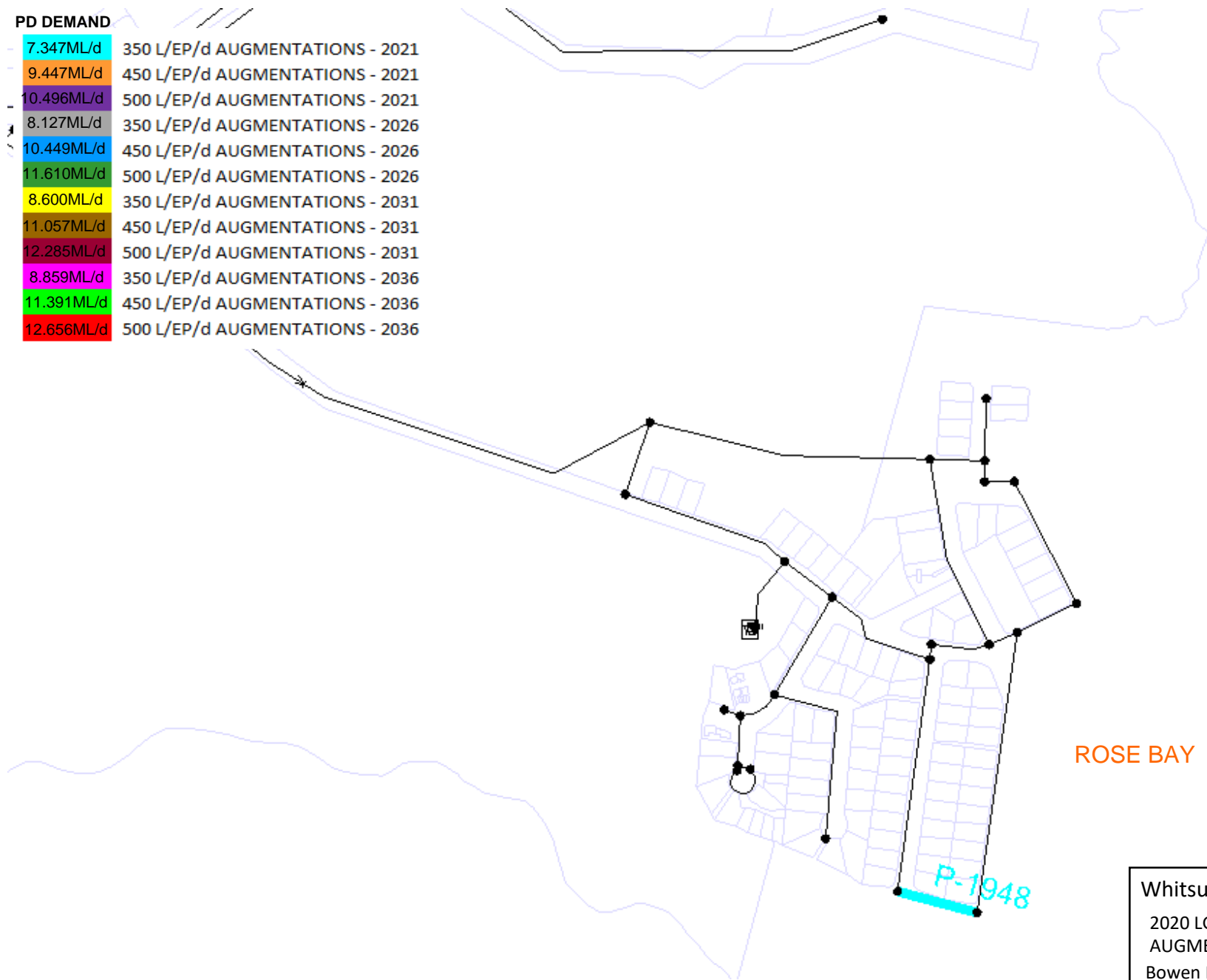


NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 2/9

PD DEMAND

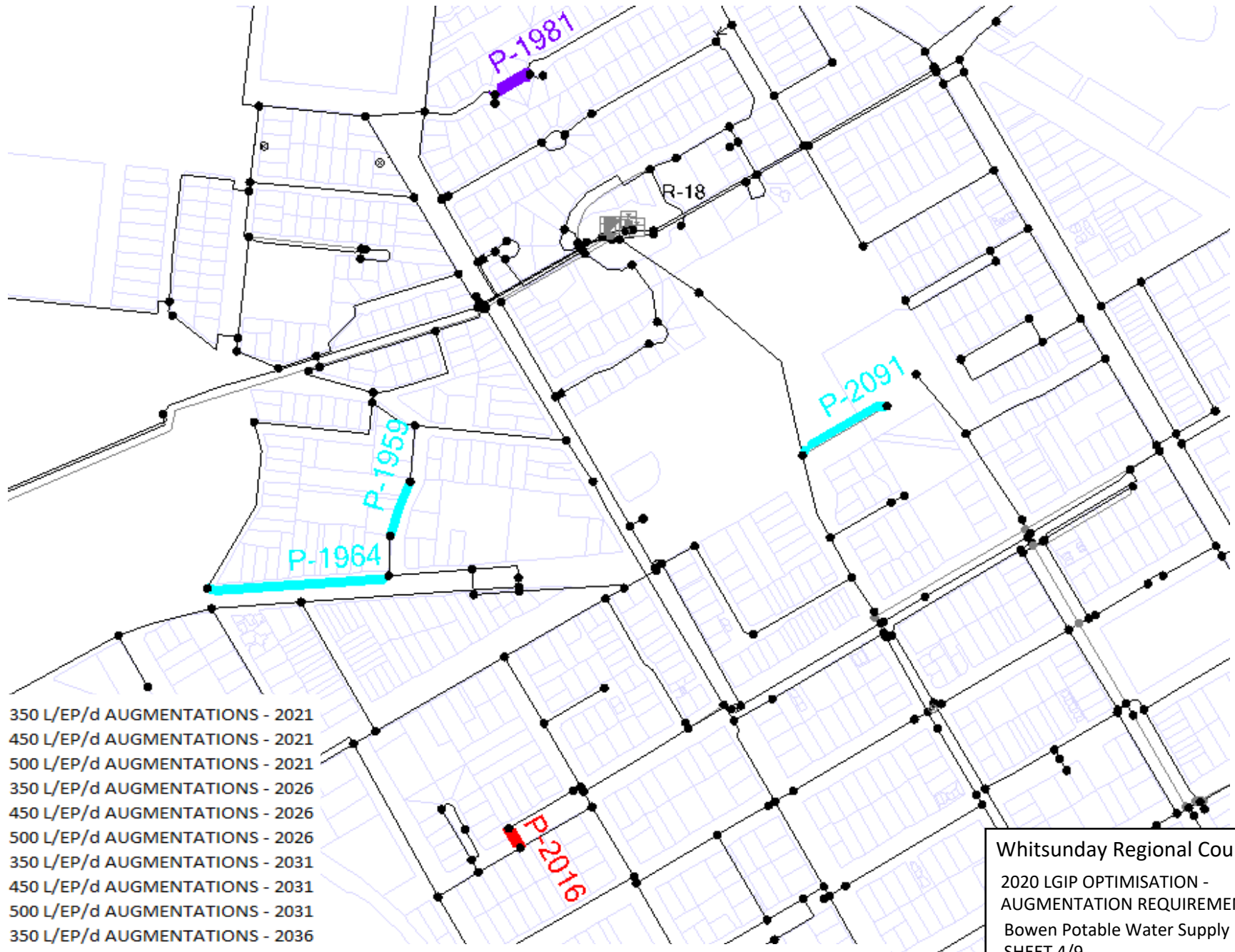
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|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |



ROSE BAY

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 3/9



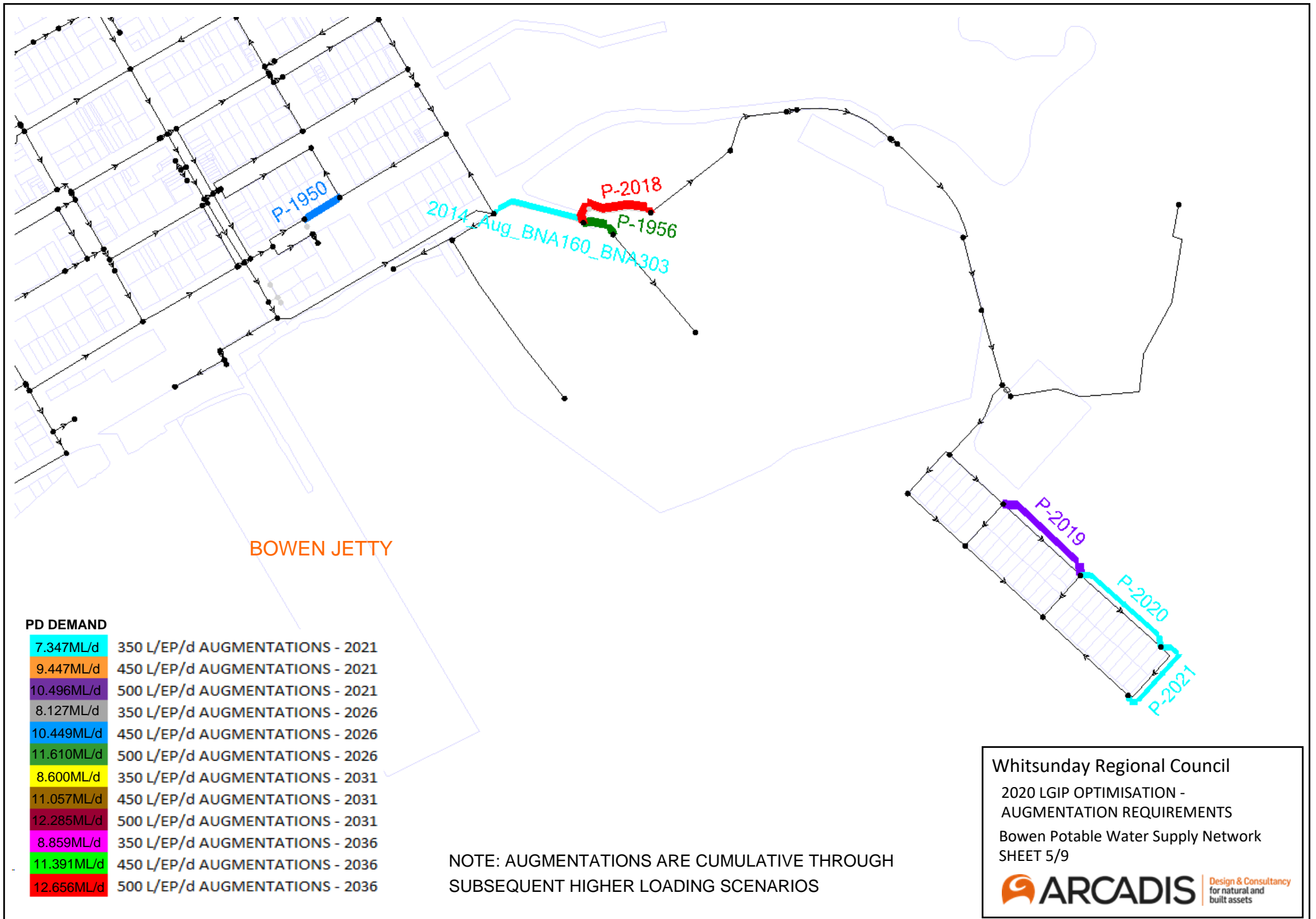
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|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
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| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
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| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

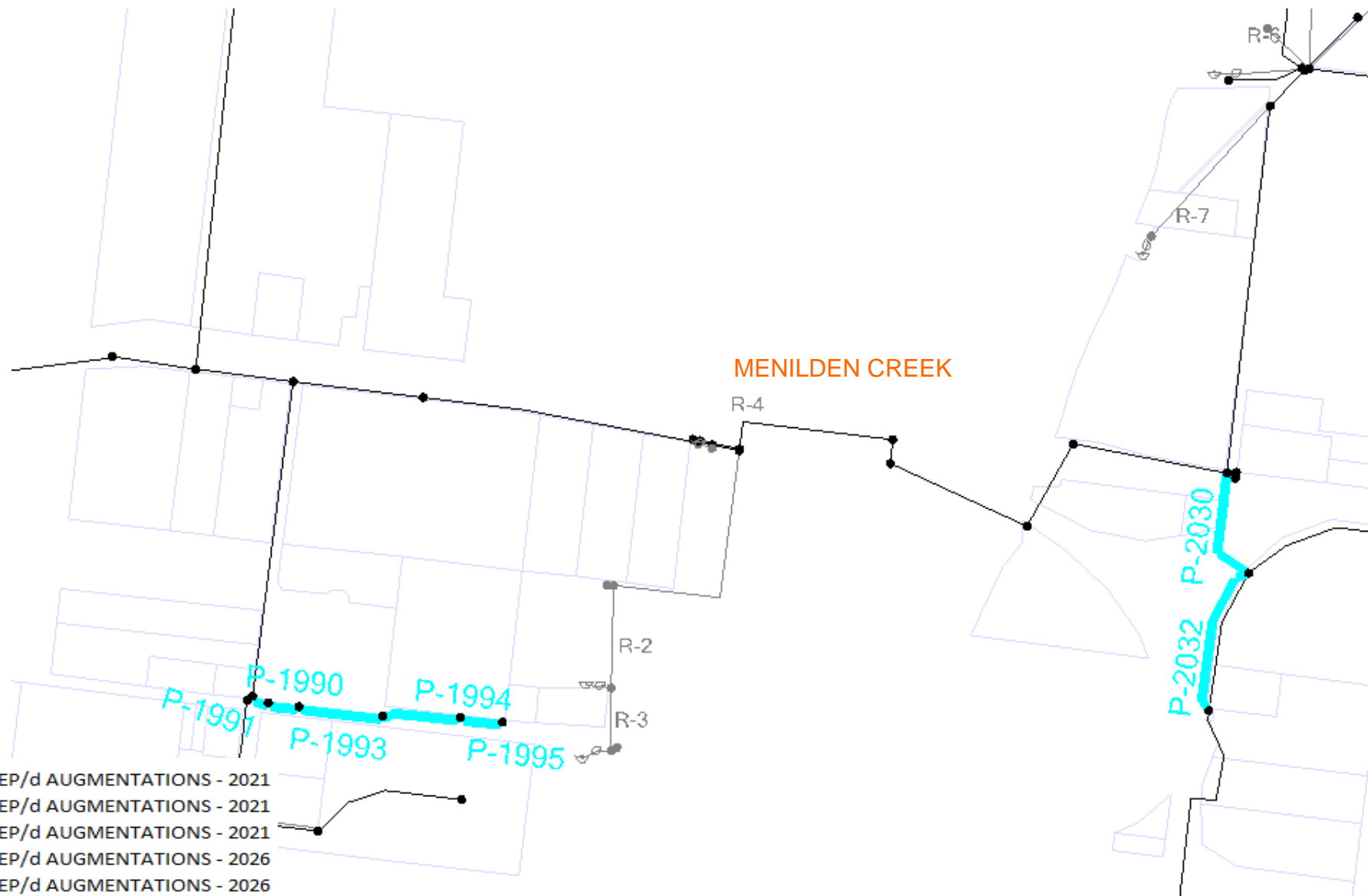
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 4/9



Design & Consultancy
for natural and built assets





PD DEMAND

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|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
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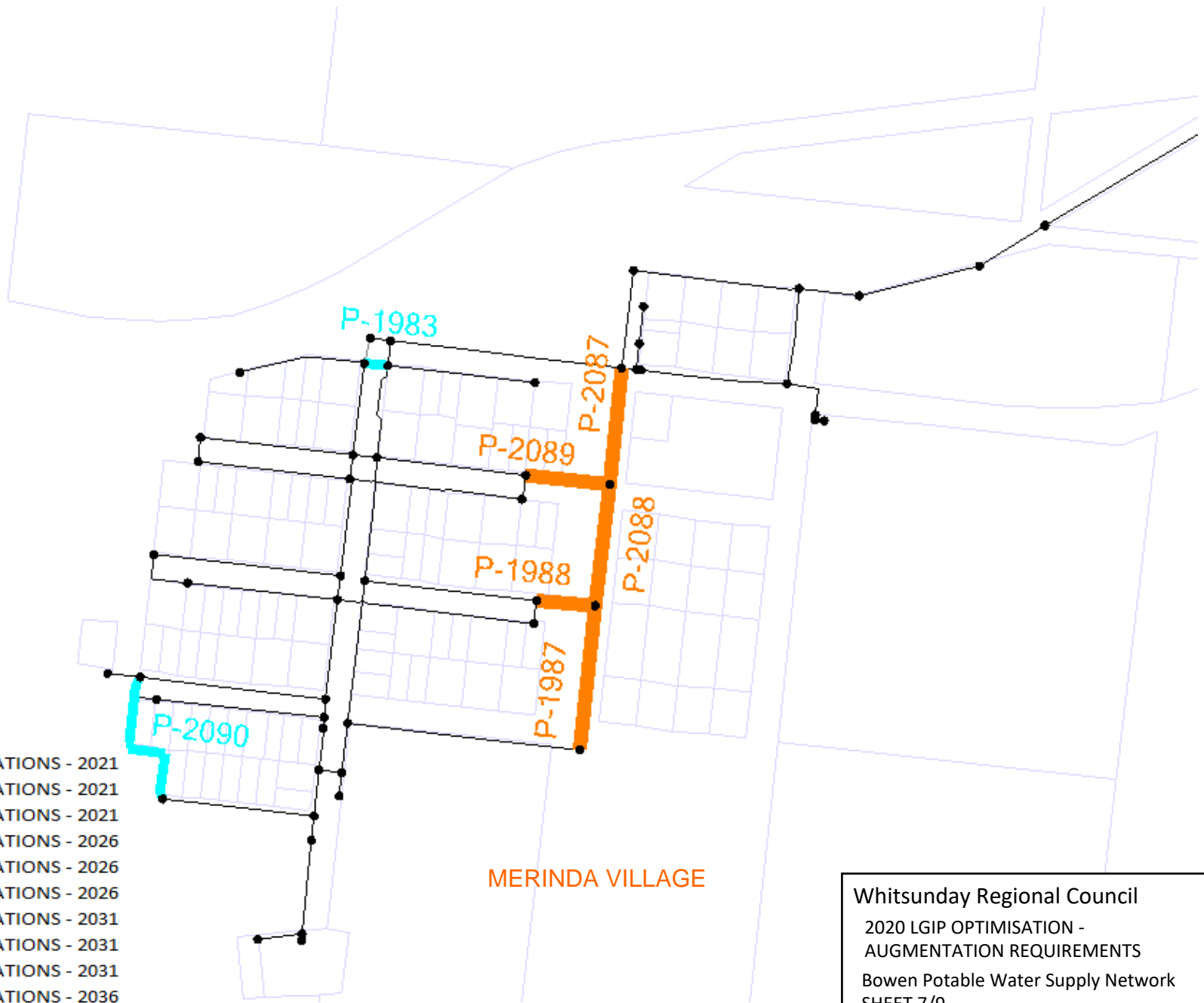
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Bowen Potable Water Supply Network
SHEET 6/9





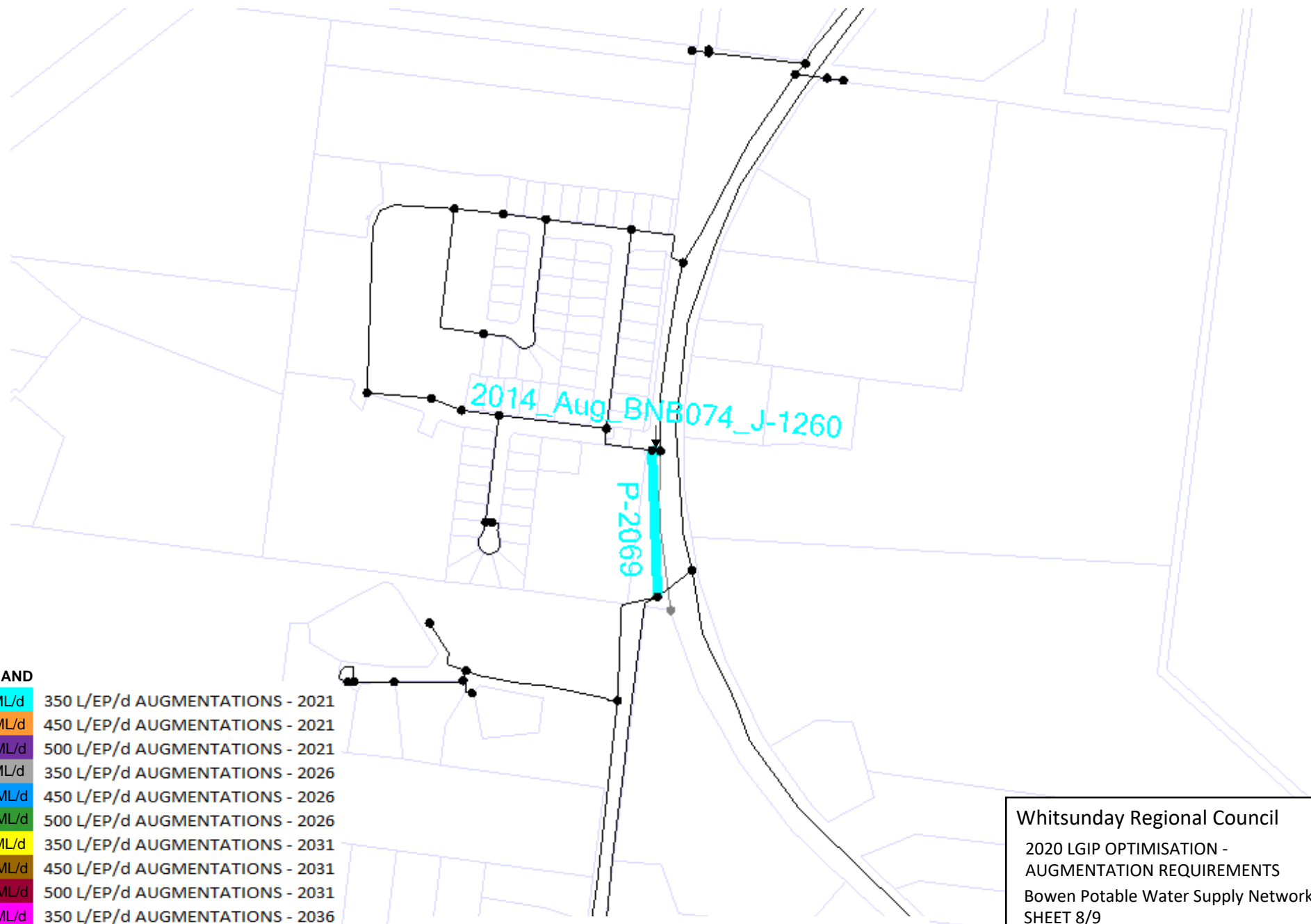
PD DEMAND

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| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
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| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 7/9



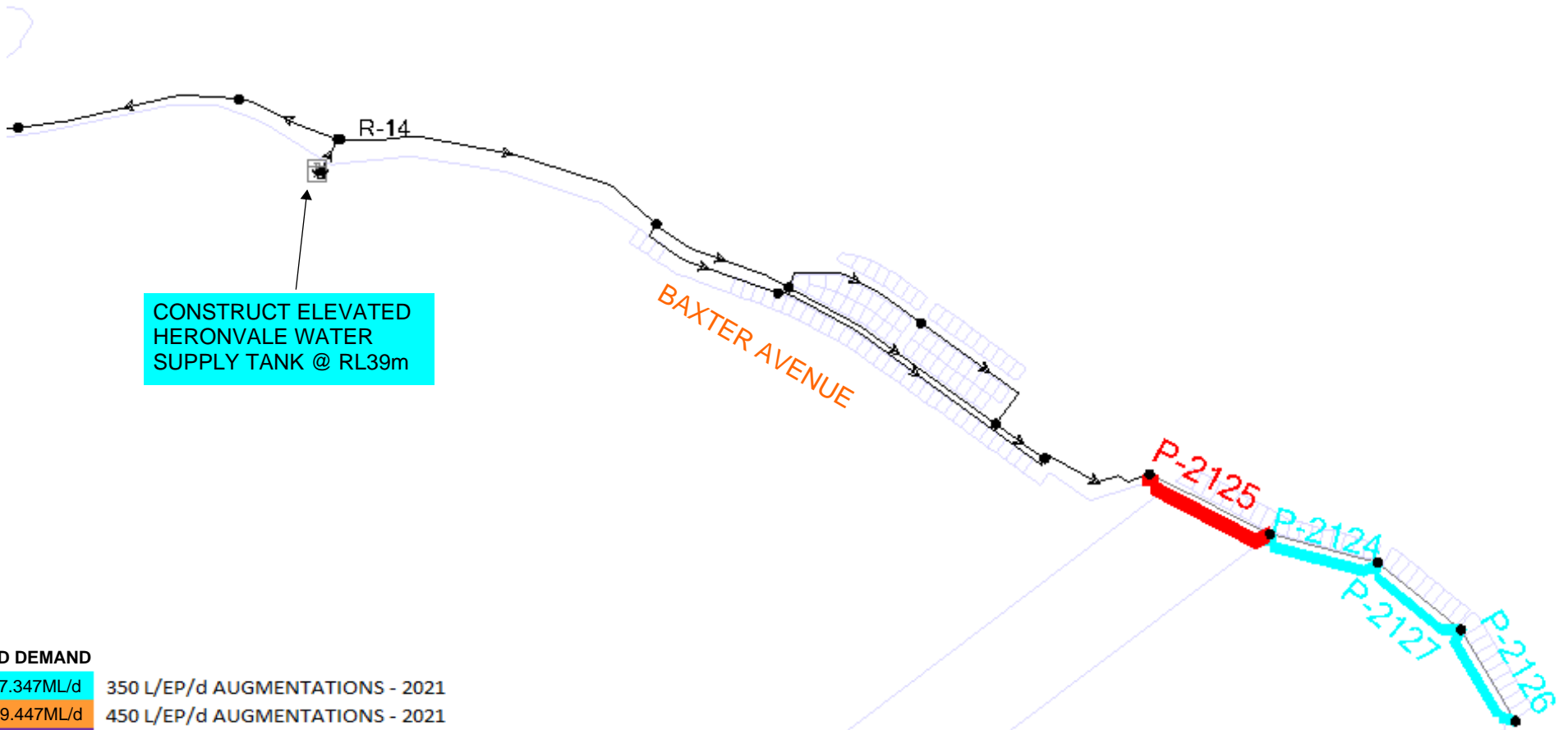


PD DEMAND

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|------------|---------------------------------|
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| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
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| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 8/9



PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
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| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 9/9



PROJECT: Whitsundays Potable Water Network Modelling Project Engineer: M.C/S.H
 DOCUMENT NUMBER: D003-10027536-AAC-03 Software: WaterCad v8i
 DATE: 11.02.2020



**WHITSUNDAYS BOWEN POTABLE WATER NETWORK
 DEMAND MANAGEMENT ASSESSMENT - AUGMENTATION REQUIREMENT SUMMARY**

| AUGMENTATION ID | LENGTH (m) | EXISTING PIPE SEGMENT | | DUPLICATION DN | NEW PIPE DN | COST - \$/m | ADJUSTMENT FACTOR FOR SOIL | 10% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW/DUPLICATION | AC/PIPE REPLACEMENT | | | | | | NON-LGIP AUGMENTATIONS | LGIP AUGMENTATIONS |
|------------------------|-------------|-----------------------|--------------|----------------|-------------|-------------|----------------------------|-------------------------------|-----------------|-------------------------|---------------------|------------|----------------|-------------|----------------------|---|------------------------|----------------------|
| | | START NODE | END NODE | | | | | | | | YEAR INSTALLED | AC/PIPE DN | REPLACEMENT DN | COST - \$/m | CAP & GROUT @ \$20/M | TOTAL COST OF DECOMMISSIONING AND REPLACEMENT | | |
| 2014_Aug_BNA160_BNA303 | 191 | 625: BNA160 | 1157: BNA303 | | | | 1.26 | 10% | 30% | | 1983 | 96.5 | 150 | \$ 262.00 | \$ 3,820.00 | \$ 98,399.38 | \$ 98,399.38 | |
| 2014_Aug_BNA581_BNA582 | 477 | 1098: BNA581 | 1322: BNA582 | | | | 1.26 | 10% | 30% | | | 96.5 | 200 | \$ 310.00 | \$ 9,540.00 | \$ 289,014.30 | \$ 289,014.30 | |
| 2014_Aug_BNA669_BNA466 | 160 | 628: BNA668 | 1310: BNA466 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 57,859.20 | | | | | | \$ 57,859.20 | \$ 57,859.20 | |
| 2014_Aug_BNA87_BNA581 | 8 | 1233: BNA87 | 1098: BNA581 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | | 1962 | 96.5 | 200 | \$ 310.00 | \$ 160.00 | \$ 4,847.20 | \$ 4,847.20 | |
| 2014_Aug_BNB074_J-1260 | 10 | 6923: J-1260 | 3595: BNB074 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | \$ 5,468.40 | | | | | | \$ 5,468.40 | \$ 5,468.40 | |
| 2014_Aug_J-1253_BNB092 | 150 | 6900: J-1253 | 3838: BNB092 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 54,243.00 | | | | | | \$ 54,243.00 | \$ 54,243.00 | |
| P-1940 | 345 | 682: BNA63 | 667: BNA62 | | | | 1.26 | 10% | 30% | | | 96.5 | 150 | \$ 262.00 | \$ 2,900.00 | \$ 74,701.10 | \$ 74,701.10 | |
| P-1941 | 169 | 960: BNA59 | 1069: BNA58 | | | | 1.26 | 10% | 30% | | | 96.5 | 150 | \$ 262.00 | \$ 3,380.00 | \$ 87,065.42 | \$ 87,065.42 | |
| P-1946 | 123 | 3124: BNA362 | 4177: BNA361 | | 100 | \$ 310.00 | 1.26 | 10% | 30% | \$ 67,261.32 | | | | | | \$ 67,261.32 | \$ 67,261.32 | |
| P-1948 | 89 | 548: BNA519 | 643: BNA520 | | | | 1.26 | 10% | 30% | | 1964 | 96.5 | 200 | \$ 310.00 | \$ 1,780.00 | \$ 53,925.10 | \$ 53,925.10 | |
| P-1950 | 81 | 1112: BNA136 | 3738: BNB151 | | 150 | \$ 205.00 | 1.26 | 10% | 30% | \$ 29,291.22 | | | | | | \$ 29,291.22 | \$ 29,291.22 | |
| P-1956 | 71 | 1157: BNA303 | 7341: J-1300 | | | | 1.26 | 10% | 30% | | 1964 | 96.5 | 150 | \$ 262.00 | \$ 1,420.00 | \$ 36,577.78 | \$ 36,577.78 | |
| P-1959 | 82 | 983: BNA276 | 521: BNA274 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 37,897.78 | | | | | | \$ 37,897.78 | \$ 37,897.78 | |
| P-1964 | 258 | 398: BNA273 | 962: BNA270 | | | | 1.26 | 10% | 30% | | | 96.5 | 150 | \$ 262.00 | \$ 5,160.00 | \$ 132,916.44 | \$ 132,916.44 | |
| P-1973 | 243 | 3760: BNA532 | 3727: BNA158 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 87,873.66 | | | | | | \$ 87,873.66 | \$ 87,873.66 | |
| P-1981 | 58 | 559: BNA249 | 1287: BNA248 | | | | 1.26 | 10% | 30% | | | 50 PVC | 100 | \$ 205.00 | | \$ 22,472.10 | \$ 22,472.10 | |
| P-1983 | 26 | 1253: BNA416 | 1269: BNA405 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 12,016.37 | | | | | | \$ 12,016.37 | \$ 12,016.37 | |
| P-1987 | 158 | 7069: J-1268 | 1266: BNA410 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | \$ 86,400.72 | | | | | | \$ 86,400.72 | \$ 86,400.72 | |
| P-1988 | 64 | 7069: J-1268 | 1281: BNA422 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 29,578.75 | | | | | | \$ 29,578.75 | \$ 29,578.75 | |
| P-1990 | 58 | 986: BNA812 | 1193: BNA816 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 26,805.74 | | | | | | \$ 26,805.74 | \$ 26,805.74 | |
| P-1991 | 36 | 1076: BNA801 | 986: BNA812 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 16,638.05 | | | | | | \$ 16,638.05 | \$ 16,638.05 | |
| P-1993 | 148 | 1193: BNA816 | 972: BNA817 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 68,400.86 | | | | | | \$ 68,400.86 | \$ 68,400.86 | |
| P-1994 | 136 | 972: BNA817 | 561: BNA818 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 62,854.85 | | | | | | \$ 62,854.85 | \$ 62,854.85 | |
| P-1995 | 76 | 561: BNA818 | 609: BNA819 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 35,124.77 | | | | | | \$ 35,124.77 | \$ 35,124.77 | |
| P-2016 | 32 | 921: BNA211 | 7104: J-1275 | | 150 | \$ 205.00 | 1.26 | 10% | 30% | \$ 11,571.84 | | | | | | \$ 11,571.84 | \$ 11,571.84 | |
| P-2018 | 175 | 1157: BNA303 | 1186: BNA305 | | | | 1.26 | 10% | 30% | | | 140 PVC | 251 | \$ 205.00 | \$ 3,500.00 | \$ 71,303.75 | \$ 71,303.75 | |
| P-2019 | 217 | 825: BNA311 | 640: BNA312 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 100,290.46 | | | | | | \$ 100,290.46 | \$ 100,290.46 | |
| P-2020 | 220 | 640: BNA312 | 1227: BNA313 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 101,676.96 | | | | | | \$ 101,676.96 | \$ 101,676.96 | |
| P-2021 | 191 | 1227: BNA313 | 1288: BNA314 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 88,274.09 | | | | | | \$ 88,274.09 | \$ 88,274.09 | |
| P-2030 | 200 | 1212: BNA323 | 7122: J-1279 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | \$ 109,368.00 | | | | | | \$ 109,368.00 | \$ 109,368.00 | |
| P-2032 | 263 | 7122: J-1279 | 7118: J-1277 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 121,550.18 | | | | | | \$ 121,550.18 | \$ 121,550.18 | |
| P-2057 | 218 | 1067: BNA225 | 7243: J-1287 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 100,752.62 | | | | | | \$ 100,752.62 | \$ 100,752.62 | |
| P-2069 | 163 | 7269: J-1288 | 3595: BNB074 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | \$ 89,134.92 | | | | | | \$ 89,134.92 | \$ 89,134.92 | |
| P-2071 | 215 | 1275: BNA54 | 1153: BNA50 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 77,748.30 | | | | | | \$ 77,748.30 | \$ 77,748.30 | |
| P-2077 | 135 | 3116: BNA529 | 7280: J-1291 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 62,392.68 | | | | | | \$ 62,392.68 | \$ 62,392.68 | |
| P-2078 | 15 | 3116: BNA529 | 3122: BNA527 | 150 | | \$ 262.00 | 1.26 | 10% | 30% | \$ 6,932.52 | | | | | | \$ 6,932.52 | \$ 6,932.52 | |
| P-2081 | 117 | 943: BNA73 | 7285: J-1292 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 42,309.54 | | | | | | \$ 42,309.54 | \$ 42,309.54 | |
| P-2082 | 280 | 1364: BNA285 | 7289: J-1293 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 101,253.60 | | | | | | \$ 101,253.60 | \$ 101,253.60 | |
| P-2087 | 127 | 569: BNA800 | 7298: J-1294 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | \$ 69,448.68 | | | | | | \$ 69,448.68 | \$ 69,448.68 | |
| P-2088 | 133 | 7298: J-1294 | 7069: J-1268 | | 200 | \$ 310.00 | 1.26 | 10% | 30% | \$ 72,729.72 | | | | | | \$ 72,729.72 | \$ 72,729.72 | |
| P-2089 | 92 | 917: BNA418 | 7298: J-1294 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 42,519.46 | | | | | | \$ 42,519.46 | \$ 42,519.46 | |
| P-2090 | 171 | 392: BNA822 | 1024: BNA428 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 61,837.02 | | | | | | \$ 61,837.02 | \$ 61,837.02 | |
| P-2091 | 142 | 1110: BNA117 | 1229: BNA235 | | | | 1.26 | 10% | 30% | | | 96.5 | 210 | \$ 310.00 | \$ 2,840.00 | \$ 86,037.80 | \$ 86,037.80 | |
| Isolation Valves Added | | | | | | | | | | | | | | | | | | |
| | 6123 | | | | | | TOTAL | | | \$ 1,937,505.28 | | | | | | \$ 957,260.37 | \$ 2,448,409.80 | \$ 446,355.85 |

PROJECT: Whitsundays Potable Water Network Modelling
 Project Engineer: M.C/S.H
 DOCUMENT NUMBER: D003-10027536-AAC-03
 DATE: 11.02.2020
 Software: WaterCad v8i



**WHITSUNDAYS BOWEN POTABLE WATER NETWORK
 DEMAND MANAGEMENT ASSESSMENT - AUGMENTATION REQUIREMENT SUMMARY**

| AUGMENTATION ID | 350 L/EP/d DEMAND - 2021 | 450 L/EP/d DEMAND - 2021 | BASELINE 500 L/EP/d DEMAND - 2021 | 350 L/EP/d DEMAND - 2026 | 450 L/EP/d DEMAND - 2026 | BASELINE 500 L/EP/d DEMAND - 2026 | 350 L/EP/d DEMAND - 2031 | 450 L/EP/d DEMAND - 2031 | BASELINE 500 L/EP/d DEMAND - 2031 | 350 L/EP/d DEMAND - 2036 | 450 L/EP/d DEMAND - 2036 | BASELINE 500 L/EP/d DEMAND - 2036 |
|------------------------|--------------------------|--------------------------|-----------------------------------|--------------------------|--------------------------|-----------------------------------|--------------------------|--------------------------|-----------------------------------|--------------------------|--------------------------|-----------------------------------|
| 2014_Aug_BNA160_BNA303 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| 2014_Aug_BNA581_BNA582 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| 2014_Aug_BNA669_BNA466 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| 2014_Aug_BNA87_BNA581 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| 2014_Aug_BNB074_J-1260 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| 2014_Aug_J-1253_BNB092 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1940 | | | | | | | | | | | | |
| P-1941 | | | | | | | | | | | | |
| P-1946 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1948 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1950 | | | | | | | | | | | | |
| P-1956 | | | | | | | | | | | | |
| P-1959 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1964 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1973 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1981 | | | | | | | | | | | | |
| P-1983 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1987 | | | | | | | | | | | | |
| P-1988 | | | | | | | | | | | | |
| P-1990 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1991 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1993 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1994 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-1995 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2016 | | | | | | | | | | | | |
| P-2018 | | | | | | | | | | | | |
| P-2019 | | | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2020 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2021 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2030 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2032 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2057 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2069 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2071 | | | | | | | | | | | | |
| P-2077 | | | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2078 | | | | | | | | | | | | |
| P-2081 | | | | | | | | | | | | |
| P-2082 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2087 | | | | | | | | | | | | |
| P-2088 | | | | | | | | | | | | |
| P-2089 | | | | | | | | | | | | |
| P-2090 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| P-2091 | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |
| Isolation Valves Added | Blue | Orange | Purple | Grey | Blue | Green | Yellow | Brown | Dark Red | Magenta | Light Green | Red |

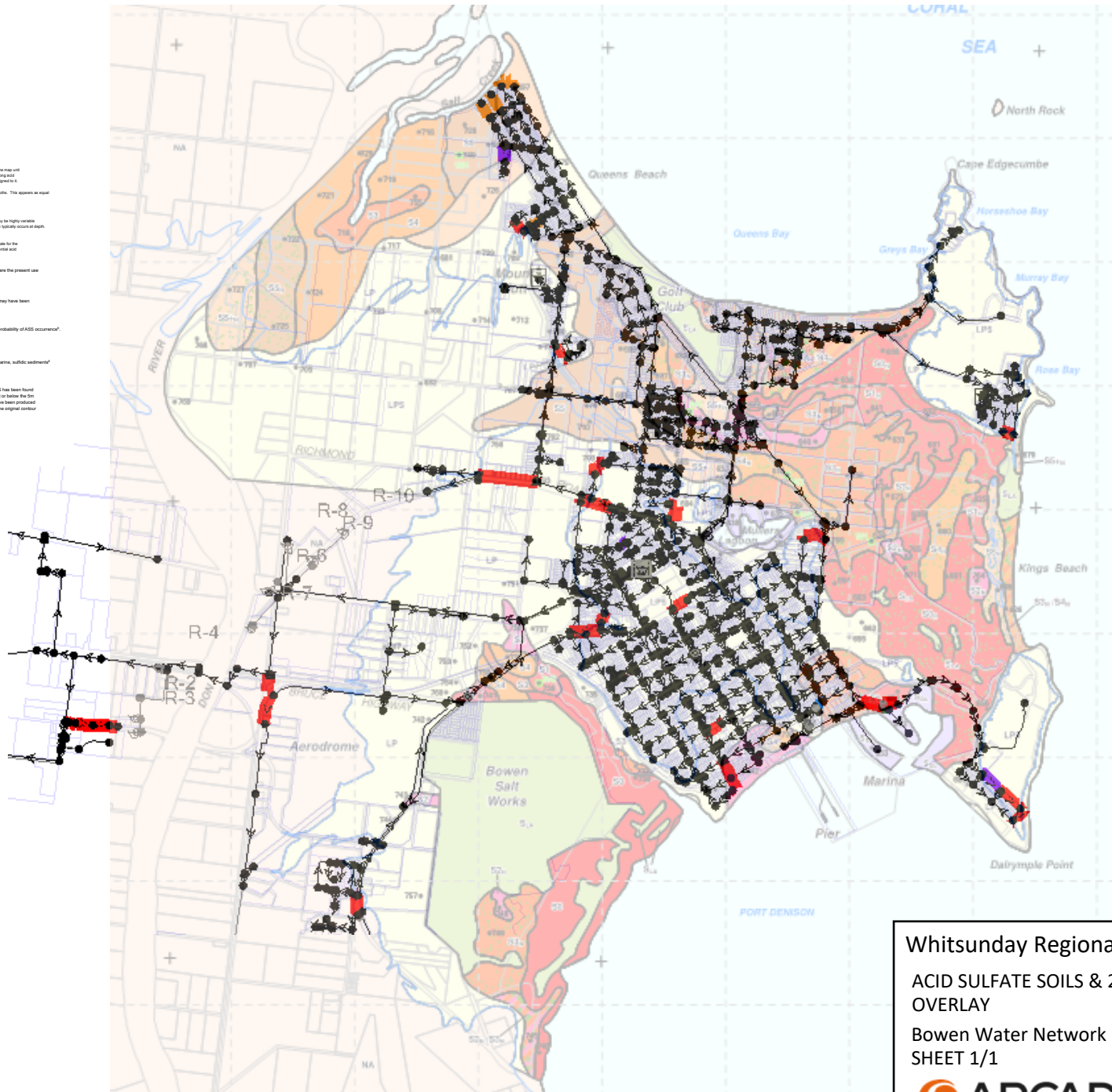
REFERENCE

ACID SULFATE SOILS (ASS) ON RELATIVELY UNDISTURBED LAND

| Depth | Depth Code | Depth to Actual Acid Subsoil (m) (A0) | Depth to Strongly Acidic Soil layer (m) (A1 to A5) | Depth to Potential Acid Sulfate Soil (m) (S0) |
|----------|------------|---------------------------------------|--|---|
| 0 - 0.5m | 0 | A0 | a0 | S0 |
| 0.5 - 1m | 1 | A1 | a1 | S1 |
| 1 - 2m | 2 | A2 | a2 | S2 |
| 2 - 3m | 3 | A3 | a3 | S3 |
| 3 - 4m | 4 | A4 | a4 | S4 |
| 4 - 5m | 5 | A5 | a5 | S5 |
| >5m | 5+ | A5+ | a5+ | S5+ |

NOTE: The depth codes above imply that a predominance of profiles in the map unit fall within the nominated depth range.

- Actual acid sulfate soils (designated with a color) often occur in potential acid sulfate soil layers (designated with an S code). Where this occurs, the ASS map unit is colored according to the depth of the outer code of the actual acid sulfate soil overlaid onto yellow soils. An 'S' preceding the soil depth code indicates a strong acid soil layer with field pH ranging from 1.4 to 3.0. This may or may not be a result of sulfate oxidation. Where 'S' depth code is shown on the map, no colour is assigned to it.
 - In areas where there is varying depth to an ASS layer that cannot be separately mapped at the operative scale, two colours are used to designate the dominant depths. This appears as equal width vertical columns, e.g. S2/S3.
 - P as a subscript indicates sediments of Pleistocene age¹, and that 'S' indicates sulfidic sediments (of Pleistocene age) deeper than 5m.
 - W as a subscript indicates areas associated with Midlandia sp. wetlands and occasionally Casuarina glauca communities. Oxidizable sulfur (%) in surface layers may be highly variable and often exceeds the 'Acid Sulfate' threshold. This may indicate soils from organic materials and reduce accuracy of sulfidic or non-organic soil environments. ASS spatially occurs at depth. Where this occurs e.g. 'S₁₀' or 'S₁₀W', the map is coloured as per the actual or potential depth category and is overlaid with 'W' pattern.
 - As a subscript indicates areas with oxidizable sulfur values that exceed the 'Acid Sulfate' but contain varying amounts of carbonate materials that may compensate for the potential acidity. Contouring the carbonate response are indicated occurring and topographic, scale features of topographic. Depth codes are as above, e.g. a potential acid sulfate soil with carbonate occurring at 1 to 2m depth is designated 'S₁'. The map unit is coloured as S2 and overlaid with green data.
 - Limited field assessment but occurs in a landscape position where there is a reasonable probability of ASS occurrence. This is usually land where the present use preclude any disturbance e.g. National Parks, Reserves etc., or land where accessibility is severely restricted.
- ACID SULFATE ON DISTURBED LAND²**
- Disturbed land, e.g. Canal estates, Marine, Aquaculture, Quarry, Urban, Industrial likely to contain ASS. (In some cases partial or full treatment may have been undertaken).
- LAND WITH A LOW PROBABILITY OF ACID SULFATE SOIL OCCURRENCE**
- Land between the 5m AHD contour and the outer limit of historic, extensive ASS (ie. land below 5m AHD) as mapped at this scale, with low probability of ASS occurrence³. Limited field investigation.
- LAND NOT ASSESSED**
- Land not assessed for ASS as part of this survey. It may include non ASS land beyond the boundary established as the limit of historic, extensive, sulfidic sediments⁴ but insufficient or no field testing was carried out⁵.
- 5m AHD⁶ CONTOUR - NORMAL LIMIT OF FIELD INVESTIGATION**
- The 5m contour was determined the normal limit of field investigation of historic, extensive sulfidic sediments⁷, which form ASS. However ASS has been found in the depth on some lands above 5m, top valley floor, sand dunes and drier benches. In other cases the limit of historic ASS⁸ is either at or below the 5m contour. In the latter case, the land between the ASS limit and the 5m contour is designated LP⁹ explained below. Contour information may have been produced at a scale different to that applied to this map. As a consequence, the boundary of contour on this map may not be as accurate as those in the original contour map.
 - Isolated locations where profiles were described in detail and samples taken for analysis.
- ¹ Digital Catalogue Database
² Base map compiled from the Queensland Digital Catalogue Database October 2005. Department of Natural Resources and Mines, Brisbane.
 NOTE: This map should be used in conjunction with the accompanying report covering the area.

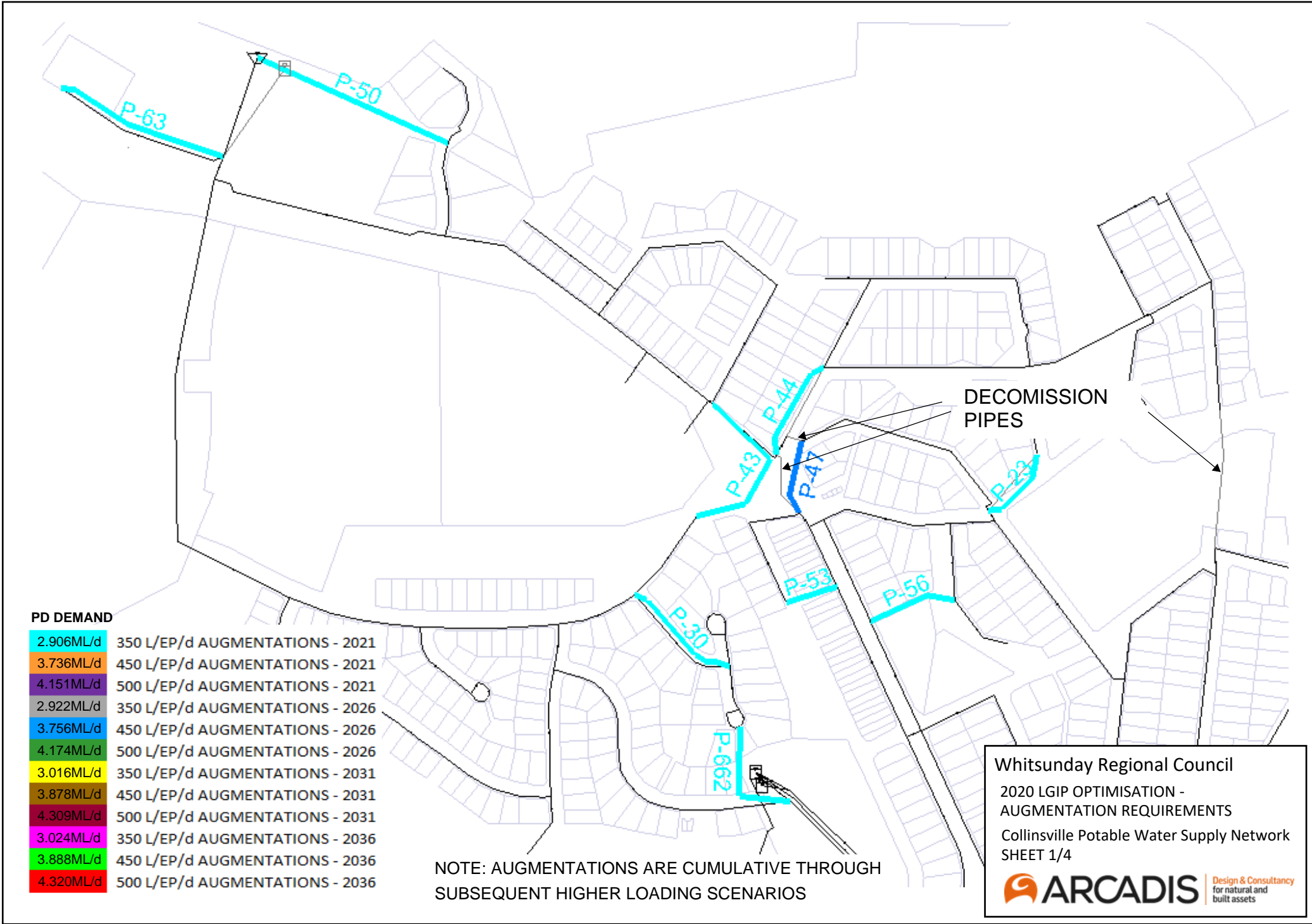


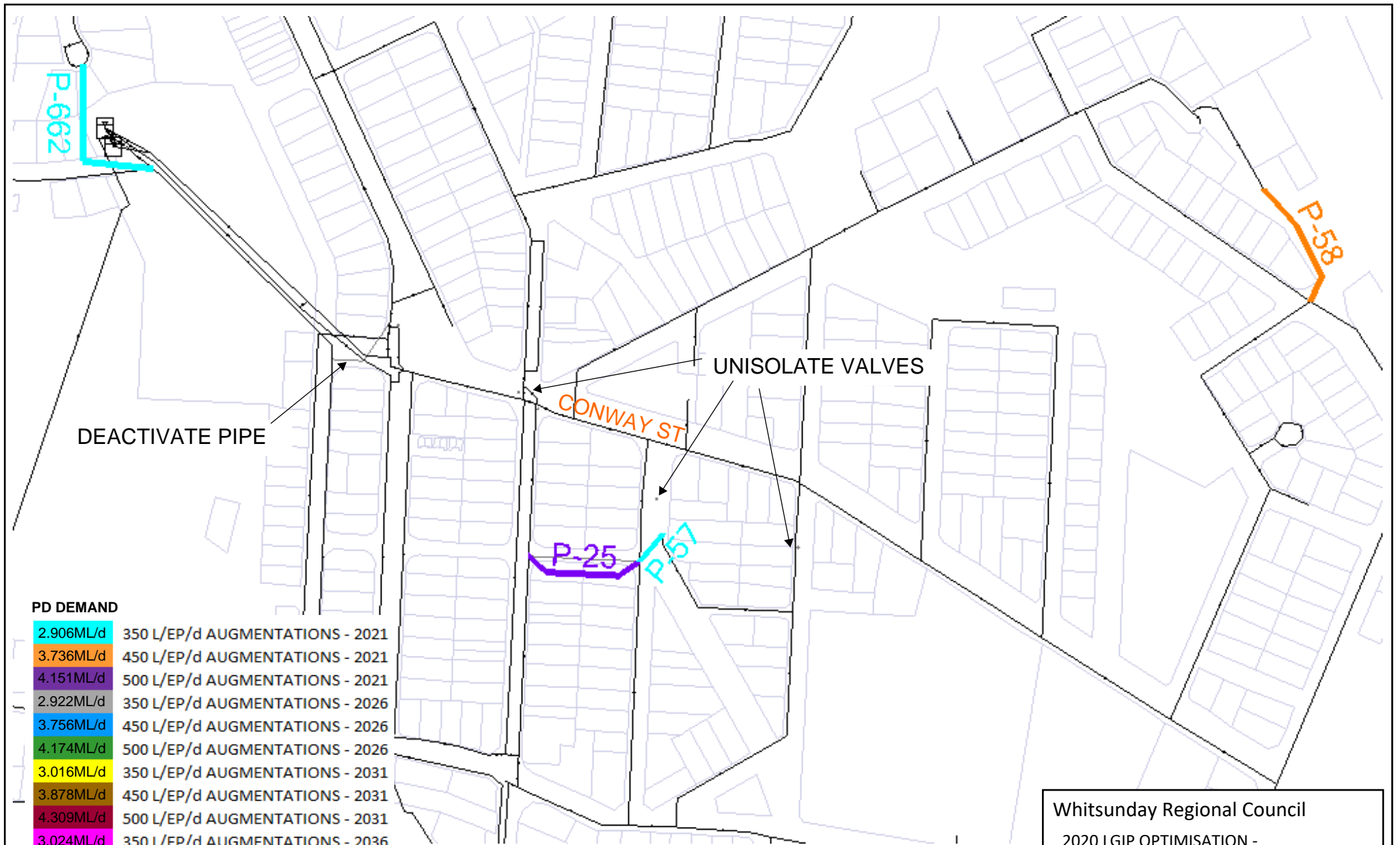
Whitsunday Regional Council
 ACID SULFATE SOILS & 2020 LGIP WATER
 OVERLAY
 Bowen Water Network
 SHEET 1/1



Design & Consultancy
 for natural and
 built assets

ACID SULFATE SOILS OVERLAY AND LEGEND SOURCED FROM QUEENSLAND GOVERNMENT



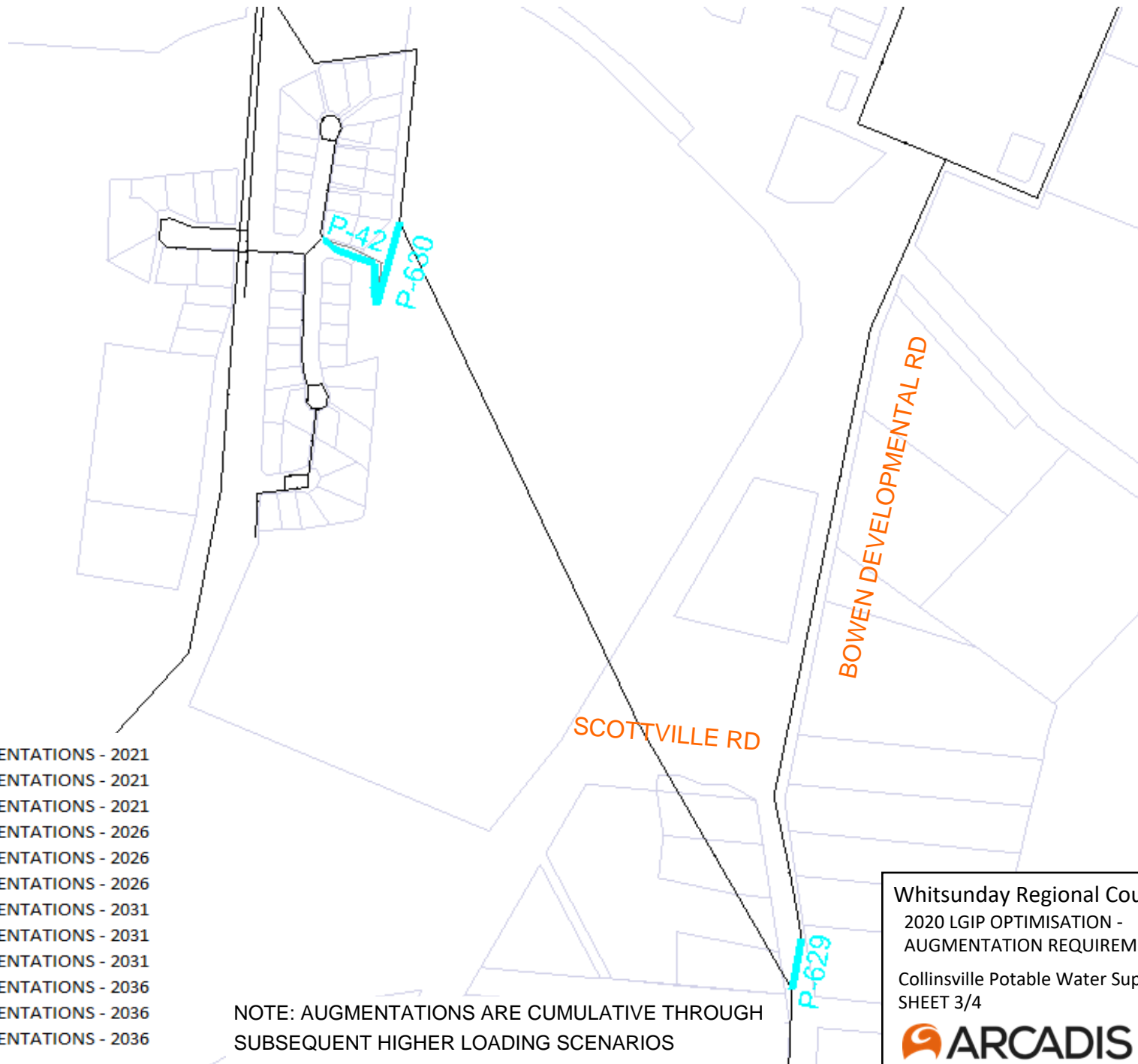


PD DEMAND

| | |
|-----------|---------------------------------|
| 2.906ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 3.736ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 4.151ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 2.922ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 3.756ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 4.174ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 3.016ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 3.878ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 4.309ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 3.024ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 3.888ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 4.320ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENTS
 Collinsville Potable Water Supply Network
 SHEET 2/4



PD DEMAND

| | |
|-----------|---------------------------------|
| 2.906ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 3.736ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 4.151ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 2.922ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 3.756ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 4.174ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 3.016ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 3.878ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 4.309ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 3.024ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 3.888ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 4.320ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS
Collinsville Potable Water Supply Network
SHEET 3/4

Design & Consultancy
for natural and
built assets



PD DEMAND

| | |
|-----------|---------------------------------|
| 2.906ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 3.736ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 4.151ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 2.922ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 3.756ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 4.174ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 3.016ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 3.878ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 4.309ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 3.024ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 3.888ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 4.320ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS

Collinsville Potable Water Supply Network
 SHEET 4/4



Design & Consultancy
 for natural and
 built assets

PROJECT: WRC Potable Water Network Modelling Project Engineer: M.C./S.H.
DOCUMENT NUMBER: D001-10027536-AAC-03 Software: WaterCad v8i
DATE: 10.02.2020



**WHITSUNDAYS COLLINSVILLE POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENT SUMMARY - DATA**

| AUGMENTATION ID | LENGTH (m) | EXISTING PIPE SEGMENT | | | NEW PIPE DN | COST - \$/m | ADJUSTMENT FACTOR FOR SOIL | 10% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW/DUPLICATION | AC/PIPE REPLACEMENT | | | | | NON-LGIP AUGMENTATIONS | LGIP AUGMENTATIONS |
|--------------------------------------|----------------|-----------------------|----------|----------------|-------------|-------------|----------------------------|-------------------------------|-----------------|-------------------------|---------------------|-------|----------------|-------------|----------------------|------------------------|------------------------|
| | | START NODE | END NODE | DUPLICATION DN | | | | | | | YEAR INSTALLED | AC DN | REPLACEMENT DN | COST - \$/m | CAP & GROUT @ \$20/M | | |
| P-23 | 108 | CLC20 | CLC24 | | | | 1.26 | 10% | 30% | | | 100 | 150 | \$ 262.00 | \$ 2,160.00 | \$ 55,639.44 | \$ 55,639.44 |
| P-25 | 118 | CLC97 | CLC98 | | | | 1.26 | 10% | 30% | | | 100 | 150 | \$ 262.00 | \$ 2,360.00 | \$ 60,791.24 | \$ 60,791.24 |
| P-30 | 165 | CLC164 | CLC166 | | | | 1.26 | 10% | 30% | | | 100 | 150 | \$ 262.00 | \$ 3,300.00 | \$ 85,004.70 | \$ 85,004.70 |
| P-35 | 309 | CLC195 | J-8 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 142,809.91 | | 100 | 150 | | | \$ 142,809.91 | \$ 142,809.91 |
| P-42 | 108 | CLC130 | CLC121 | | | | 1.26 | 10% | 30% | | | 100 | 150 | | | \$ 55,639.44 | \$ 55,639.44 |
| P-43 | 243 | CLC165 | CLC12 | | 160 | \$ 262.00 | 1.26 | 10% | 30% | \$ 112,306.82 | | 100 | 160 | 262 | \$ 2,160.00 | \$ 55,639.44 | \$ 112,306.82 |
| P-44 | 141 | CLC6 | CLC18 | | | | 1.26 | 10% | 30% | | | 100 | 160.7 | \$ 272.00 | \$ 2,820.00 | \$ 75,305.28 | \$ 75,305.28 |
| P-47 | 101 | CLC7 | J-4 | | 152.4 | \$ 262.00 | 1.26 | 10% | 30% | \$ 46,678.97 | | | 152.4 | | | \$ 46,678.97 | \$ 46,678.97 |
| P-50 | 279 | R-1 | J-5 | | 160 | \$ 262.00 | 1.26 | 10% | 30% | \$ 128,944.87 | | | 160 | | | \$ 128,944.87 | \$ 128,944.87 |
| P-53 | 71 | CLC55 | J-6 | | 110.7 | \$ 216.00 | 1.26 | 10% | 30% | \$ 27,052.70 | | | 110.7 | | | \$ 27,052.70 | \$ 27,052.70 |
| P-56 | 121 | CLC38 | J-7 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 55,922.33 | | | 150 | | | \$ 55,922.33 | \$ 55,922.33 |
| P-57 | 35 | CLC98 | CLC213 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 16,175.88 | | | 150 | | | \$ 16,175.88 | \$ 16,175.88 |
| P-58 | 129 | CLC72 | CLC266 | | 106 | \$ 205.00 | 1.26 | 10% | 30% | \$ 46,648.98 | | | 106 | | | \$ 46,648.98 | \$ 46,648.98 |
| P-61 | 203 | CLC183 | CLC184 | | 110 | \$ 205.00 | 1.26 | 10% | 30% | \$ 73,408.86 | | | 110 | | | \$ 73,408.86 | \$ 73,408.86 |
| P-63 | 237 | CLC258 | CLC259 | | 160 | \$ 272.00 | 1.26 | 10% | 30% | \$ 113,714.50 | | | | | | \$ 113,714.50 | \$ 113,714.50 |
| P-629 | 55 | CLC205 | CLC267 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 25,419.24 | | | 150 | | | \$ 25,419.24 | \$ 25,419.24 |
| P-630 | 91 | CLC172 | CLC121 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 42,057.29 | | | 150 | | | \$ 42,057.29 | \$ 42,057.29 |
| P-662 | 163 | CLC170 | CLC147 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 58,944.06 | | | 100 | | | \$ 58,944.06 | \$ 58,944.06 |
| Deactivate P-45 | | | | | | | | | | | | | | | | | |
| Deactivate 20858 | | | | | | | | | | | | | | | | | |
| Deactivate 20909 | | | | | | | | | | | | | | | | | |
| Deactivate 20848 | | | | | | | | | | | | | | | | | |
| Deactivate Multiple Isolation Valves | | | | | | | | | | | | | | | | | |
| | 2677.00 | | | | | | TOTAL | | | \$ 890,084.41 | | | | | | \$ 332,380.10 | \$ 1,222,464.51 |

APPENDIX B

POTABLE WATER NETWORK RESERVOIR ASSESSMENT SUMMARY

PROJECT: Whitsundays Water Network Modelling
DOCUMENT NUMBER: D010-10027536-02
DATE: 10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**WHITSUNDAYS POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 500L/EP/d)**

Developed based on WRC Development Manual v1.3 - D5

| Name | ID | Diameter (m) - N3 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N6 | | | | TOTAL - Required Difference (Spare ML) | | | | Name | | | | |
|----------------------------|--------|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|-----------|-----------|--------------|--|--------------|---------------|--------------|---------------|---------------|---------------|--------|--------------------------|
| | | | Min Lvl | Max Lvl | Min Lvl - N5 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | | | | | |
| Proserpine LL | WPGR | 35 | 10.2 | 15.2 | 14.2 | 14.95 | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 4.811 | 3.848 | 0.722 | 11.119 | 11.414 | 11.709 | 11.955 | -6.308 | -6.604 | -6.899 | -7.145 | -7.271 | -7.566 | -7.861 | -8.107 | Proserpine LL |
| Proserpine HL - (Elevated) | WPWT | 9.11 | 35.7 | 42.5 | 38 | 42.11 | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 0.443 | 0.355 | 0.268 | 2.100 | 2.824 | 2.898 | 2.964 | | | | | | | | | Proserpine HL |
| Proserpine Combined - N4 | | | | | | | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 5.254 | 4.203 | 0.989 | | | | | | | | | | | | | Proserpine Combined - N4 |
| Mt Julian 1 | WCGR03 | 11.8 | 76 | 79.3 | 76.5 | 79.1 | | | | | 0.361 | 0.289 | 0.284 | | | | | | | | | | | | | Mt Julian 1 |
| Mt Julian 2 | WCGR15 | 11.8 | 76 | 79.3 | 76.5 | 79.1 | | | | | 0.361 | 0.289 | 0.284 | | | | | | | | | | | | | Mt Julian 2 |
| Mt Julian Combined | | | | | | | 382.74 | 443.61 | 500.68 | 562.57 | 0.722 | 0.577 | 0.569 | 0.581 | 0.649 | 0.713 | 0.783 | 0.141 | 0.073 | 0.009 | -0.061 | -0.003 | -0.072 | -0.136 | -0.205 | Mt Julian Combined |
| Cannon Valley - N2 | T-7 | 39.41 | 89 | 102 | 93 | 101 | 4464.98 | 5175.12 | 5840.87 | 6562.84 | 15.858 | 12.686 | 9.759 | 5.581 | 6.469 | 7.301 | 8.204 | 10.277 | 9.389 | 8.557 | 7.654 | 7.105 | 6.217 | 5.385 | 4.483 | Cannon Valley - N2 |
| Coyne Rd LL | WCGR04 | 26.6 | 69.5 | 73.6 | 71.14 | 73.2 | 2304.65 | 2671.19 | 3014.83 | 3387.48 | 2.278 | 1.823 | 1.145 | 3.093 | 3.505 | 3.892 | 4.311 | -0.814 | -1.227 | -1.613 | -2.032 | -1.270 | -1.682 | -2.069 | -2.488 | Coyne Rd LL |
| Coyne Rd HL | WCGR06 | 8.2 | 93.2 | 95.7 | 93.6 | 95.575 | 290.88 | 337.14 | 380.51 | 427.55 | 0.132 | 0.106 | 0.104 | 0.477 | 0.529 | 0.578 | 0.631 | -0.345 | -0.397 | -0.446 | -0.499 | -0.372 | -0.424 | -0.472 | -0.525 | Coyne Rd HL |
| Cannonvale | WCGR05 | 40 | 70 | 80.2 | 75.1 | 79.18 | 4913.17 | 5694.59 | 6427.17 | 7221.61 | 12.818 | 10.254 | 5.127 | 6.141 | 7.118 | 8.034 | 9.027 | 6.676 | 5.699 | 4.784 | 3.791 | 4.113 | 3.136 | 2.220 | 1.227 | Cannonvale |
| Airlie Summit | T-8 | 13 | 150 | 153 | 150.45 | 152.7 | 180.62 | 209.35 | 236.28 | 265.48 | 0.398 | 0.319 | 0.299 | 0.353 | 0.386 | 0.416 | 0.449 | 0.045 | 0.013 | -0.018 | -0.050 | -0.035 | -0.067 | -0.097 | -0.130 | Airlie Summit |
| Moonlight Dr 1 | WCGR01 | 5.3 | 92 | 97.6 | 93.9 | 97.32 | | | | | 0.124 | 0.099 | 0.075 | | | | | | | | | | | | | Moonlight Dr 1 |
| Moonlight Dr 2 | WCGR02 | 5.3 | 92 | 97.6 | 93.9 | 97.32 | | | | | 0.124 | 0.099 | 0.075 | | | | | | | | | | | | | Moonlight Dr 2 |
| Moonlight Dr Combined | | | | | | | 69.58 | 80.65 | 91.02 | 102.27 | 0.247 | 0.198 | 0.151 | 0.228 | 0.241 | 0.252 | 0.265 | 0.019 | 0.006 | -0.005 | -0.018 | -0.031 | -0.043 | -0.055 | -0.067 | Moonlight Dr Combined |
| Sanctuary Dr (NEW) | T-10 | 5 | 61 | 66 | 62 | 65.5 | | | | | 0.098 | 0.079 | 0.069 | 0.108 | 0.108 | 0.108 | 0.108 | -0.010 | -0.010 | -0.010 | -0.010 | -0.029 | -0.029 | -0.029 | -0.029 | Sanctuary Dr (NEW) |
| Shute Harbour LL | WCGR13 | 11.6 | 28.8 | 32.2 | 31.5 | 32.1 | 3.95 | 4.58 | 5.17 | 5.81 | 0.359 | 0.287 | 0.063 | 0.154 | 0.155 | 0.156 | 0.157 | 0.205 | 0.204 | 0.204 | 0.203 | 0.133 | 0.132 | 0.132 | 0.131 | Shute Harbour LL |
| Shute Harbour HL | WCGR12 | 13.9 | 97.9 | 101.7 | 100.5 | 101.5 | 1176.85 | 1364.02 | 1539.50 | 1729.79 | 0.577 | 0.461 | 0.152 | 1.824 | 2.035 | 2.232 | 2.446 | -1.247 | -1.458 | -1.655 | -1.869 | -1.363 | -1.573 | -1.771 | -1.985 | Shute Harbour HL |
| Daydream - N7 | | | 10 | 15 | 20 | | 798.31 | 925.28 | 1044.31 | 1173.39 | 0.393 | 0.314 | | 0.473 | 0.525 | 0.923 | 0.975 | -0.081 | -0.132 | -0.530 | -0.583 | -0.159 | -0.211 | -0.609 | -0.661 | Daydream |
| Satinwood Ct 1 | WCGR18 | 13.4 | 166.45 | 170 | 169 | 169.8 | | | | | 0.501 | 0.401 | 0.113 | | | | | | | | | | | | | Satinwood Ct 1 |
| Satinwood Ct 2 | WCGR19 | 13.4 | 166.45 | 170 | 169 | 169.8 | | | | | 0.501 | 0.401 | 0.113 | | | | | | | | | | | | | Satinwood Ct 2 |
| Satinwood Combined | | | | | | | 201.16 | 233.15 | 263.15 | 295.67 | 1.001 | 0.801 | 0.226 | 0.376 | 0.412 | 0.446 | 0.483 | 0.625 | 0.589 | 0.555 | 0.519 | 0.425 | 0.389 | 0.355 | 0.318 | Satinwood Combined |
| Micaona Cres | WCGR07 | 9 | 110 | 114.2 | 113.2 | 114 | 178.02 | 206.33 | 232.88 | 261.66 | 0.267 | 0.214 | 0.051 | 0.350 | 0.382 | 0.412 | 0.444 | -0.083 | -0.115 | -0.145 | -0.177 | -0.137 | -0.168 | -0.198 | -0.231 | Micaona Cres |
| Pepperberry Ln 1 | WCGR14 | 7 | 128.7 | 133.3 | 132.61 | 133.07 | | | | | 0.177 | 0.142 | 0.018 | | | | | | | | | | | | | Pepperberry Ln 1 |
| Pepperberry Ln 2 | WCGR17 | 9.83 | 130 | 133.3 | 132.61 | 133.07 | | | | | 0.250 | 0.200 | 0.035 | | | | | | | | | | | | | Pepperberry Ln 2 |
| Pepperberry Combined | | | | | | | 219.16 | 254.02 | 286.69 | 322.13 | 0.427 | 0.342 | 0.053 | 0.397 | 0.436 | 0.473 | 0.512 | 0.031 | -0.008 | -0.045 | -0.085 | -0.055 | -0.094 | -0.131 | -0.170 | Pepperberry Combined |
| Hamilton Park | WB001 | 5 | 89 | 91 | 137 | 137.8 | 50.70 | 58.76 | 66.32 | 74.52 | 0.039 | 0.031 | 0.016 | 0.207 | 0.216 | 0.225 | 0.234 | -0.168 | -0.177 | -0.185 | -0.195 | -0.176 | -0.185 | -0.193 | -0.202 | Hamilton Park |
| TOTAL | | | | | | | | | | | | | | | | | 8.962 | 5.846 | 2.556 | -0.558 | 0.877 | -2.239 | -5.529 | -8.643 | | |

N1 - Factored based on 2021 supply catchments

N2 - EPs supplied = Dedicated supply from Cannon Valley - does not include catchments of reservoirs supplied via Cannon Valley gravity feed

N3 - Diameter assumed as internal tank diameter

N4 - Proserpine LL tank supplies HL via pumpset - combined capacity will require supply redundancy

N5 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points

N6 - Based on 500L/EP/d

N7 - Daydream AD adopted as 180L/EP/d based on discussions with WRC and in line with tourism based water usage

PROJECT: Whitsundays Water Network Modelling
DOCUMENT NUMBER: D010-10027536-02
DATE: 10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**WHITSUNDAYS POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 450L/EP/d)**

Developed based on WRC Development Manual v1.3 - D5

| Name | ID | Diameter (m) - N3 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N6 | | | | TOTAL - Required Difference (Spare ML) | | | | Name | | | | |
|----------------------------|--------|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|--------------|--------------|--------------|--|---------------|---------------|--------|--------|--------|--------|--------|--------------------------|
| | | | Min Lvl | Max Lvl | Min Lvl - N5 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | | | | | |
| Proserpine LL | WPGR | 35 | 10.2 | 15.2 | 14.2 | 14.95 | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 4.811 | 3.848 | 0.722 | 10.007 | 10.273 | 10.538 | 10.760 | -5.197 | -5.462 | -5.728 | -5.949 | -6.159 | -6.424 | -6.690 | -6.911 | Proserpine LL |
| Proserpine HL - (Elevated) | WPWT | 9.11 | 35.7 | 42.5 | 38 | 42.11 | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 0.443 | 0.355 | 0.268 | 1.933 | 2.542 | 2.608 | 2.668 | | | | | | | | | Proserpine HL |
| Proserpine Combined - N4 | | | | | | | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 5.254 | 4.203 | 0.989 | | | | | | | | | | | | | Proserpine Combined - N4 |
| Mt Julian 1 | WCGR03 | 11.8 | 76 | 79.3 | 76.5 | 79.1 | | | | | 0.361 | 0.289 | 0.284 | | | | | | | | | | | | | Mt Julian 1 |
| Mt Julian 2 | WCGR15 | 11.8 | 76 | 79.3 | 76.5 | 79.1 | | | | | 0.361 | 0.289 | 0.284 | | | | | | | | | | | | | Mt Julian 2 |
| Mt Julian Combined | | | | | | | 382.74 | 443.61 | 500.68 | 562.57 | 0.722 | 0.577 | 0.569 | 0.538 | 0.599 | 0.657 | 0.720 | 0.184 | 0.123 | 0.065 | 0.002 | 0.040 | -0.022 | -0.080 | -0.142 | Mt Julian Combined |
| Cannon Valley - N2 | T-7 | 39.41 | 89 | 102 | 93 | 101 | 4464.98 | 5175.12 | 5840.87 | 6562.84 | 15.858 | 12.686 | 9.759 | 5.023 | 5.822 | 6.571 | 7.383 | 10.835 | 10.036 | 9.287 | 8.475 | 7.663 | 6.864 | 6.115 | 5.303 | Cannon Valley - N2 |
| Coyne Rd LL | WCGR04 | 26.6 | 69.5 | 73.6 | 71.14 | 73.2 | 2304.65 | 2671.19 | 3014.83 | 3387.48 | 2.278 | 1.823 | 1.145 | 2.833 | 3.205 | 3.553 | 3.930 | -0.555 | -0.926 | -1.274 | -1.651 | -1.011 | -1.382 | -1.730 | -2.107 | Coyne Rd LL |
| Coyne Rd HL | WCGR06 | 8.2 | 93.2 | 95.7 | 93.6 | 95.575 | 290.88 | 337.14 | 380.51 | 427.55 | 0.132 | 0.106 | 0.104 | 0.445 | 0.491 | 0.535 | 0.583 | -0.312 | -0.359 | -0.403 | -0.451 | -0.339 | -0.386 | -0.430 | -0.477 | Coyne Rd HL |
| Cannonvale | WCGR05 | 40 | 70 | 80.2 | 75.1 | 79.18 | 4913.17 | 5694.59 | 6427.17 | 7221.61 | 12.818 | 10.254 | 5.127 | 5.527 | 6.406 | 7.231 | 8.124 | 7.290 | 6.411 | 5.587 | 4.693 | 4.727 | 3.848 | 3.024 | 2.130 | Cannonvale |
| Airlie Summit | T-8 | 13 | 150 | 153 | 150.45 | 152.7 | 180.62 | 209.35 | 236.28 | 265.48 | 0.398 | 0.319 | 0.299 | 0.333 | 0.362 | 0.389 | 0.419 | 0.065 | 0.036 | 0.009 | -0.021 | -0.014 | -0.043 | -0.071 | -0.100 | Airlie Summit |
| Moonlight Dr 1 | WCGR01 | 5.3 | 92 | 97.6 | 93.9 | 97.32 | | | | | 0.124 | 0.099 | 0.075 | | | | | | | | | | | | | Moonlight Dr 1 |
| Moonlight Dr 2 | WCGR02 | 5.3 | 92 | 97.6 | 93.9 | 97.32 | | | | | 0.124 | 0.099 | 0.075 | | | | | | | | | | | | | Moonlight Dr 2 |
| Moonlight Dr Combined | | | | | | | 69.58 | 80.65 | 91.02 | 102.27 | 0.247 | 0.198 | 0.151 | 0.220 | 0.232 | 0.242 | 0.254 | 0.027 | 0.015 | 0.005 | -0.006 | -0.023 | -0.034 | -0.044 | -0.056 | Moonlight Dr Combined |
| Sanctuary Dr (NEW) | T-10 | 5 | 61 | 66 | 62 | 65.5 | | | | | 0.098 | 0.079 | 0.069 | 0.108 | 0.108 | 0.108 | 0.108 | -0.010 | -0.010 | -0.010 | -0.010 | -0.029 | -0.029 | -0.029 | -0.029 | Sanctuary Dr (NEW) |
| Shute Harbour LL | WCGR13 | 11.6 | 28.8 | 32.2 | 31.5 | 32.1 | 3.95 | 4.58 | 5.17 | 5.81 | 0.359 | 0.287 | 0.063 | 0.154 | 0.155 | 0.155 | 0.156 | 0.205 | 0.205 | 0.204 | 0.203 | 0.133 | 0.133 | 0.132 | 0.132 | Shute Harbour LL |
| Shute Harbour HL | WCGR12 | 13.9 | 97.9 | 101.7 | 100.5 | 101.5 | 1176.85 | 1364.02 | 1539.50 | 1729.79 | 0.577 | 0.461 | 0.152 | 1.692 | 1.881 | 2.059 | 2.251 | -1.115 | -1.304 | -1.482 | -1.675 | -1.230 | -1.420 | -1.597 | -1.790 | Shute Harbour HL |
| Daydream - N7 | T-4 | 10 | 15 | 20 | | | 798.31 | 925.28 | 1044.31 | 1173.39 | 0.393 | 0.314 | | 0.473 | 0.525 | 0.923 | 0.975 | -0.081 | -0.132 | -0.530 | -0.583 | -0.159 | -0.211 | -0.609 | -0.661 | Daydream |
| Satinwood Ct 1 | WCGR18 | 13.4 | 166.45 | 170 | 169 | 169.8 | | | | | 0.501 | 0.401 | 0.113 | | | | | | | | | | | | | Satinwood Ct 1 |
| Satinwood Ct 2 | WCGR19 | 13.4 | 166.45 | 170 | 169 | 169.8 | | | | | 0.501 | 0.401 | 0.113 | | | | | | | | | | | | | Satinwood Ct 2 |
| Satinwood Combined | | | | | | | 201.16 | 233.15 | 263.15 | 295.67 | 1.001 | 0.801 | 0.226 | 0.354 | 0.386 | 0.416 | 0.449 | 0.648 | 0.615 | 0.585 | 0.552 | 0.447 | 0.415 | 0.385 | 0.352 | Satinwood Combined |
| Macona Cres | WCGR07 | 9 | 110 | 114.2 | 113.2 | 114 | 178.02 | 206.33 | 232.88 | 261.66 | 0.267 | 0.214 | 0.051 | 0.330 | 0.359 | 0.386 | 0.415 | -0.063 | -0.092 | -0.119 | -0.148 | -0.116 | -0.145 | -0.172 | -0.201 | Macona Cres |
| Pepperberry Ln 1 | WCGR14 | 7 | 128.7 | 133.3 | 132.61 | 133.07 | | | | | 0.177 | 0.142 | 0.018 | | | | | | | | | | | | | Pepperberry Ln 1 |
| Pepperberry Ln 2 | WCGR17 | 9.83 | 130 | 133.3 | 132.61 | 133.07 | | | | | 0.250 | 0.200 | 0.035 | | | | | | | | | | | | | Pepperberry Ln 2 |
| Pepperberry Combined | | | | | | | 219.16 | 254.02 | 286.69 | 322.13 | 0.427 | 0.342 | 0.053 | 0.372 | 0.407 | 0.440 | 0.476 | 0.056 | 0.020 | -0.013 | -0.049 | -0.030 | -0.065 | -0.098 | -0.134 | Pepperberry Combined |
| Hamilton Park | WB001 | 5 | 89 | 91 | 137 | 137.8 | 50.70 | 58.76 | 66.32 | 74.52 | 0.039 | 0.031 | 0.016 | 0.201 | 0.209 | 0.217 | 0.225 | -0.162 | -0.170 | -0.178 | -0.186 | -0.170 | -0.178 | -0.186 | -0.194 | Hamilton Park |
| TOTAL | | | | | | | | | | | | | 11.815 | 9.006 | 6.005 | 3.197 | 3.730 | 0.921 | -2.080 | -4.888 | | | | | | |

N1 - Factored based on 2021 supply catchments

N2 - EPs supplied = Dedicated supply from Cannon Valley - does not include catchments of reservoirs supplied via Cannon Valley gravity feed

N3 - Diameter assumed as internal tank diameter

N4 - Proserpine LL tank supplies HL via pumpset - combined capacity will require supply redundancy

N5 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points

N6 - Based on 450L/EP/d

N7 - Daydream AD adopted as 180L/EP/d based on discussions with WRC and in line with tourism based water usage

PROJECT: Whitsundays Water Network Modelling
 DOCUMENT NUMBER: D010-10027536-02
 DATE: 10.02.2020

PROJECT ENGINEER: M.C/S.H
 SOFTWARE: Bentley WaterCAD v8i



**WHITSUNDAYS POTABLE WATER NETWORK
 2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 350L/EP/d)**

Developed based on WRC Development Manual v1.3 - D5

| Name | ID | Diameter (m) - N3 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N6 | | | | TOTAL - Required Difference (Spare ML) | | | | Name | | | | |
|----------------------------|--------|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|-----------|-----------|-----------|--|--------|--------|--------|--------|--------|--------|--------|--------------------------|
| | | | Min Lvl | Max Lvl | Min Lvl - N5 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | | | | | |
| Proserpine LL | WPGR | 35 | 10.2 | 15.2 | 14.2 | 14.95 | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 4.811 | 3.848 | 0.722 | 7.783 | 7.990 | 8.197 | 8.369 | -2.973 | -3.179 | -3.386 | -3.558 | -3.935 | -4.141 | -4.348 | -4.520 | Proserpine LL |
| Proserpine HL - (Elevated) | WPWT | 9.11 | 35.7 | 42.5 | 38 | 42.11 | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 0.443 | 0.355 | 0.268 | 1.599 | 1.977 | 2.028 | 2.075 | | | | | | | | | Proserpine HL |
| Proserpine Combined - N4 | | | | | | | 8895.20 | 9131.36 | 9367.51 | 9564.31 | 5.254 | 4.203 | 0.989 | | | | | | | | | | | | | Proserpine Combined - N4 |
| Mt Julian 1 | WCGR03 | 11.8 | 76 | 79.3 | 76.5 | 79.1 | | | | | 0.361 | 0.289 | 0.284 | | | | | | | | | | | | | Mt Julian 1 |
| Mt Julian 2 | WCGR15 | 11.8 | 76 | 79.3 | 76.5 | 79.1 | | | | | 0.361 | 0.289 | 0.284 | | | | | | | | | | | | | Mt Julian 2 |
| Mt Julian Combined | | | | | | | 382.74 | 443.61 | 500.68 | 562.57 | 0.722 | 0.577 | 0.569 | 0.451 | 0.499 | 0.544 | 0.593 | 0.270 | 0.222 | 0.177 | 0.129 | 0.126 | 0.078 | 0.033 | -0.016 | Mt Julian Combined |
| Cannon Valley - N2 | T-7 | 39.41 | 89 | 102 | 93 | 101 | 4464.98 | 5175.12 | 5840.87 | 6562.84 | 15.858 | 12.686 | 9.759 | 4.016 | 4.575 | 5.111 | 5.742 | 11.842 | 11.283 | 10.747 | 10.115 | 8.670 | 8.111 | 7.576 | 6.944 | Cannon Valley - N2 |
| Coyne Rd LL | WCGR04 | 26.6 | 69.5 | 73.6 | 71.14 | 73.2 | 2304.65 | 2671.19 | 3014.83 | 3387.48 | 2.278 | 1.823 | 1.145 | 2.315 | 2.604 | 2.874 | 3.168 | -0.036 | -0.325 | -0.596 | -0.889 | -0.492 | -0.781 | -1.051 | -1.345 | Coyne Rd LL |
| Coyne Rd HL | WCGR06 | 8.2 | 93.2 | 95.7 | 93.6 | 95.575 | 290.88 | 337.14 | 380.51 | 427.55 | 0.132 | 0.106 | 0.104 | 0.379 | 0.416 | 0.450 | 0.487 | -0.247 | -0.283 | -0.318 | -0.355 | -0.273 | -0.310 | -0.344 | -0.381 | Coyne Rd HL |
| Cannonvale | WCGR05 | 40 | 70 | 80.2 | 75.1 | 79.18 | 4913.17 | 5694.59 | 6427.17 | 7221.61 | 12.818 | 10.254 | 5.127 | 4.369 | 4.984 | 5.624 | 6.319 | 8.449 | 7.833 | 7.194 | 6.499 | 5.885 | 5.270 | 4.630 | 3.935 | Cannonvale |
| Airlie Summit | T-8 | 13 | 150 | 153 | 150.45 | 152.7 | 180.62 | 209.35 | 236.28 | 265.48 | 0.398 | 0.319 | 0.299 | 0.292 | 0.315 | 0.336 | 0.359 | 0.106 | 0.083 | 0.062 | 0.039 | 0.026 | 0.004 | -0.018 | -0.041 | Airlie Summit |
| Moonlight Dr 1 | WCGR01 | 5.3 | 92 | 97.6 | 93.9 | 97.32 | | | | | 0.124 | 0.099 | 0.075 | | | | | | | | | | | | | Moonlight Dr 1 |
| Moonlight Dr 2 | WCGR02 | 5.3 | 92 | 97.6 | 93.9 | 97.32 | | | | | 0.124 | 0.099 | 0.075 | | | | | | | | | | | | | Moonlight Dr 2 |
| Moonlight Dr Combined | | | | | | | 69.58 | 80.65 | 91.02 | 102.27 | 0.247 | 0.198 | 0.151 | 0.205 | 0.214 | 0.222 | 0.231 | 0.042 | 0.034 | 0.025 | 0.017 | -0.007 | -0.016 | -0.024 | -0.033 | Moonlight Dr Combined |
| Sanctuary Dr (NEW) | T-10 | 5 | 61 | 66 | 62 | 65.5 | | | | | 0.098 | 0.079 | 0.069 | 0.108 | 0.108 | 0.108 | 0.108 | -0.010 | -0.010 | -0.010 | -0.010 | -0.029 | -0.029 | -0.029 | -0.029 | Sanctuary Dr (NEW) |
| Shute Harbour LL | WCGR13 | 11.6 | 28.8 | 32.2 | 31.5 | 32.1 | 3.95 | 4.58 | 5.17 | 5.81 | 0.359 | 0.287 | 0.063 | 0.153 | 0.154 | 0.154 | 0.155 | 0.206 | 0.206 | 0.205 | 0.205 | 0.134 | 0.134 | 0.133 | 0.133 | Shute Harbour LL |
| Shute Harbour HL | WCGR12 | 13.9 | 97.9 | 101.7 | 100.5 | 101.5 | 1176.85 | 1364.02 | 1539.50 | 1729.79 | 0.577 | 0.461 | 0.152 | 1.427 | 1.574 | 1.712 | 1.862 | -0.850 | -0.998 | -1.136 | -1.286 | -0.965 | -1.113 | -1.251 | -1.401 | Shute Harbour HL |
| Daydream - N7 | T-4 | 10 | 15 | 20 | | | 798.31 | 925.28 | 1044.31 | 1173.39 | 0.393 | 0.314 | | 0.473 | 0.525 | 0.923 | 0.975 | -0.081 | -0.132 | -0.530 | -0.583 | -0.159 | -0.211 | -0.609 | -0.661 | Daydream |
| Satinwood Ct 1 | WCGR18 | 13.4 | 166.45 | 170 | 169 | 169.8 | | | | | 0.501 | 0.401 | 0.113 | | | | | | | | | | | | | Satinwood Ct 1 |
| Satinwood Ct 2 | WCGR19 | 13.4 | 166.45 | 170 | 169 | 169.8 | | | | | 0.501 | 0.401 | 0.113 | | | | | | | | | | | | | Satinwood Ct 2 |
| Satinwood Combined | | | | | | | 201.16 | 233.15 | 263.15 | 295.67 | 1.001 | 0.801 | 0.226 | 0.308 | 0.334 | 0.357 | 0.383 | 0.693 | 0.668 | 0.644 | 0.618 | 0.493 | 0.467 | 0.444 | 0.418 | Satinwood Combined |
| Macona Cres | WCGR07 | 9 | 110 | 114.2 | 113.2 | 114 | 178.02 | 206.33 | 232.88 | 261.66 | 0.267 | 0.214 | 0.051 | 0.290 | 0.312 | 0.333 | 0.356 | -0.023 | -0.045 | -0.066 | -0.089 | -0.076 | -0.099 | -0.120 | -0.142 | Macona Cres |
| Pepperberry Ln 1 | WCGR14 | 7 | 128.7 | 133.3 | 132.61 | 133.07 | | | | | 0.177 | 0.142 | 0.018 | | | | | | | | | | | | | Pepperberry Ln 1 |
| Pepperberry Ln 2 | WCGR17 | 9.83 | 130 | 133.3 | 132.61 | 133.07 | | | | | 0.250 | 0.200 | 0.035 | | | | | | | | | | | | | Pepperberry Ln 2 |
| Pepperberry Combined | | | | | | | 219.16 | 254.02 | 286.69 | 322.13 | 0.427 | 0.342 | 0.053 | 0.323 | 0.350 | 0.376 | 0.404 | 0.105 | 0.077 | 0.052 | 0.024 | 0.019 | -0.008 | -0.034 | -0.062 | Pepperberry Combined |
| Hamilton Park | WB001 | 5 | 89 | 91 | 137 | 137.8 | 50.70 | 58.76 | 66.32 | 74.52 | 0.039 | 0.031 | 0.016 | 0.190 | 0.196 | 0.202 | 0.209 | -0.151 | -0.157 | -0.163 | -0.169 | -0.159 | -0.165 | -0.171 | -0.177 | Hamilton Park |
| | | | | | | | | | | | | | TOTAL | 17.342 | 15.276 | 12.903 | 10.707 | 9.257 | 7.191 | 4.818 | 2.622 | | | | | |

N1 - Factored based on 2021 supply catchments

N2 - EPs supplied = Dedicated supply from Cannon Valley - does not include catchments of reservoirs supplied via Cannon Valley gravity feed

N3 - Diameter assumed as internal tank diameter

N4 - Proserpine LL tank supplies HL via pumpset - combined capacity will require supply redundancy

N5 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points

N6 - Based on 350L/EP/d

N7 - Daydream AD adopted as 180L/EP/d based on discussions with WRC and in line with tourism based water usage

PROJECT: Bowen Water Network Modelling
DOCUMENT NUMBER: D011-10027536-01
DATE: 10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**BOWEN POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 500L/EP/d)**

| Name | ID | Diameter (m) - N2 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N4 | | | | TOTAL - Required Difference (Spare ML) | | | | Name | | | | | | | | | |
|----------------|------|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|-----------|-----------|--------------|--|--------------|--------------|--------------|--------------|--------------|--------------|-------|----------------|--|--|--|--|--|
| | | | Min Lvl | Max Lvl | Min Lvl - N3 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | | 2021 | 2026 | 2031 | 2036 | | | | | |
| Heronvale | 1494 | 15 | 39 | 44 | 39.75 | 43.75 | 197.10 | 199.96 | 207.77 | 210.72 | 0.884 | 0.707 | 0.707 | 0.654 | 0.657 | 0.666 | 0.669 | 0.230 | 0.227 | 0.218 | 0.215 | 0.053 | 0.050 | 0.041 | 0.038 | Heronvale | | | | | |
| Bowen Main Res | 6963 | 43.5 | 48 | 59.5 | 49.7 | 58.925 | 9019.32 | 9513.51 | 10088.76 | 10581.60 | 17.091 | 13.673 | 13.710 | 11.274 | 11.892 | 12.611 | 13.227 | 5.817 | 5.199 | 4.480 | 3.864 | 2.399 | 1.781 | 1.062 | 0.446 | Bowen Main Res | | | | | |
| TOTAL | | | | | | | | | | | | | | | | | 6.047 | 5.426 | 4.698 | 4.078 | 2.452 | 1.831 | 1.103 | 0.484 | | | | | | | |

N1 - Factored based on 2021 supply catchments
N2 - Diameter assumed as internal tank diameter
N3 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points
N4 - Based on 500L/EP/d

PROJECT: Bowen Water Network Modelling
DOCUMENT NUMBER: D011-10027536-01
DATE: 10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**BOWEN POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 450L/EP/d)**

| Name | ID | Diameter (m) - N2 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N4 | | | | TOTAL - Required Difference (Spare ML) | | | | Name | | | | |
|----------------|------|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|--------------|--------------|--------------|--|--------------|--------------|-------|-------|-------|-------|-------|----------------|
| | | | Min Lvl | Max Lvl | Min Lvl - N3 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | | 2021 | 2026 | 2031 | 2036 |
| Heronvale | 1494 | 14.5 | 39 | 44 | 39.75 | 43.75 | 197.10 | 199.96 | 207.77 | 210.72 | 0.826 | 0.661 | 0.661 | 0.632 | 0.634 | 0.642 | 0.645 | 0.194 | 0.191 | 0.183 | 0.180 | 0.029 | 0.026 | 0.018 | 0.015 | Heronvale |
| Bowen Main Res | 6963 | 43.5 | 48 | 59.5 | 49.7 | 58.925 | 9019.32 | 9513.51 | 10088.76 | 10581.60 | 17.091 | 13.673 | 13.710 | 10.147 | 10.703 | 11.350 | 11.904 | 6.944 | 6.388 | 5.741 | 5.187 | 3.526 | 2.970 | 2.323 | 1.768 | Bowen Main Res |
| TOTAL | | | | | | | | | | | | | 7.138 | 6.579 | 5.924 | 5.367 | 3.555 | 2.996 | 2.341 | 1.784 | | | | | | |

N1 - Factored based on 2021 supply catchments
N2 - Diameter assumed as internal tank diameter
N3 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points
N4 - Based on 450L/EP/d

PROJECT: Bowen Water Network Modelling
DOCUMENT NUMBER: D011-10027536-01
DATE: 10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**BOWEN POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 350L/EP/d)**

| Name | ID | Diameter (m) - N2 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N4 | | | | TOTAL - Required Difference (Spare ML) | | | | Name | | | | | |
|----------------|------|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|--------------|--------------|--------------|--|--------------|--------------|-------|-------|-------|-------|-------|----------------|--|
| | | | Min Lvl | Max Lvl | Min Lvl - N3 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | | | | | | |
| Heronvale | 1494 | 14 | 39 | 44 | 39.75 | 43.75 | 197.10 | 199.96 | 207.77 | 210.72 | 0.770 | 0.616 | 0.616 | 0.587 | 0.589 | 0.596 | 0.598 | 0.182 | 0.180 | 0.174 | 0.172 | 0.029 | 0.026 | 0.020 | 0.018 | Heronvale | |
| Bowen Main Res | 6963 | 43.5 | 48 | 59.5 | 49.7 | 58.925 | 9019.32 | 9513.51 | 10088.76 | 10581.60 | 17.091 | 13.673 | 13.710 | 7.892 | 8.324 | 8.828 | 9.259 | 9.199 | 8.767 | 8.263 | 7.832 | 5.781 | 5.348 | 4.845 | 4.414 | Bowen Main Res | |
| TOTAL | | | | | | | | | | | | | 9.382 | 8.947 | 8.437 | 8.004 | 5.809 | 5.375 | 4.865 | 4.432 | | | | | | | |

N1 - Factored based on 2021 supply catchments
N2 - Diameter assumed as internal tank diameter
N3 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points
N4 - Based on 350L/EP/d

PROJECT:
DOCUMENT NUMBER:
DATE:

Collinsville Water Network Modelling
D012-10027536-01
10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**COLLINSVILLE POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 500L/EP/d)**

| Name | ID | Diameter (m) - N3 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N5 | | | | TOTAL - Required Difference (Spare ML) | | | | 15%-95% Operational - Required Difference (Spare ML) | | | | Name |
|------------------------------------|-----|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|-----------|-----------|-----------|--|--------|--------|--------|--|--------|--------|--------|------------------------------------|
| | | | Min Lvl | Max Lvl | Min Lvl - N4 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | 2021 | 2026 | 2031 | 2036 | |
| 6ML Tank - Peter Delemothe Rd - N2 | 528 | 36 | 254 | 260 | 254.9 | 259.9 | 3689.98 | 2002.90 | 2067.26 | 2155.79 | 6.107 | 4.886 | 5.089 | 4.612 | 2.685 | 2.758 | 2.857 | 1.495 | 3.422 | 3.350 | 3.250 | 0.273 | 2.201 | 2.128 | 2.029 | 6ML Tank - Peter Delemothe Rd - N2 |
| 1.136ML Tank - Miller St | 529 | 18.3 | 232.1 | 236.6 | 232.1 | 236.375 | | | | | 1.184 | 0.947 | 1.124 | | | | | | | | | | | | | 1.136ML Tank - Miller St |
| 454kL Tank - Miller St | 530 | 12.8 | 233.25 | 236.6 | 233.25 | 236.433 | | | | | 0.431 | 0.345 | 0.410 | | | | | | | | | | | | | 454kL Tank - Miller St |
| Miller St Combined | | | | | | | 0.00 | 1707.05 | 1762.71 | 1784.16 | 1.615 | 1.292 | 1.534 | 0.150 | 2.420 | 2.483 | 2.507 | 1.465 | -0.806 | -0.868 | -0.893 | 1.142 | -1.129 | -1.191 | -1.215 | Miller St Combined |
| | | | | | | | | | | | | | TOTAL | | | | | 2.939 | 2.616 | 2.481 | 2.257 | 1.415 | 1.072 | 0.937 | 0.813 | |

N1 - Factored based on 2021 supply catchments
N2 - EPs supplied - Dedicated supply from Peter Delemothe Rd reservoir - does not include catchments of reservoirs supplied via Peter Delemothe Rd gravity feed
N3 - Diameter assumed as internal tank diameter
N4 - Operational min level model extract - BWL may be defined higher than 15% tank volume in some instances to meet minimum pressure requirements at supply demand points
N5 - Based on 500L/EP/d

PROJECT:
DOCUMENT NUMBER:
DATE:

Collinsville Water Network Modelling
D012-10027536-01
10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



**COLLINSVILLE POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 450L/EP/d)**

| Name | ID | Diameter (m) - N3 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | | | Actual Operational Volume (ML) | Required Storage Volume (ML) - N5 | | | | TOTAL - Required Difference (Spare ML) | | | | 15%-95% Operational - Required Difference (Spare ML) | | | | Name | | |
|------------------------------------|-----|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|-----------|-----------|--------------------------------|-----------------------------------|--------------|--------------|--------------|--|--------------|--------------|--------------|--|--------|------------------------------------|--|------|--|--|
| | | | Min Lvl | Max Lvl | Min Lvl - N4 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | 2021 | 2026 - N1 | 2031 - N1 | | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | 2021 | 2026 | 2031 | 2036 | | | | | | |
| 6ML Tank - Peter Delemothe Rd - N2 | 528 | 36 | 254 | 260 | 254.9 | 259.9 | 3689.98 | 2002.90 | 2067.26 | 2155.79 | 6.107 | 4.886 | 5.089 | 4.168 | 2.460 | 2.525 | 2.615 | 1.939 | 3.647 | 3.582 | 3.493 | 0.718 | 2.426 | 2.361 | 2.271 | 6ML Tank - Peter Delemothe Rd - N2 | | | | |
| 1.136ML Tank - Miller St | 529 | 18.3 | 232.1 | 236.6 | 232.1 | 236.375 | | | | | 1.184 | 0.947 | 1.124 | | | | | | | | | | | | | 1.136ML Tank - Miller St | | | | |
| 454kL Tank - Miller St | 530 | 12.8 | 233.25 | 236.6 | 233.25 | 236.433 | | | | | 0.431 | 0.345 | 0.410 | | | | | | | | | | | | | 454kL Tank - Miller St | | | | |
| Miller St Combined | | | | | | | 0.00 | 1707.05 | 1762.71 | 1784.16 | 1.615 | 1.292 | 1.534 | 0.150 | 2.228 | 2.285 | 2.306 | 1.465 | -0.614 | -0.670 | -0.692 | 1.142 | -0.937 | -0.993 | -1.015 | Miller St Combined | | | | |
| TOTAL | | | | | | | | | | | | | | | | 3.404 | 3.034 | 2.912 | 2.801 | 1.859 | 1.489 | 1.368 | 1.256 | | | | | | | |

N1 - Factored based on 2021 supply catchments
N2 - EPs supplied - Dedicated supply from Peter Delemothe Rd reservoir - does not include catchments of reservoirs supplied via Peter Delemothe Rd gravity feed
N3 - Diameter assumed as internal tank diameter
N4 - Operational min level model extract - may be defined in some instances to meet minimum pressure requirements at supply demand points
N5 - Based on 450L/EP/d

PROJECT:
DOCUMENT NUMBER:
DATE:

Collinsville Water Network Modelling
D012-10027536-01
10.02.2020

PROJECT ENGINEER: M.C/S.H
SOFTWARE: Bentley WaterCAD v8i



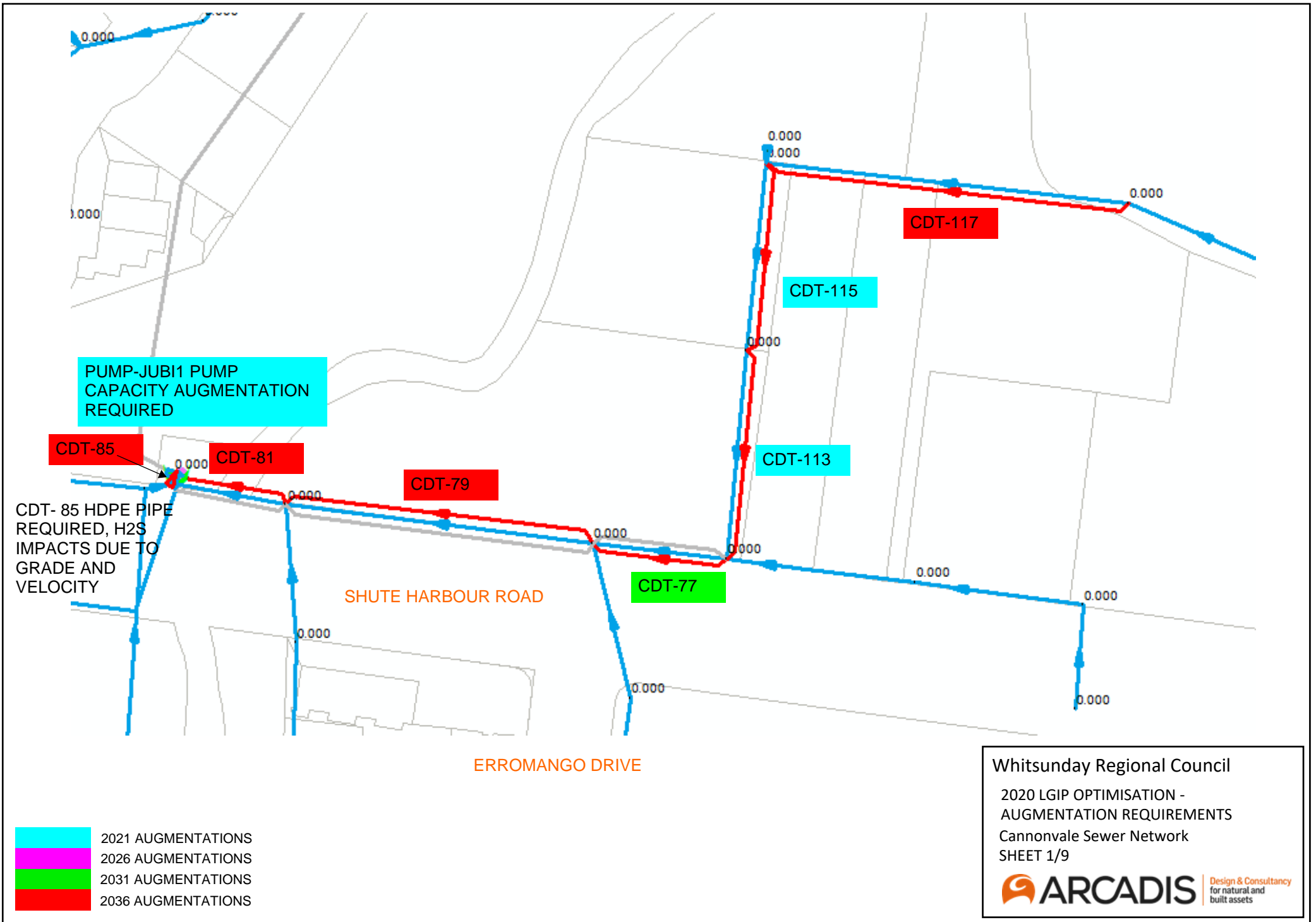
**COLLINSVILLE POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - RESERVOIR CAPACITY ASSESSMENT (AD = 350L/EP/d)**

| Name | ID | Diameter (m) - N3 | Physical | | Operational | | EPs Supplied | | | | Total Volume (ML) | 15%-95% Operational Storage Volume (ML) | Actual Operational Volume (ML) | Required Storage Volume (ML) - N5 | | | | TOTAL - Required Difference (Spare ML) | | | | 15%-95% Operational - Required Difference (Spare ML) | | | | Name | | | | | | | | |
|------------------------------------|-----|-------------------|----------|---------|--------------|---------|--------------|-----------|-----------|-----------|-------------------|---|--------------------------------|-----------------------------------|--------------|--------------|--------------|--|--------------|--------------|--------|--|--------|--------|--------|------------------------------------|--|--|--|--|--|--|--|--|
| | | | Min Lvl | Max Lvl | Min Lvl - N4 | Max Lvl | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | | | | 2021 | 2026 - N1 | 2031 - N1 | 2036 - N1 | 2021 | 2026 | 2031 | 2036 | 2021 | 2026 | 2031 | 2036 | | | | | | | | | |
| 6ML Tank - Peter Delemothe Rd - N2 | 528 | 36 | 254 | 260 | 254.9 | 259.9 | 3689.98 | 2002.90 | 2067.26 | 2155.79 | 6.107 | 4.886 | 5.089 | 3.338 | 2.009 | 2.060 | 2.130 | 2.769 | 4.098 | 4.047 | 3.978 | 1.548 | 2.877 | 2.826 | 2.756 | 6ML Tank - Peter Delemothe Rd - N2 | | | | | | | | |
| 1.136ML Tank - Miller St | 529 | 18.3 | 232.1 | 236.6 | 232.1 | 236.375 | | | | | 1.184 | 0.947 | 1.124 | | | | | | | | | | | | | 1.136ML Tank - Miller St | | | | | | | | |
| 454kL Tank - Miller St | 530 | 12.8 | 233.25 | 236.6 | 233.25 | 236.433 | | | | | 0.431 | 0.345 | 0.410 | | | | | | | | | | | | | 454kL Tank - Miller St | | | | | | | | |
| Miller St Combined | | | | | | | 0.00 | 1707.05 | 1762.71 | 1784.16 | 1.615 | 1.292 | 1.534 | 0.150 | 1.844 | 1.888 | 1.905 | 1.465 | -0.230 | -0.273 | -0.290 | 1.142 | -0.553 | -0.596 | -0.613 | Miller St Combined | | | | | | | | |
| TOTAL | | | | | | | | | | | | | 4.234 | 3.868 | 3.774 | 3.607 | 2.690 | 2.324 | 2.229 | 2.143 | | | | | | | | | | | | | | |

N1 - Factored based on 2021 supply catchments
N2 - EPs supplied - Dedicated supply from Peter Delemothe Rd reservoir - does not include catchments of reservoirs supplied via Peter Delemothe Rd gravity feed
N3 - Diameter assumed as internal tank diameter
N4 - Operational min level model extract - may be defined in some instances to meet minimum pressure requirements at supply demand points
N5 - Based on 350L/EP/d

APPENDIX C

SEWER NETWORK AUGMENTATION MAPPING AND SUMMARY



- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 1/9

ARCADIS Design & Consultancy for natural and built assets

PUMP-CANN14 PUMP
CAPACITY AUGMENTATION
REQUIRED

BROADWATER DRIVE

CDT-167

SHUTE HARBOUR ROAD

CDT-159

CDT-157

CDT-155

CDT-153

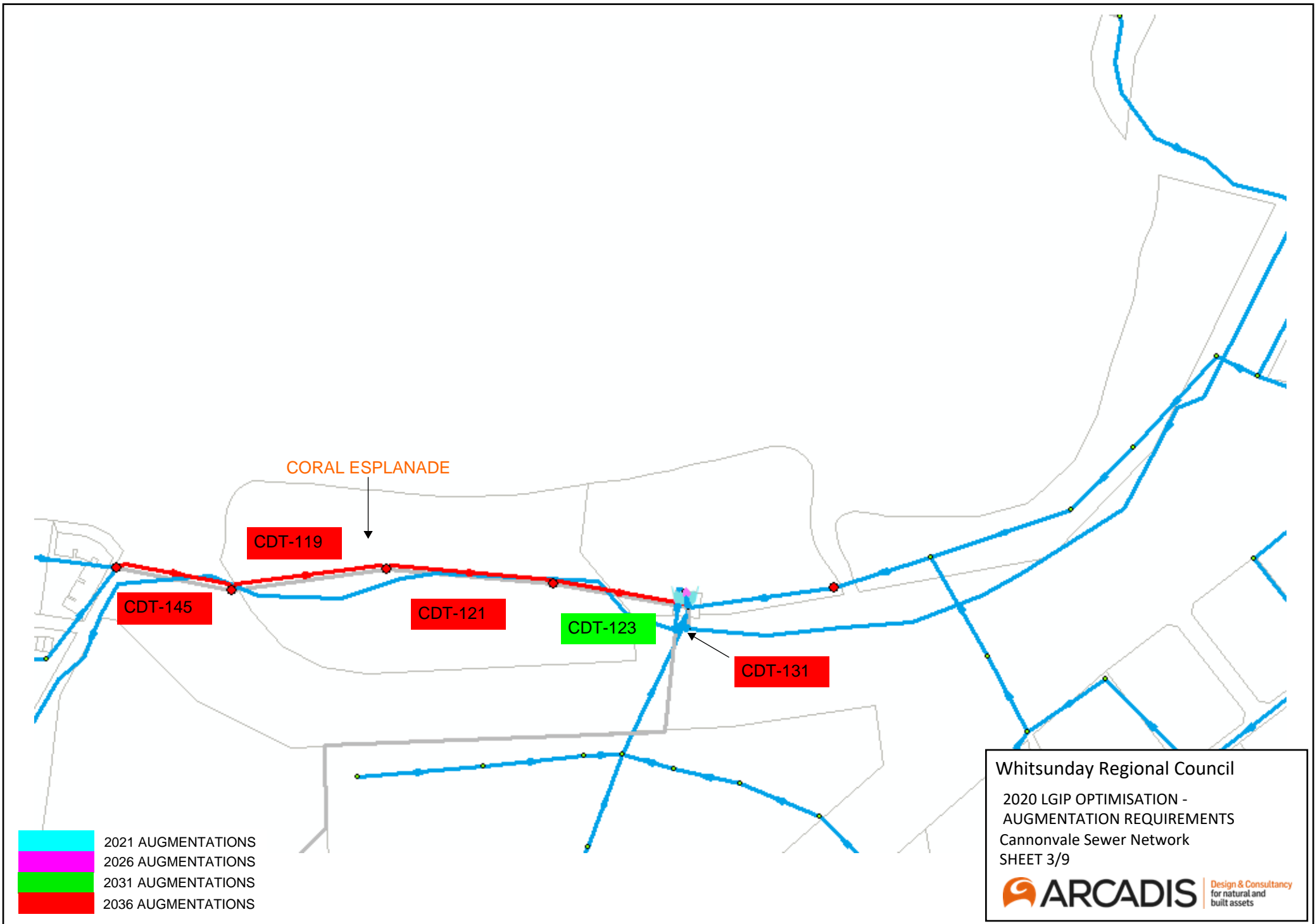
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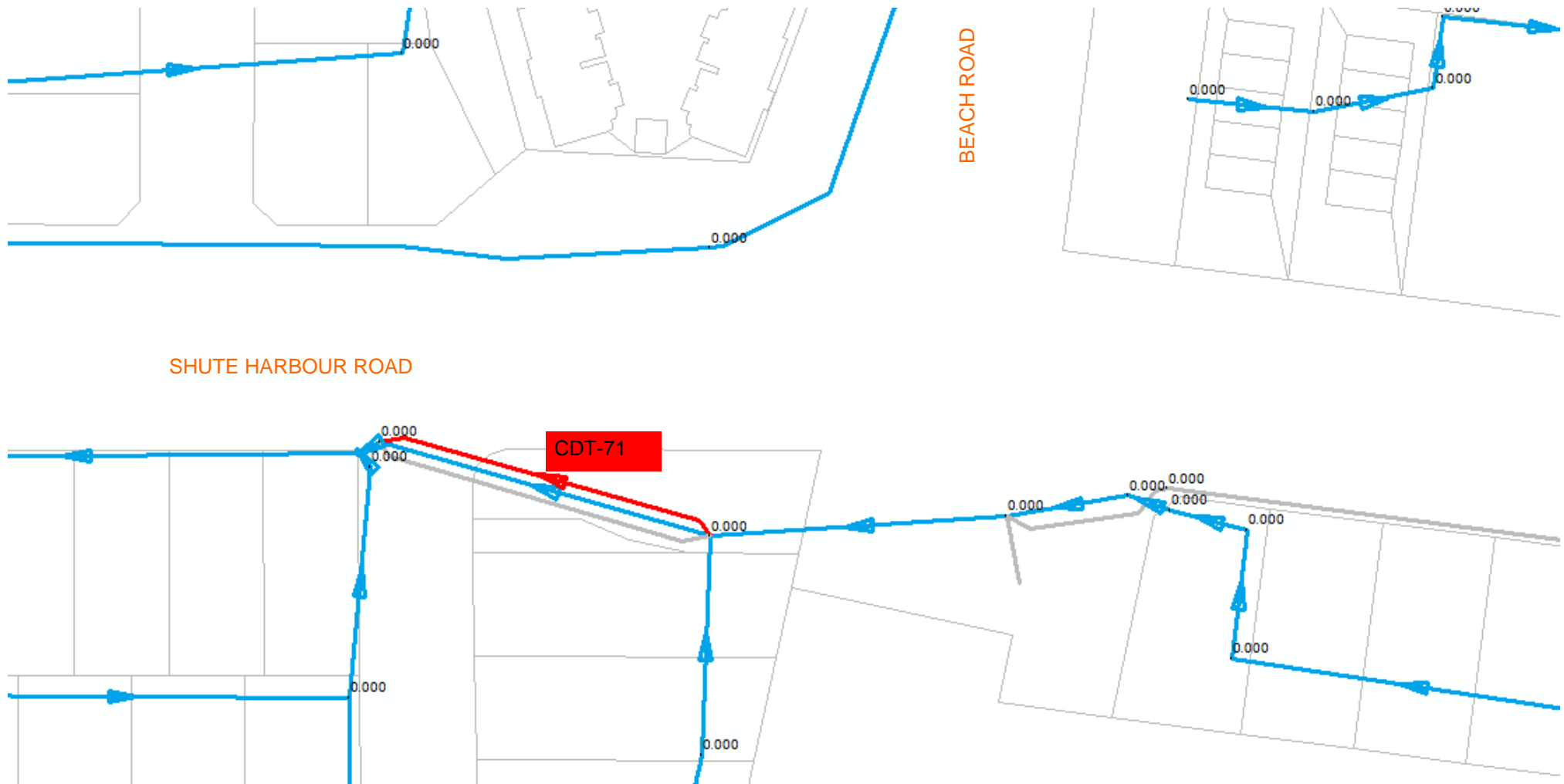
CDT-149

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS
Cannonvale Sewer Network
SHEET 2/9





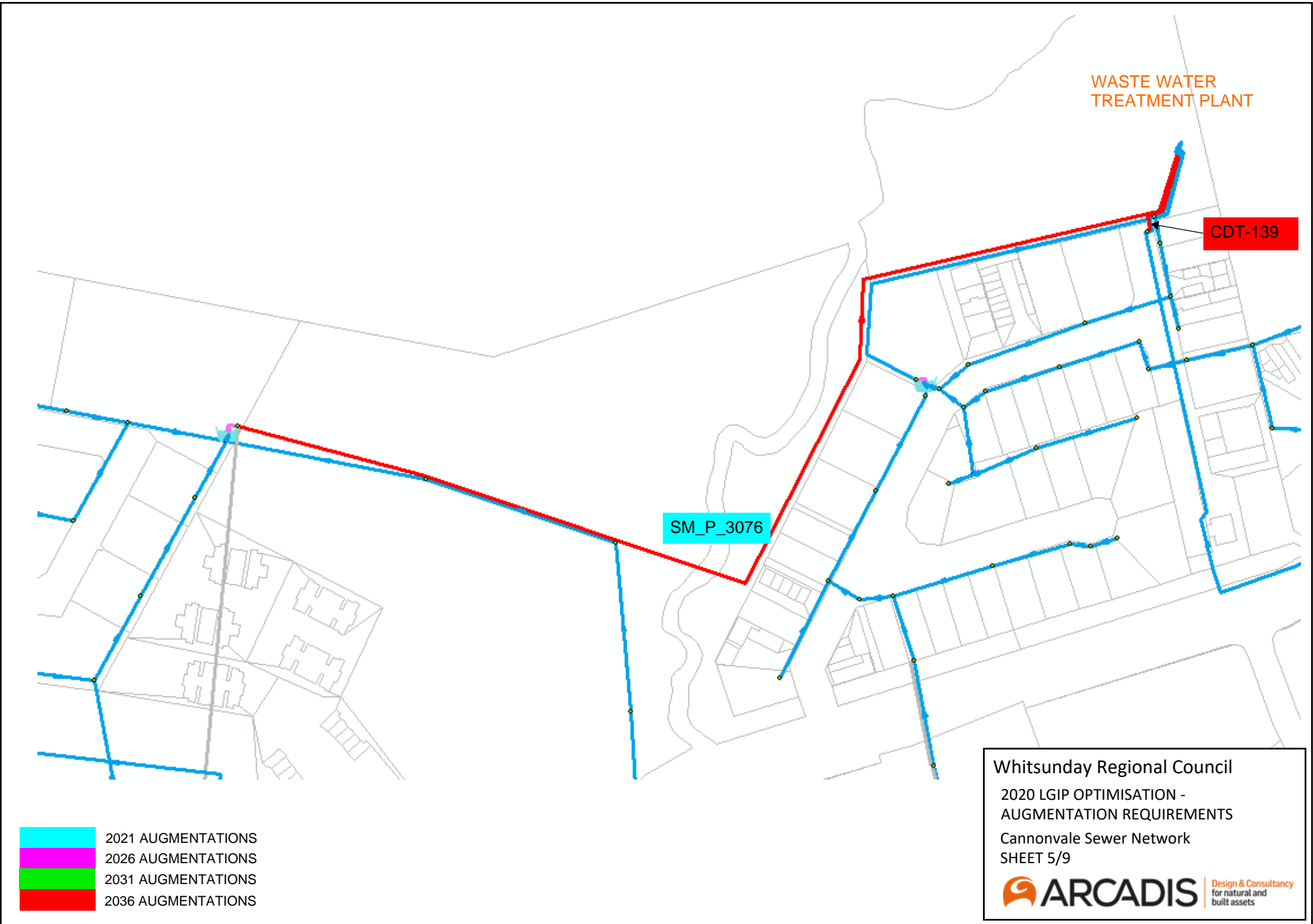


- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 4/9



ARCADIS | Design & Consultancy
 for natural and built assets



WASTE WATER TREATMENT PLANT

CDT-139

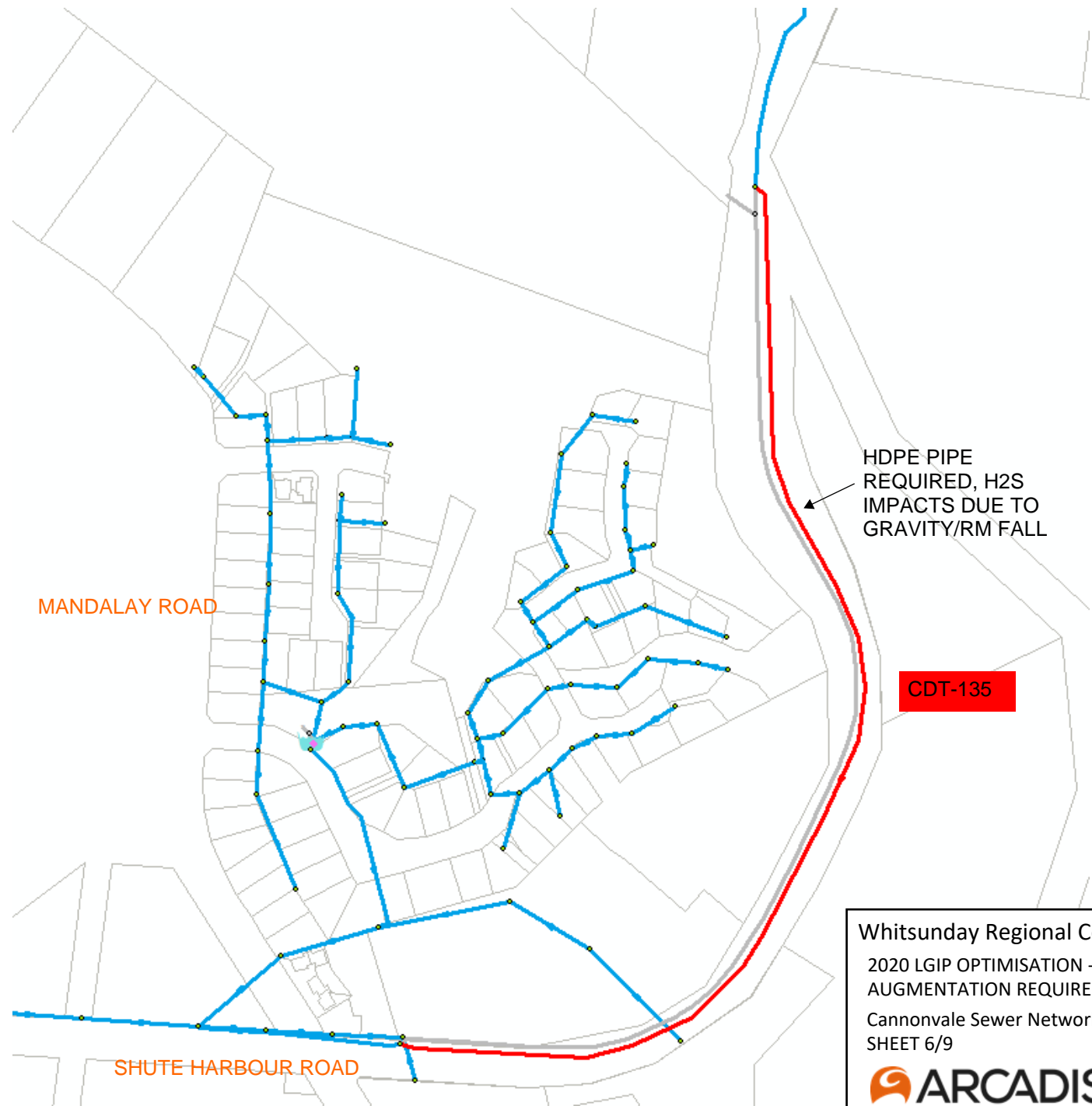
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- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 5/9



- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

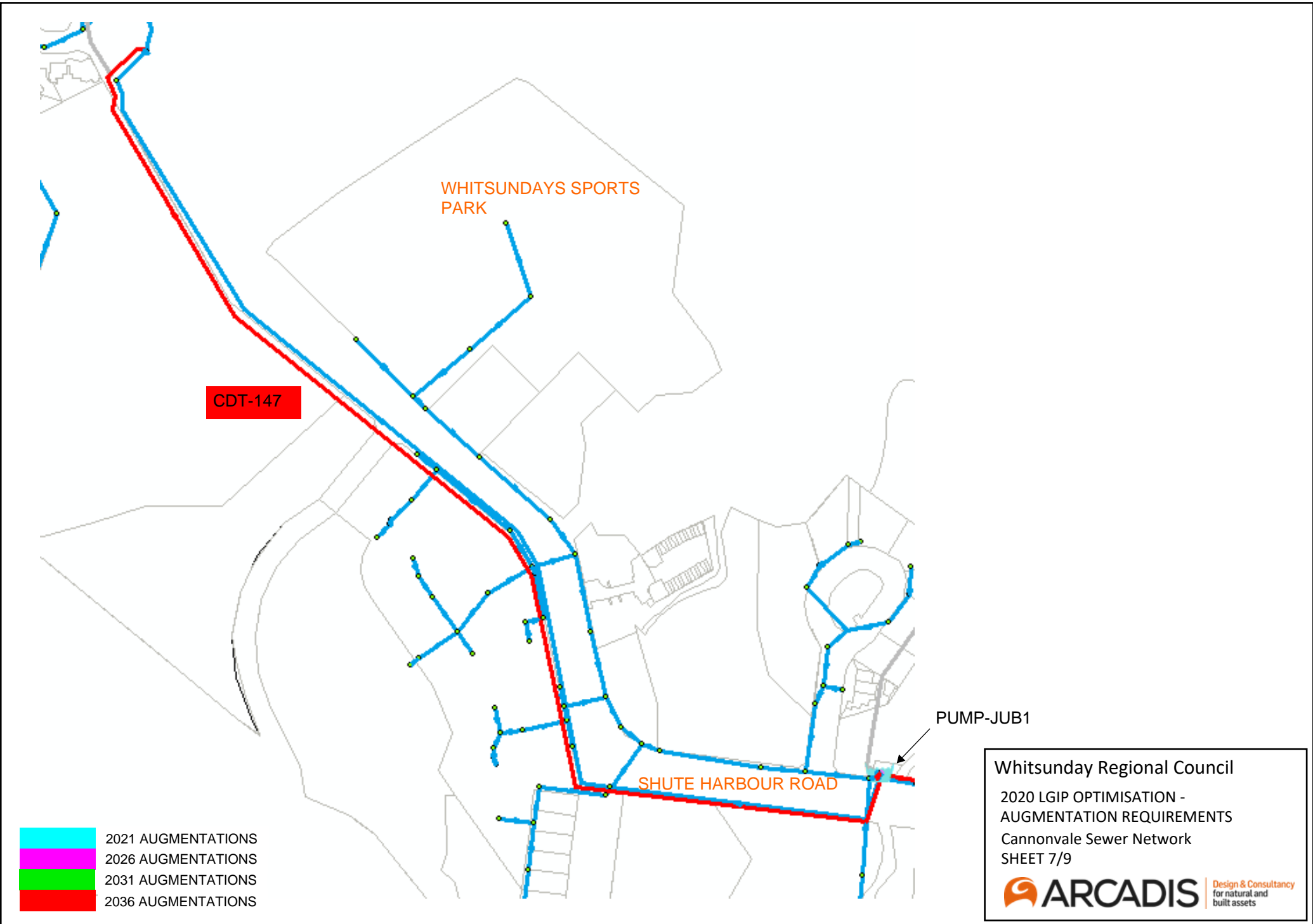


HDPE PIPE
REQUIRED, H2S
IMPACTS DUE TO
GRAVITY/RM FALL

CDT-135

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 6/9

ARCADIS | Design & Consultancy
 for natural and
 built assets



CDT-147

WHITSUNDAYS SPORTS PARK

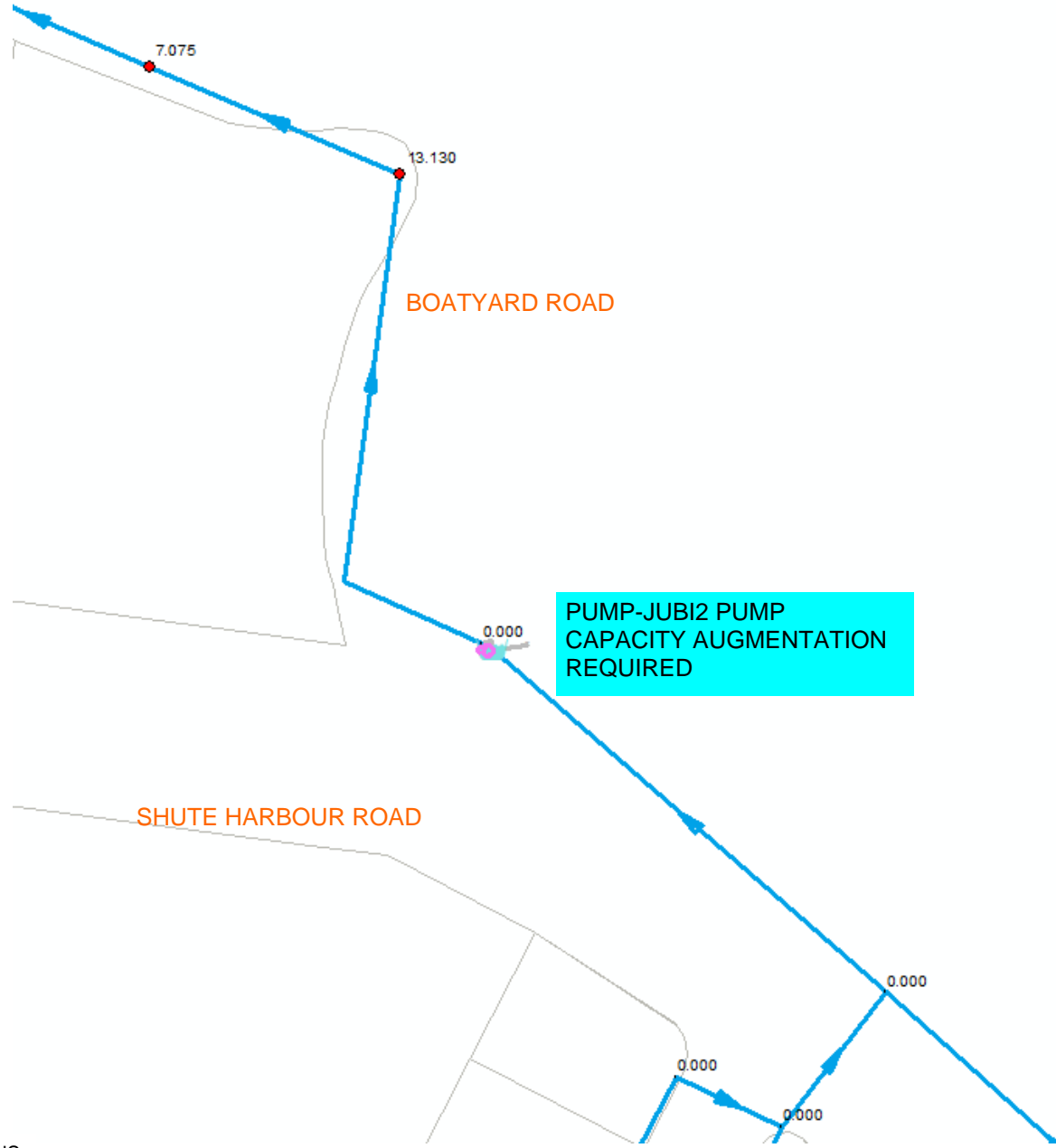
SHUTE HARBOUR ROAD

PUMP-JUB1

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS


Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 7/9

ARCADIS | Design & Consultancy
 for natural and built assets

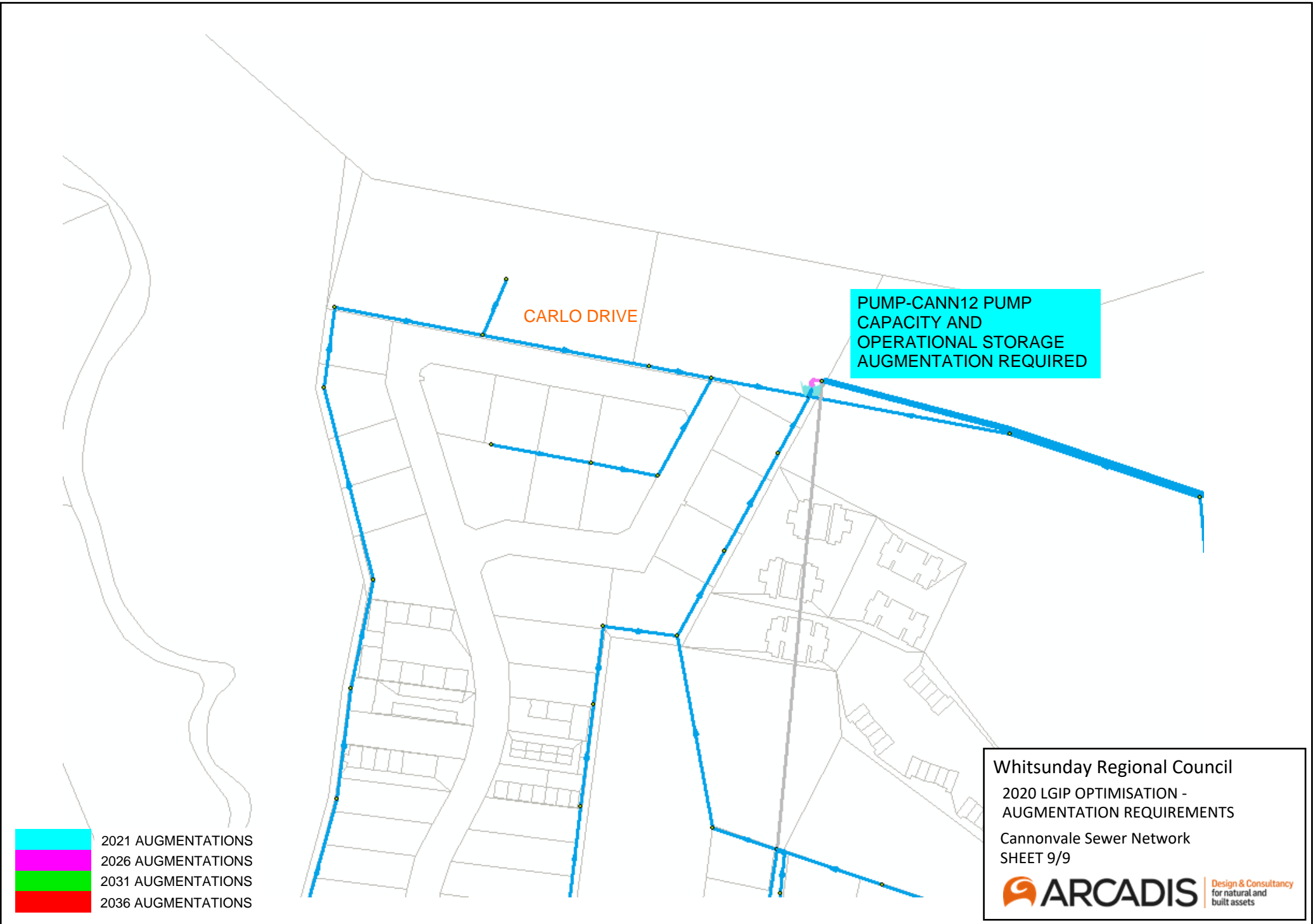


- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 8/9




Design & Consultancy
for natural and
built assets



- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 9/9

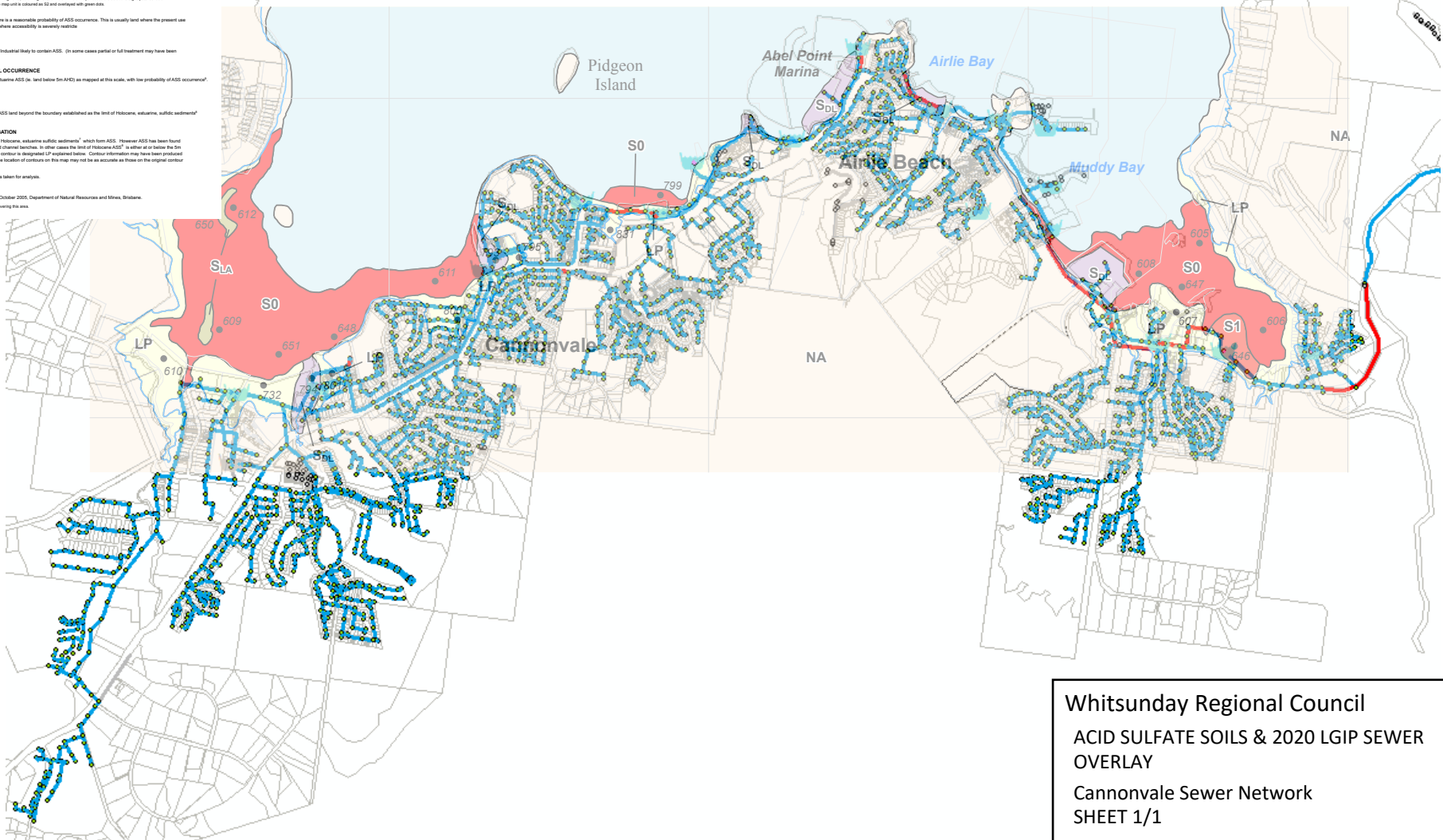
 **ARCADIS** | Design & Consultancy
 for natural and built assets

REFERENCE

ACID SULFATE SOILS (ASS) ON RELATIVELY UNDISTURBED LAND

| Depth Code | Depth to Surface Soil (m) | Depth to Acid Sulfate Soil Layer (m) | Depth to Potential Acid Sulfate Soil (m) |
|------------|---------------------------|--------------------------------------|--|
| 0 | 0 | A0 | A0 |
| 0.5-1m | 1 | A1 | A1 |
| 1-2m | 2 | A2 | A2 |
| 2-3m | 3 | A3 | A3 |
| 3-4m | 4 | A4 | A4 |
| 4-5m | 5 | A5 | A5 |
| >5m | >5 | A6+ | A6+ |

- NOTE:**
- The depth codes above imply that a predominance of profiles in the map unit fall within the nominated depth range.
 - Actual acid sulfate soil layers (designated with an A code) often occur in discrete layers (designated with an S code). Where this occurs, ASS for the map unit is indicated according to the depth of the upper surface of the 'actual' soil (A) and overlaid with either A0, A1 or A2 depending on soil depth code within a strata but layer with field pH ranging from 1.4 to 5.0. This may or may not be a result of sulfate oxidation. Where a depth code is shown on the map, no colour is assigned to it.
 - In areas where there is varying depth to an ASS layer that cannot be separately mapped at the operative scale, two colours are used to designate the dominant depth. This appears as equal width vertical stripes, e.g. S0/S1.
 - P as a prefix indicates sediments of Pleistocene age, W as a prefix indicates sediments of Holocene age.
 - W as a subscript indicates areas associated with Melaleuca sp. wetlands and occasionally Casuarina glauca communities. Occasional sulfur % in surface layers may be highly variable and does not exceed the 'Actual' Code. This may include sulfur from organic materials and residual acidities in soil organic soil environments. ASS typically occurs at depth. Where this occurs e.g. S_W, S_W or A_W, the map is coloured as per the actual or potential depth category and is overlaid with W₁ pattern.
 - W as a subscript indicates areas with sulfidic sulfur values that exceed the 'Actual Code' but contain varying amounts of carbonate materials that may compensate for the potential acidity. Carbonates in carbonate sequences are indicated according to lithology, color, frequency of fracturing. Depth codes are as above, e.g. a potential soil with sulfur soil pH in carbonate occurring at 1 to 2m depth is designated S₁. The map unit is coloured as S₁ and overlaid with green dots.
- SLA** Limited field assessment but occurs in a landscape position where there is a reasonable probability of ASS occurrence. This usually land where the present use precludes any disturbance e.g. National Parks, Reserves etc., or land where accessibility is severely restricted.
- ACID SULFATE ON DISTURBED LAND**
S_D Disturbed land, e.g. Canal estates, Marine, Aquaculture, Quarry, Urban, Industrial likely to contain ASS. (In some cases partial or full treatment may have been undertaken).
- LAND WITH A LOW PROBABILITY OF ACID SULFATE SOIL OCCURRENCE**
LP Land between the 5m AHD contour and the outer limit of Holocene, estuarine ASS (ie. land below 5m AHD) as mapped at this scale, with low probability of ASS occurrence. Limited field investigation.
- LAND NOT ASSESSED**
NA Land not assessed for ASS as part of this survey. It may include non ASS land beyond the boundary established as the limit of Holocene, estuarine, sulfidic sediment but significant or no field testing was carried out.
- 5m AHD CONTOUR - NORMAL LIMIT OF FIELD INVESTIGATION**
 The 5m contour line determines the normal limit of field investigation of Holocene, estuarine sulfidic sediments, which form ASS. However ASS has been found in the study on some lands above the 5m, 10m, 15m, 20m, 30m and 40m contours. In other cases the limit of Holocene ASS is either at or below the 5m contour. In the latter case, the land between the ASS limit and the 5m contour is designated LP as explained above. Contour information may have been produced at a scale different to that applied to this map. As a consequence, the location of contours on this map may not be as accurate as those on the original contour map.
- ?** Specific locations where profiles were described in detail and samples taken for analysis.
- Digital Catalogue Database
 Base map compiled from the Queensland Digital Catalogue Database October 2005. Department of Natural Resources and Mines, Brisbane.
- NOTE: This map should be used in conjunction with the accompanying report covering the area.



Whitsunday Regional Council
 ACID SULFATE SOILS & 2020 LGIP SEWER
 OVERLAY
 Cannonvale Sewer Network
 SHEET 1/1

Design & Consultancy
 for natural and
 built assets

ACID SULFATE SOILS OVERLAY AND LEGEND SOURCED FROM QUEENSLAND GOVERNMENT

WASTE WATER
TREATMENT PLANT

CDT-99


CDT-101

BRUCE HIGHWAY

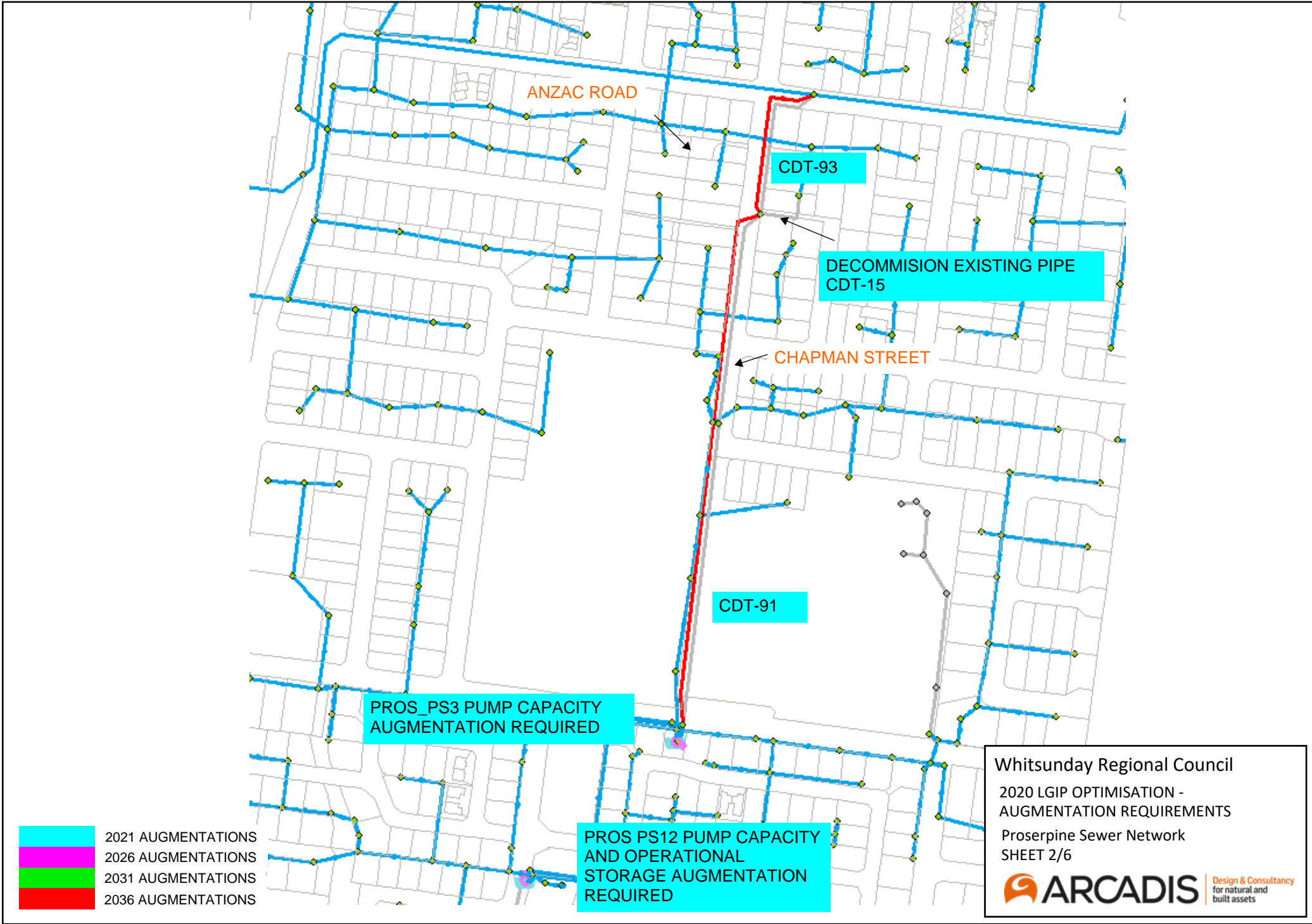
DECOMISSION
EXISTING PIPES →

-  2021 AUGMENTATIONS
-  2026 AUGMENTATIONS
-  2031 AUGMENTATIONS
-  2036 AUGMENTATIONS

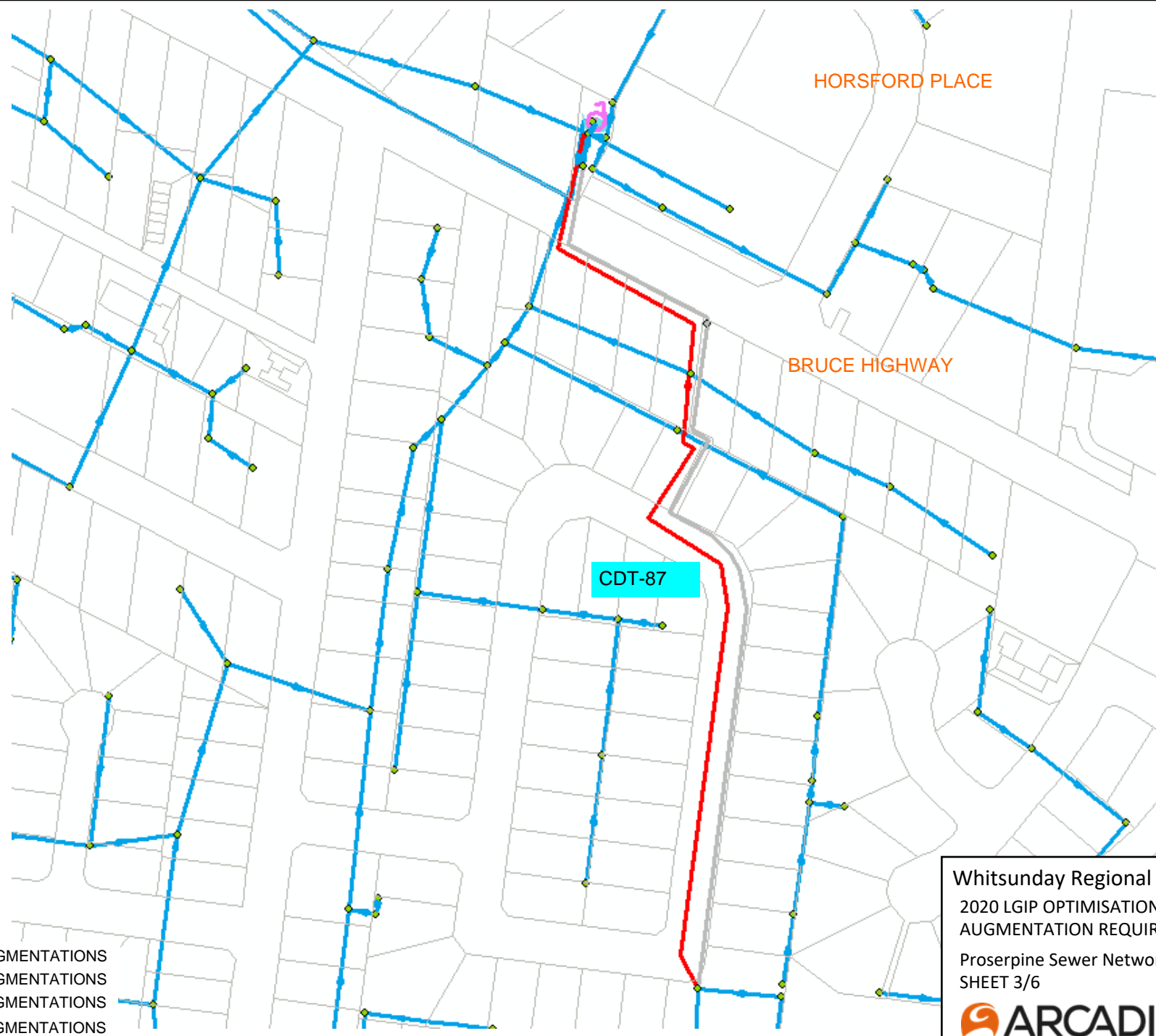
Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS
Proserpine Sewer Network
SHEET 1/6



Design & Consultancy
for natural and
built assets




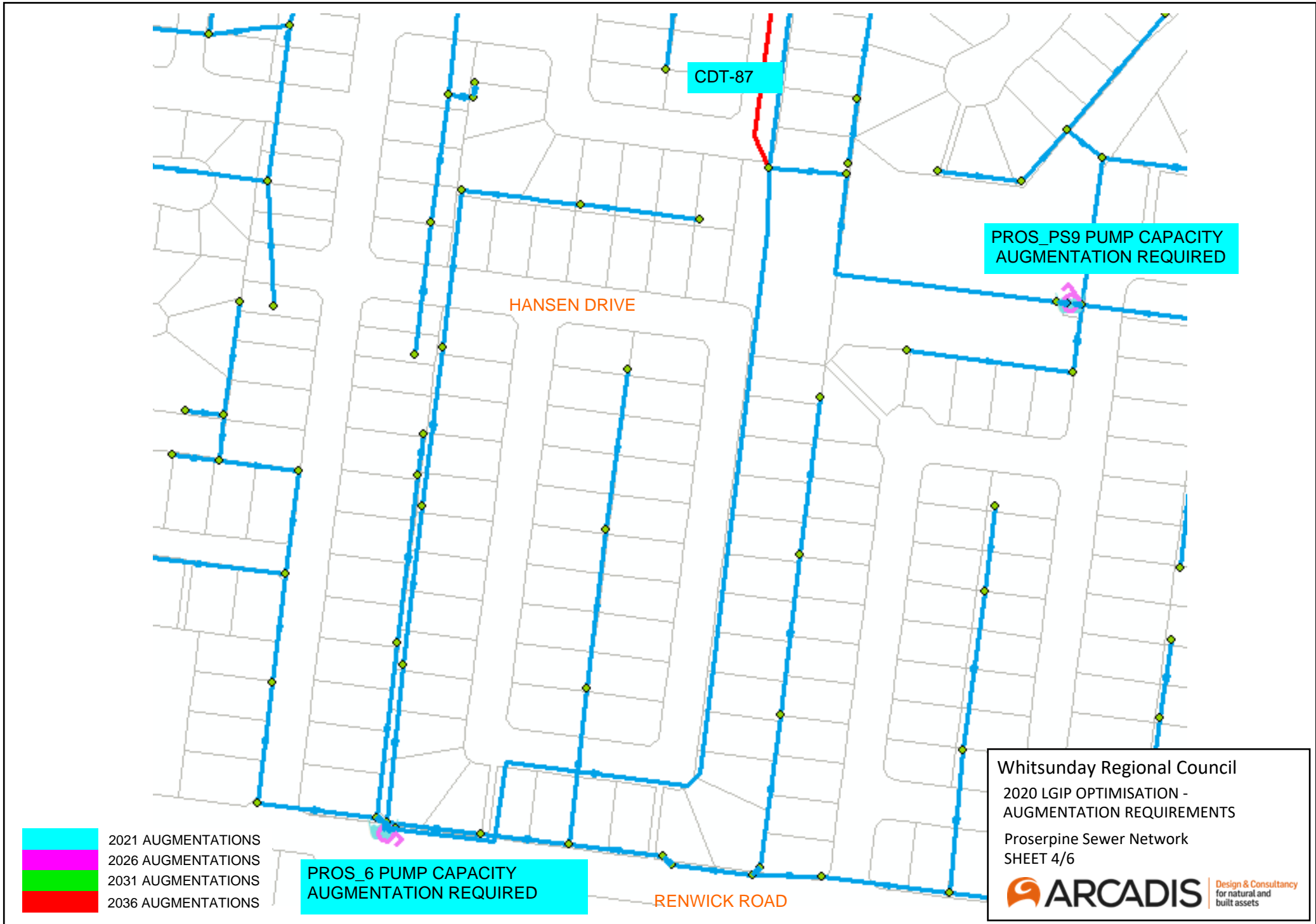
- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS



- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 3/6

 **ARCADIS** | Design & Consultancy
 for natural and built assets

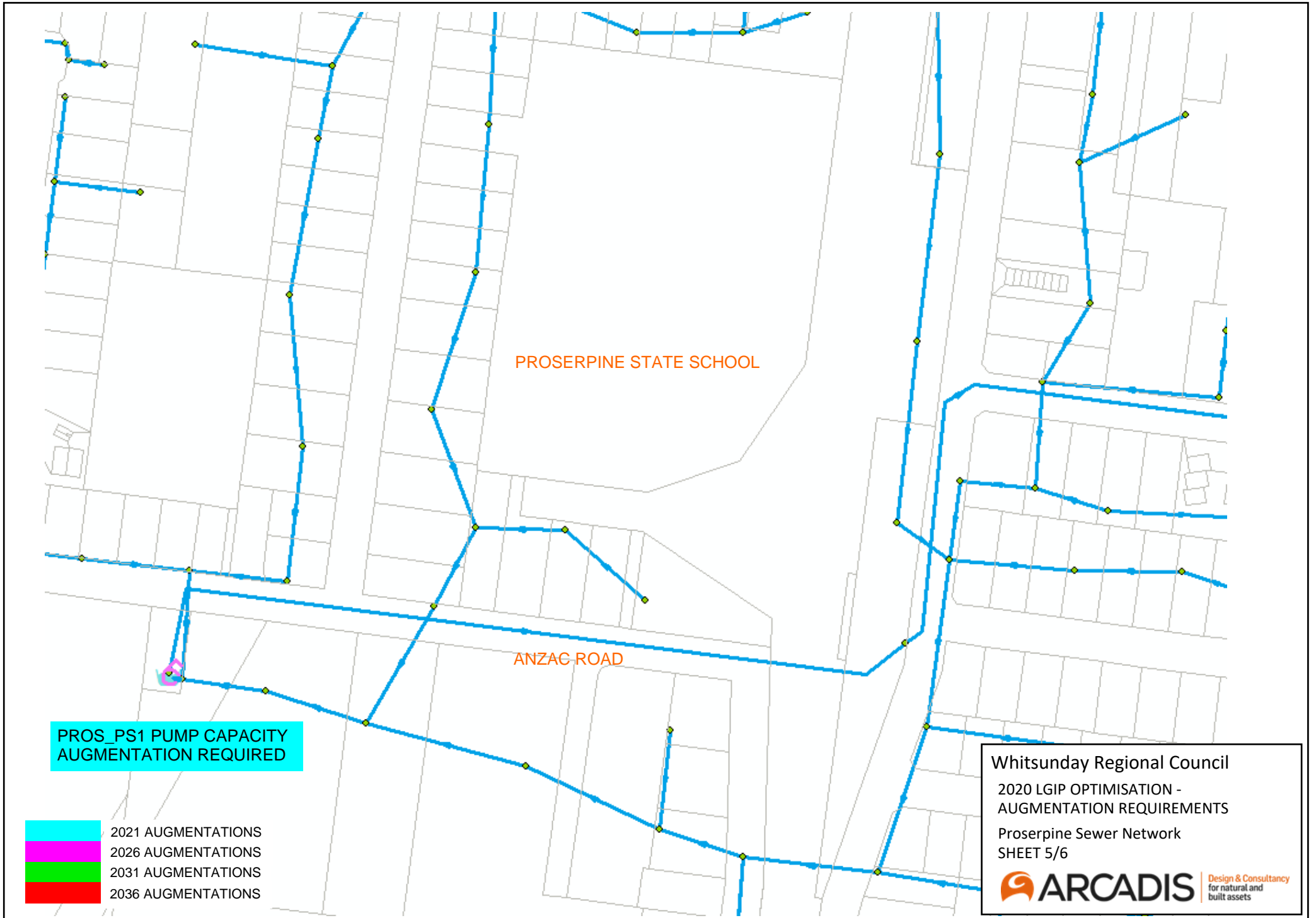


- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

**PROS_6 PUMP CAPACITY
AUGMENTATION REQUIRED**

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 4/6

ARCADIS Design & Consultancy
for natural and
built assets

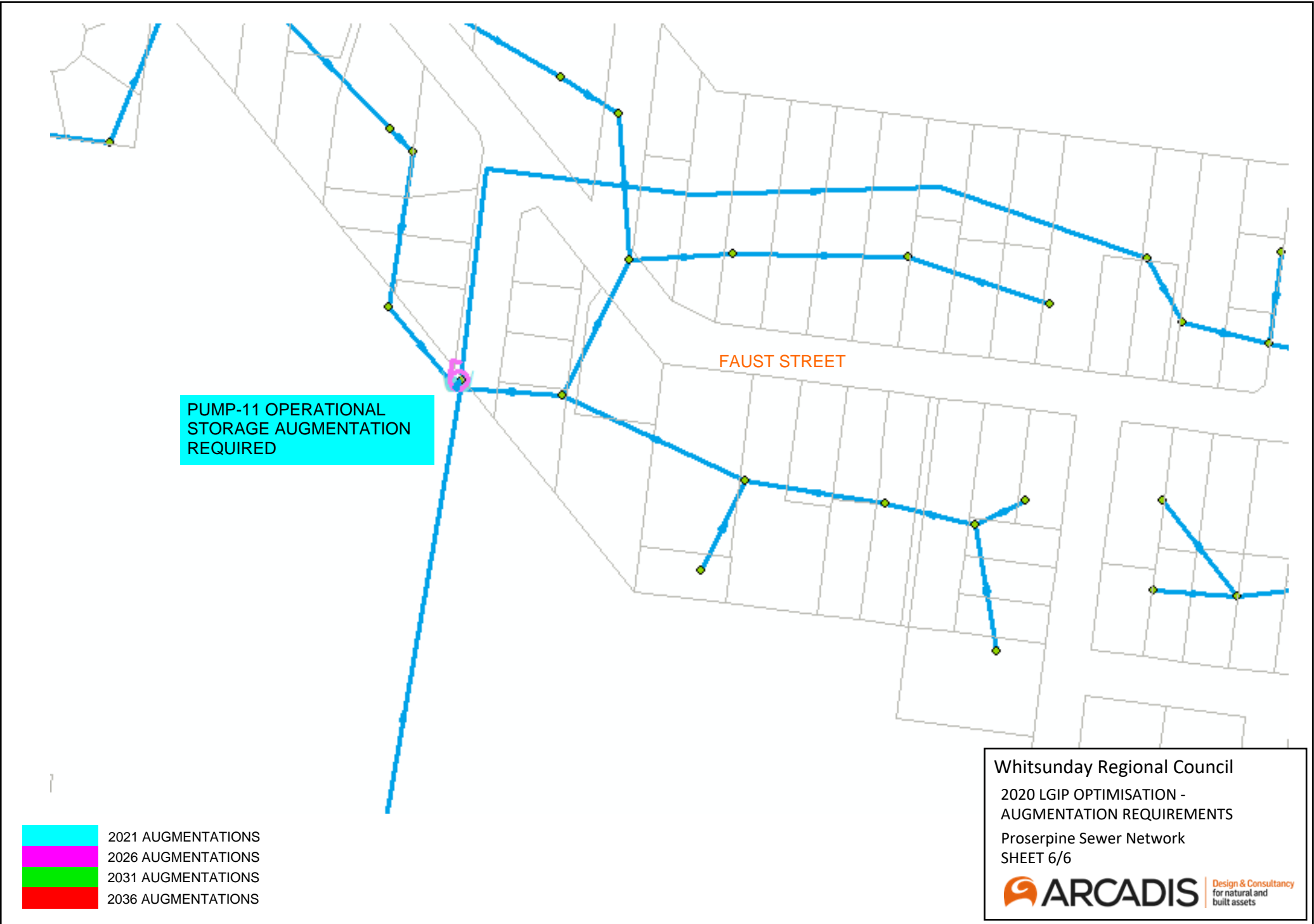


PROS_PS1 PUMP CAPACITY
AUGMENTATION REQUIRED

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS
Proserpine Sewer Network
SHEET 5/6





PUMP-11 OPERATIONAL
STORAGE AUGMENTATION
REQUIRED

FAUST STREET

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council

2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS

Proserpine Sewer Network
SHEET 6/6

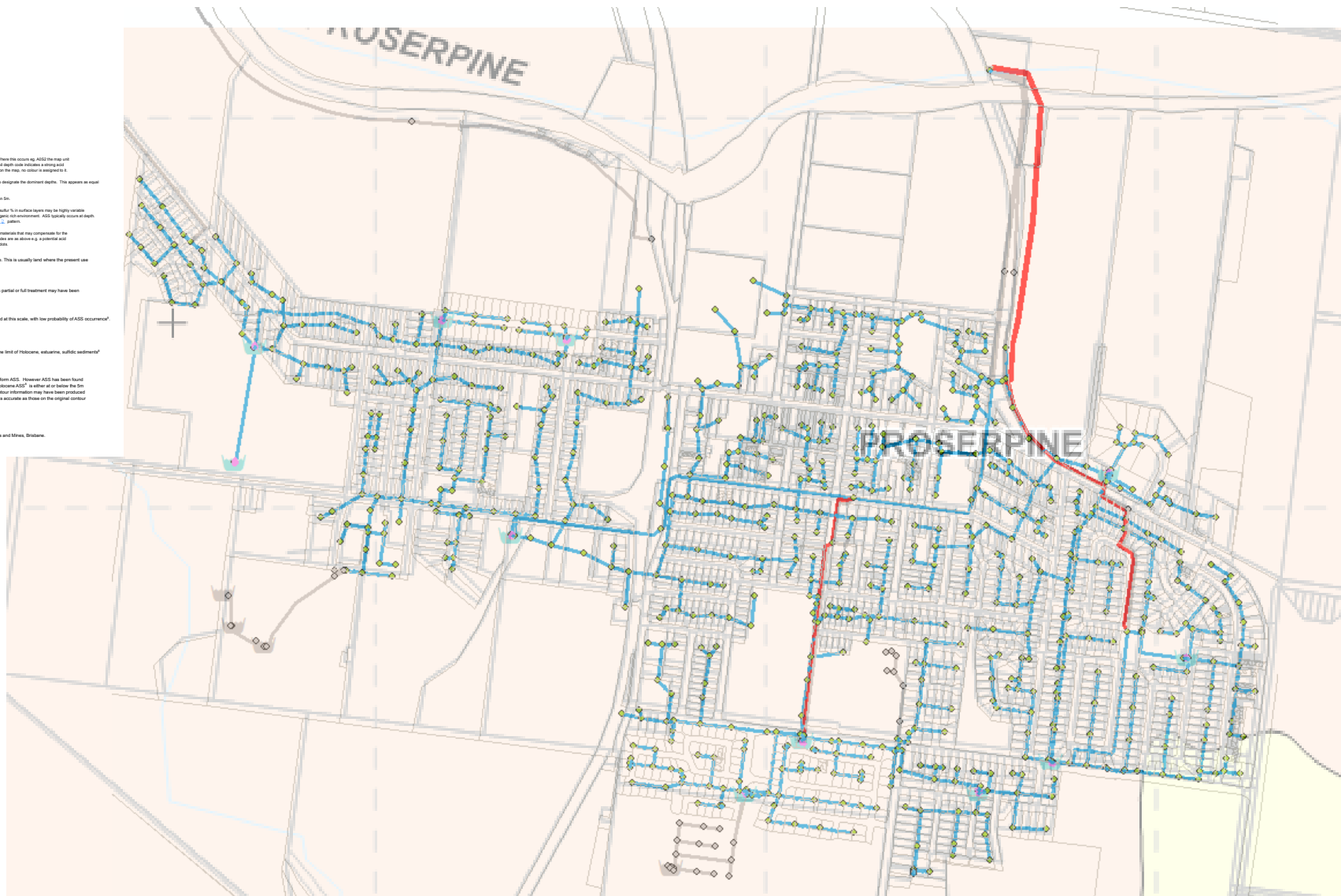


REFERENCE

ACID SULFATE SOILS (ASS) ON RELATIVELY UNDISTURBED LAND

| Depth | Depth Code | Depth to Actual Acid Sulfate Soil (m) | Depth to Potentially Acid Sulfate Soil (m) | Depth to Potential Acid Sulfate Soil (m) |
|--------|------------|---------------------------------------|--|--|
| 0-0.5m | 0 | A0 | A0 | S0 |
| 0.5-1m | 1 | A1 | A1 | S1 |
| 1-2m | 2 | A2 | A2 | S2 |
| 2-3m | 3 | A3 | A3 | S3 |
| 3-4m | 4 | A4 | A4 | S4 |
| 4-5m | 5 | A5 | A5 | S5 |
| >5m | 5+ | A5+ | A5+ | S5+ |

- NOTE:** The depth codes above imply that a predominance of profiles in the map unit fall within the nominated depth range.
- Actual acid sulfate soils (designated with a color) often occur in potential acid sulfate soil layers (designated with an S code). Where this occurs, the ASS layer is overlain according to the depth of the actual soil (A) and overlaid with either A0 to A5, depending on the depth code within a stratum, or A5+ where the depth code is greater than 5.0. This may or may not be a result of sulfate oxidation. Where 'W' depth code is shown on the map, no color is assigned to it.
 - In areas where there is varying depth to an ASS layer that cannot be separately mapped at the operative scale, two colors are used to designate the dominant depths. This appears as equal width vertical columns, e.g. S2/S3.
 - P as a subscript indicates sediments of Pleistocene age¹, while P* indicates sulfidic sediments (of Pleistocene age) deeper than 5m.
 - W as a subscript indicates areas associated with Midallan tip, sulfidic and occasionally Casuarina glauca communities. Oxidizable sulfur % in surface layers may be highly variable and often exceeds the 'Actual Contour'. This may include sulfur from organic materials and residual oxidation of sulfides in soil origins (soil development). ASS typically occurs at depth. Where this occurs e.g. S₁₀ or A₁₀, the map is colored as per the actual or potential depth category and is overlaid with W₁₀ pattern.
 - W as a subscript indicates areas with oxidizable sulfur values that exceed the actual contour² but contain varying amounts of carbonaceous materials that may compensate for the potential acidity. Contours of carbonaceous materials are indicated by orange lines, with figures of percentage. Depth codes are as above, e.g. a potential acid sulfate soil with carbonaceous occurring at 1 to 2m depth is designated S2₁₀. The map unit is colored as S2 and overlaid with green data.
- LA** Limited field assessment but occurs in a landscape position where there is a reasonable probability of ASS occurrence. This is usually land where the present use precludes any disturbance e.g. National Parks, Reserves etc., or land where accessibility is severely restricted.
- ACID SULFATE ON DISTURBED LAND³**
DU Disturbed land, e.g. Canal estates, Marlin, Aquaculture, Quarry, Urban, Industrial likely to contain ASS. (In some cases partial or full treatment may have been undertaken).
- LAND WITH A LOW PROBABILITY OF ACID SULFATE SOIL OCCURRENCE**
LP Land between the 5m AHD contour and the outer limit of Holocene, estuarine ASS (ie. land below 5m AHD) as mapped at this scale, with low probability of ASS occurrence⁴. Limited field investigation.
- LAND NOT ASSESSED**
NA Land not assessed for ASS as part of this survey. It may include non ASS land beyond the boundary established as the limit of Holocene, estuarine, sulfidic sediments⁵ but insufficient or no field testing was carried out.
- 5m AHD⁶ CONTOUR - NORMAL LIMIT OF FIELD INVESTIGATION**
 The 5m contour line determines the normal limit of field investigation of Holocene, estuarine sulfidic sediments⁷, which form ASS. However ASS has been found in the study on areas below the 5m AHD valley floor, near drains and channel benches. In other cases the limit of Holocene ASS is either at or below the 5m contour. In the latter case, the land between the ASS limit and the 5m contour is designated LP as explained below. Contour information may have been produced at a scale different to that applied to this map. As a consequence, the location of contours on this map may not be as accurate as those on the original contour map.
- ?** Specific locations where profiles were described in detail and samples taken for analysis.
- Digital Cadastral Database**
 Base map compiled from the Queensland Digital Cadastral Database October 2005. Department of Natural Resources and Mines, Brisbane.
- NOTE:** This map should be used in conjunction with the accompanying report covering the area.



Whitsunday Regional Council
 ACID SULFATE SOILS & 2020 LGIP SEWER
 OVERLAY
 Proserpine Sewer Network
 SHEET 1/1






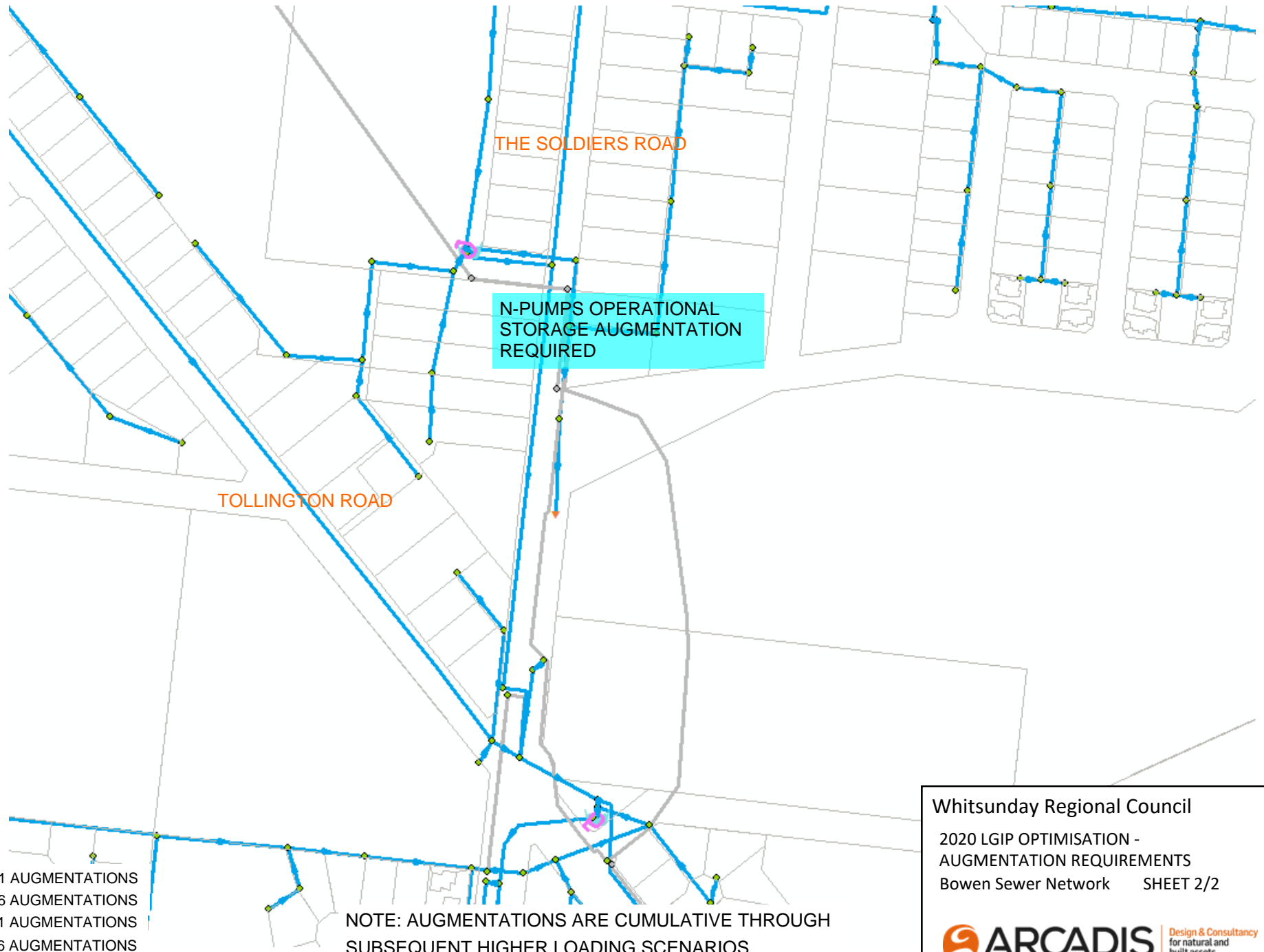
- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENTS
 Bowen Sewer Network
 SHEET 1/2



Design & Consultancy for natural and built assets



THE SOLDIERS ROAD

TOLLINGTON ROAD

N-PUMPS OPERATIONAL
STORAGE AUGMENTATION
REQUIRED

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Bowen Sewer Network SHEET 2/2



Design & Consultancy
for natural and
built assets

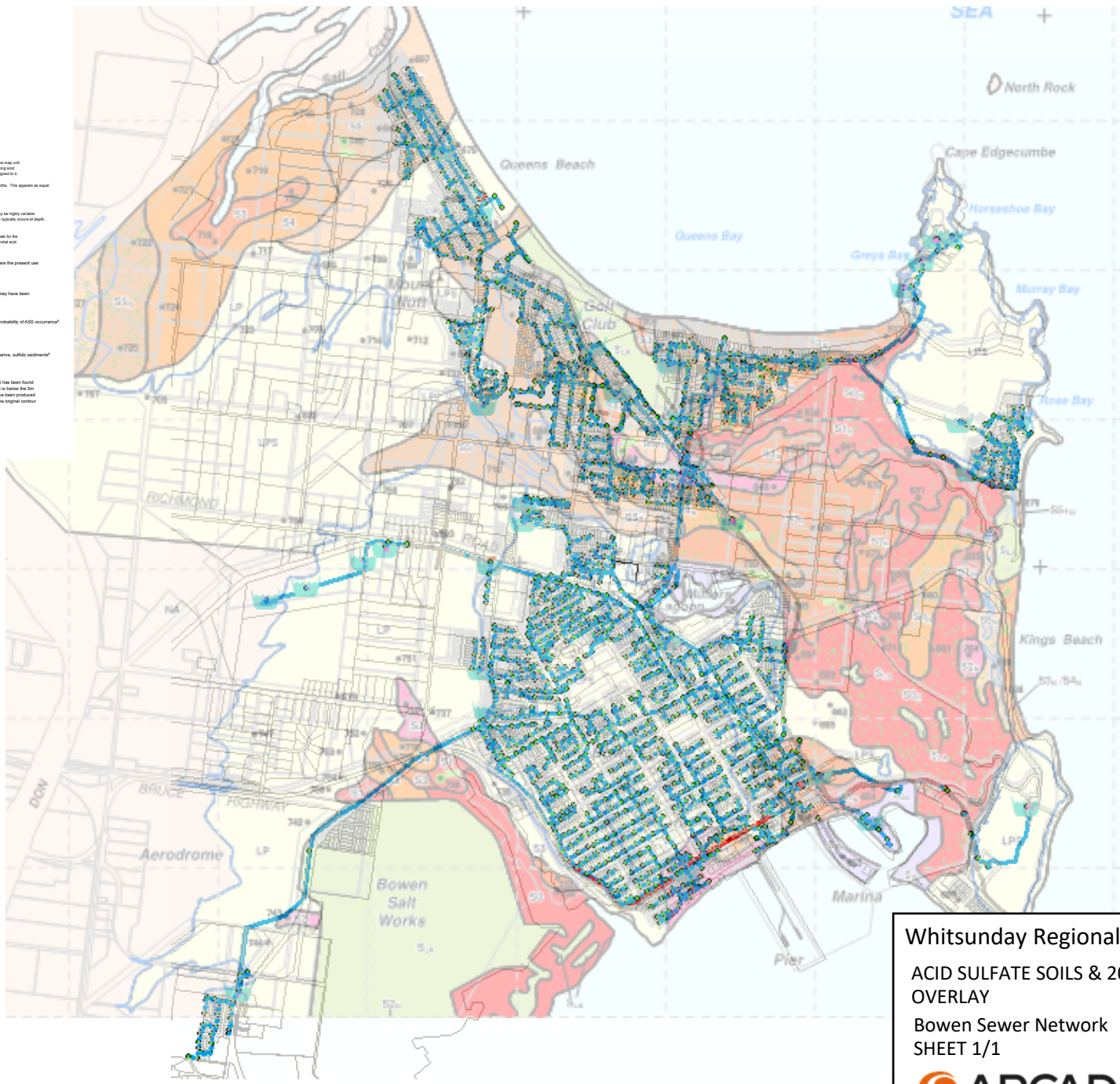
REFERENCE

ACID SULFATE SOILS (ASS) ON RELATIVELY UNDISTURBED LAND

| Depth | Depth Code | Depth to Actual Acid Sulfate Soil (pH < 5.5) | Depth to Potential Acid Sulfate Soil (pH < 5.5) |
|----------|------------|--|---|
| 0 - 0.5m | 0 | A0 | A0 |
| 0.5 - 1m | 1 | A1 | A1 |
| 1 - 2m | 2 | A2 | A2 |
| 2 - 3m | 3 | A3 | A3 |
| 3 - 4m | 4 | A4 | A4 |
| 4 - 5m | 5 | A5 | A5 |
| > 5m | 5+ | A5+ | A5+ |

NOTE: The depth codes above imply that a predominance of profiles in the map unit fall within the nominated depth range.

- Actual acid sulfate soil layers (designated with an A code) often contain potential acid sulfate soil layers (designated with an S code). Where this occurs an ASS map unit is outlined according to the depth of the outer code of the 'actual' soil (A) and overlaid with yellow. An 'S' preceding the soil depth code indicates a strong acid soil layer with field pH ranging from 1.4 to 5.5. This may or may not be a result of sulfate oxidation. Where 'W' depth code is shown on the map, no colour is assigned to it.
 - In areas where there is varying depth to an ASS layer that cannot be separately mapped at the operative scale, two colours are used to designate the dominant depths. This appears as equal width vertical columns, e.g. S2/S3.
 - P as a subscript indicates sediments of Pleistocene age¹, and 'P' indicates surficial sediments of Pleistocene age) deeper than 5m.
 - W as a subscript indicates areas associated with Melaleuca sp. wetlands and occasionally Casuarina glauca communities. Oxidisable sulfur % in surface layers may be highly variable and often exceeds the 'Actual Code'. This may indicate sulfur from organic materials and reduced oxidation of sulfides in soil organic soil environments. ASS spatially occurs at depth. Where this occurs e.g. S₁₀ or S₂₀ or A₁₀, the map is coloured as per the actual or potential depth category and is overlaid with 'W' pattern.
 - If a subscript indicates areas with oxidisable sulfur values that exceed the 'Actual Code', but contain varying amounts of carbonate materials that may compensate for the potential acidity. Carbonate concentrations are indicated according to the legend, using figures of 0.000001 to 0.000005. Depth codes are also shown e.g. a potential acid sulfate soil unit in carbon occurring at 1 to 2m depth is designated S₁. The map unit is coloured as S2 and overlaid with green dots.
 - Limited field assessment but occurs in a landscape position where there is a reasonable probability of ASS occurrence. This is usually land where the present use preclude any disturbance eg. National Parks, Reserves etc., or land where accessibility is severely restricted.
- ACID SULFATE ON DISTURBED LAND²**
- Disturbed land, eg. Canal estates, Marine, Aquaculture, Quarry, Urban, Industrial likely to contain ASS. (In some cases partial or full treatment may have been undertaken).
- LAND WITH A LOW PROBABILITY OF ACID SULFATE SOIL OCCURRENCE**
- Land between the 5m AHD contour and the outer limit of historic, extensive ASS (ie. land below 5m AHD) as mapped at this scale, with low probability of ASS occurrence³. Limited field investigation.
- LAND NOT ASSESSED**
- Land not assessed for ASS as part of this survey. It may include non ASS land beyond the boundary established as the limit of historic, extensive, sulfidic sediments⁴ but significant or no field testing was carried out⁵.
- 5m AHD⁶ CONTOUR - NORMAL LIMIT OF FIELD INVESTIGATION**
- The 5m contour line determines the normal limit of field investigation of historic, extensive sulfidic sediments⁴ which form ASS. However ASS has been found in the depth or some lands above 5m. Top, valley floor, road, canal and channel benches. In other cases the limit of historic ASS⁴ is either at or below the 5m contour. In the latter case, the land between the ASS limit and the 5m contour is designated LP⁷ explained below. Contour information may have been produced at a scale different to that applied to this map. As a consequence, the boundary of contour on this map may not be as accurate as those on the original contour map.
- 7⁸**
- Specific locations where profiles were described in detail and samples taken for analysis.
- Digital Catalogue Database
- Base map compiled from the Queensland Digital Catalogue Database October 2005. Department of Natural Resources and Mines, Brisbane.
- NOTE: This map should be used in conjunction with the accompanying report covering the area.

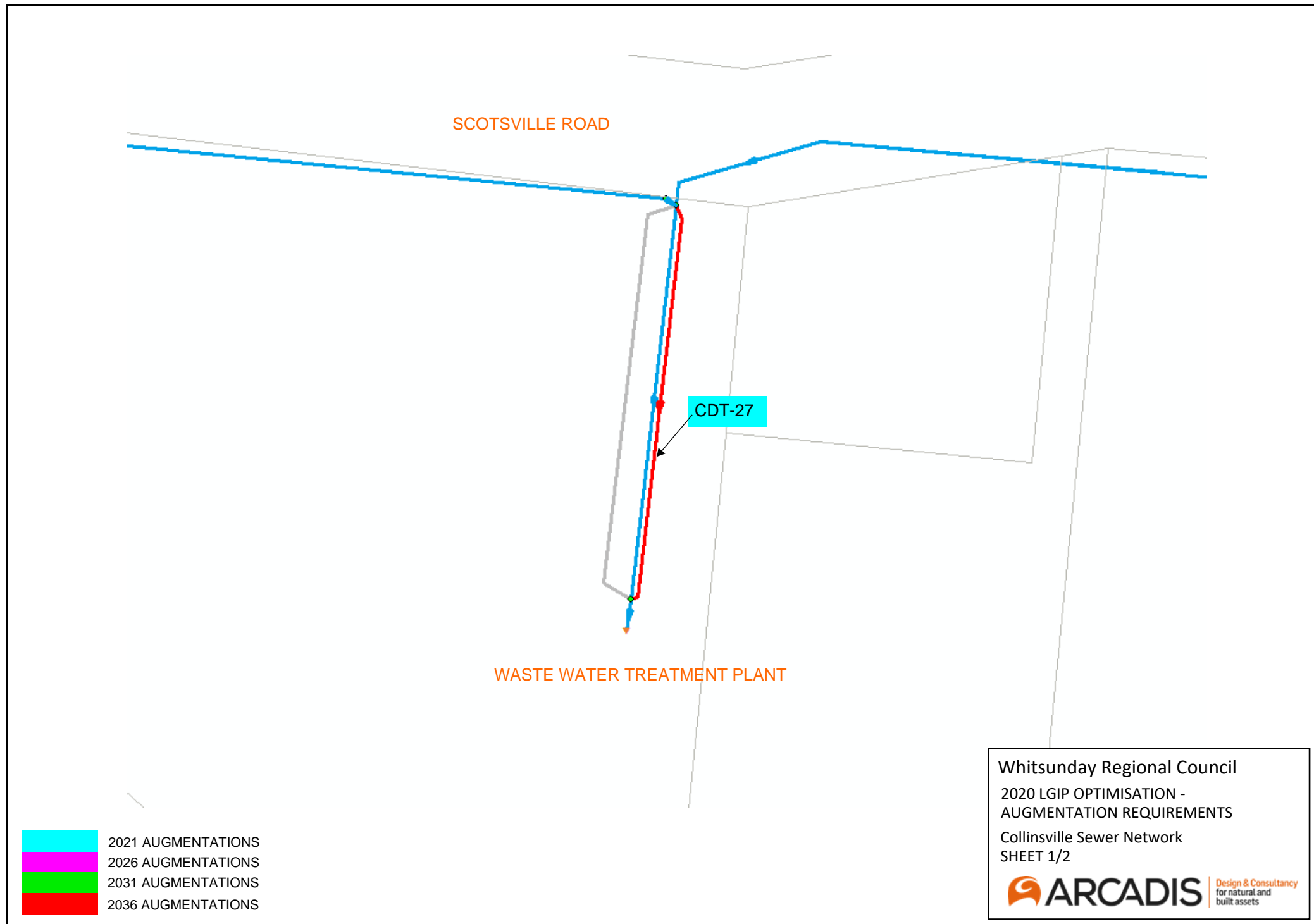


Whitsunday Regional Council
 ACID SULFATE SOILS & 2020 LGIP SEWER
 OVERLAY
 Bowen Sewer Network
 SHEET 1/1



Design & Consultancy
for natural and
built assets

ACID SULFATE SOILS OVERLAY AND LEGEND SOURCED FROM QUEENSLAND GOVERNMENT




SCOTSVILLE ROAD

CDT-27

WASTE WATER TREATMENT PLANT

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 20231 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 2020 LGIP OPTIMISATION -
 AUGMENTATION REQUIREMENTS
 Collinsville Sewer Network
 SHEET 1/2



Design & Consultancy
for natural and
built assets



2-C_PUMPS PUMP
CAPACITY AUGMENTATION
REQUIRED

STATION ROAD

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
2020 LGIP OPTIMISATION -
AUGMENTATION REQUIREMENTS
Collinsville Sewer Network
SHEET 2/2



PROJECT: Whitsundays Sewer Network
 DOCUMENT NUMBER: 0002-10027336-AAC-03
 DATE: 06.02.2020

Project Engineer: M.C/S/H
 Software: InfoSWMM 5A



WHITSUNDAYS COLLINSVILLE SEWER NETWORK
 2020 LGIP OPTIMISATION - AUGMENTATION REQUIREMENT SUMMARY

| AUGMENTATION ID | LENGTH [m] | EXISTING PIPE SEGMENT | | NEW / DUPLICATION DN [mm] | DEPTH RANGE [m] | COST - \$/m | ADJUSTMENT FACTOR FOR SOIL | SCALE FACTOR | 30% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW /DUPLICATION | PIPE REPLACEMENT | | | | COST INCREASE COMPARED TO DUPLICATION | NON-LGIP AUGMENTATIONS | LGIP AUGMENTATIONS | 2021 | 2026 | 2031 | 2036 | NOTES | | | | | |
|-----------------|------------|-----------------------|----------|---------------------------|-----------------|-------------|----------------------------|--------------|-------------------------------|-----------------|--------------------------|------------------|-------------|----------------|--------------|---------------------------------------|------------------------|--------------------|--------------|------|------|------|-------|----------------------|------------|--|--|--|
| | | START NODE | END NODE | | | | | | | | | NOTES | ORIGINAL DN | REPLACEMENT DN | COST - \$/m | | | | | | | | | CAP & GROUT @ \$20/M | TOTAL COST | | | |
| CD1-27 | 174.774 | P1-2 | IC1-12 | 200 | 3 | 240.00 | 1.35 | 1.23 | 10% | 10% | \$ 97,511.31 | REISING MAIN | 225 | 375 | \$ 822.00 | \$ 3,495.48 | \$ 37,295.41 | \$ 239,554.11 | \$ 97,511.31 | | | | | | | | | |
| TOTAL | | | | | | | | | | | | \$ 97,511.31 | | | \$ 37,295.41 | \$ 239,554.11 | \$ 97,511.31 | | | | | | | | | | | |

PUMP AUGMENTATIONS REQUIRED

| PUMP STATION LABEL | AUGMENTATION TYPE | VALUE | COST - \$/A | TOTAL COST | 2021 | 2026 | 2031 | 2036 |
|--------------------|-------------------|-------------|-------------|--------------|------|------|------|------|
| P1-2 | PUMP CAPACITY | 3 x 7.5 kW* | 375000W | \$1125000.00 | | | | |

ADDITIONAL EMERGENCY STORAGE REQUIREMENTS HAVE NOT BEEN COSTED - TO BE VERIFIED AGAINST EXISTING UPSTREAM NETWORK CAPACITY (PIPEWORK + MANHOLES)

*Assumes 70% efficiency at duty

This augment is solely to resolve velocity issues

APPENDIX D

SEWER NETWORK PUMP STATION ASSESSMENT SUMMARY

PROJECT: Whitsundays Sewer Network Modelling
 DOCUMENT N006-10027538-03
 DATE:

Project Engineer: M C/S/H
 Software: InfoSWMM 5A

WHITSUNDAYS CANNONVALE SEWER NETWORK
 2020 LGIP OPTIMISATION - SEWER PUMP STATION ASSESSMENT SUMMARY



| Pump ID (Chgr) | PUMP | Startup Depth (m) | Shutoff Depth (m) | Max Depth | Area | CURRENT | |
|----------------|------|-------------------|-------------------|-----------|-------|-------------|-------------|
| | | | | | | Op. Storage | Em. Storage |
| PUMP-CANN1.2 | 3.9 | 1.1 | 6.6 | 10 | 15.00 | 17.00 | |
| PUMP-CANN1.1 | 0.87 | 0.27 | 4.37 | 4.34 | 2.50 | 13.83 | |
| PUMP-CANN2.1 | 1.7 | 1.2 | 5.95 | 3.6 | 1.80 | 14.22 | |
| PUMP-CANN14.2 | 0.86 | 0.36 | 3.66 | 2.46 | 1.23 | 6.15 | |
| PUMP-CANN15.2 | 0.47 | 0.17 | 5.8 | 1.43 | 0.43 | 7.19 | |
| PUMP-CANN17.1 | 2.6 | 1 | 5 | 10 | 16.00 | 21.00 | |
| PUMP-CANN18.1 | 2.6 | 1 | 5 | 10 | 16.00 | 21.00 | |
| PUMP-CANN2.2 | 2.82 | 1.82 | 5.22 | 2.04 | 2.04 | 4.28 | |
| PUMP-CANN1.1 | 4.26 | 2.76 | 6.86 | 10 | 15.00 | 23.00 | |
| PUMP-CANN4.1 | 1.04 | 1.04 | 5.04 | 4.34 | 8.68 | 7.38 | |
| PUMP-CANN5.2 | 2.8 | 1.9 | 6.3 | 4.34 | 6.51 | 13.89 | |
| PUMP-CANN1.1 | 2.5 | 1.5 | 5 | 2 | 2.00 | 4.00 | |
| PUMP-CANN6.2 | 2.65 | 1.35 | 4.97 | 30 | 30.00 | 60.60 | |
| PUMP-JUB1.1 | 1.85 | 1.85 | 8.1 | 4.34 | 8.68 | 17.14 | |
| PUMP-JUB1.2 | 0.6 | 0.6 | 6.05 | 2.95 | 1.95 | 7.94 | |
| PUMP-JUB1.2 | 2.07 | 0.57 | 3.67 | 10 | 15.00 | 13.00 | |
| PUMP-JUB1A.2 | 1.11 | 1.11 | 6.86 | 3.66 | 1.93 | 11.04 | |
| PUMP-SHUT1.2 | 3 | 2 | 6 | 2.54 | 2.54 | 4.32 | |
| PUMP-SHUT1.1 | 2.6 | 1 | 5 | 2.54 | 4.06 | 5.33 | |
| PUMP-SHUT1.1 | 2.6 | 1 | 3 | 10 | 16.00 | 3.00 | |

| PUMP FACTORS | 0.680 | | | | 0.780 | | | | 0.880 | | | | PWPF | HEAD GAIN |
|--------------|-------------|---------------|------------|-------------|---------------|------|-------------|---------------|-------|-------------|---------------|------|------|-----------|
| | Op. Storage | Emer. Storage | 2021 (L/s) | Op. Storage | Emer. Storage | 2026 | Op. Storage | Emer. Storage | 2031 | Op. Storage | Emer. Storage | 2036 | | |
| 2.82 | 108.16 | 37.35 | 3.36 | 125.36 | 43.53 | 3.46 | 141.49 | 49.13 | 4.24 | 158.88 | 53.20 | 35 | | |
| 1.14 | 43.65 | 15.15 | 1.32 | 50.59 | 17.56 | 1.49 | 57.09 | 19.82 | 1.67 | 64.15 | 22.28 | 22 | | |
| 2.60 | 99.93 | 34.70 | 3.02 | 115.82 | 40.22 | 3.40 | 130.72 | 45.39 | 3.81 | 146.88 | 51.00 | 12 | | |
| 0.12 | 4.79 | 1.66 | 0.14 | 5.55 | 1.91 | 0.16 | 6.37 | 2.18 | 0.18 | 7.64 | 2.45 | 20 | | |
| 0.02 | 0.62 | 0.21 | 0.02 | 0.72 | 0.25 | 0.02 | 0.81 | 0.28 | 0.02 | 0.91 | 0.32 | 0 | | |
| 0.17 | 6.57 | 2.28 | 0.20 | 7.62 | 2.60 | 0.22 | 8.60 | 2.99 | 0.25 | 9.66 | 3.36 | 53 | | |
| 0.37 | 14.12 | 4.90 | 0.43 | 16.36 | 5.68 | 0.48 | 18.47 | 6.41 | 0.54 | 20.75 | 7.21 | 3.9 | | |
| 2.08 | 79.85 | 27.73 | 2.41 | 92.54 | 32.93 | 2.72 | 104.65 | 36.71 | 3.06 | 117.86 | 40.75 | 30 | | |
| 0.14 | 5.24 | 1.82 | 0.16 | 6.07 | 2.11 | 0.18 | 6.86 | 2.38 | 0.20 | 7.70 | 2.68 | 35 | | |
| 2.55 | 97.97 | 34.02 | 2.96 | 113.55 | 39.43 | 3.34 | 128.16 | 44.50 | 3.75 | 144.00 | 50.00 | 51 | | |
| 0.03 | 1.02 | 0.35 | 0.03 | 1.18 | 0.41 | 0.03 | 1.33 | 0.46 | 0.04 | 1.50 | 0.52 | 19 | | |
| 0.80 | 30.62 | 10.63 | 0.92 | 35.48 | 12.32 | 1.04 | 40.05 | 13.91 | 1.17 | 45.00 | 15.63 | 0 | | |
| 4.58 | 174.21 | 60.49 | 5.26 | 201.92 | 70.11 | 5.93 | 227.89 | 79.13 | 6.67 | 256.06 | 86.51 | 35 | | |
| 1.80 | 69.17 | 24.02 | 2.09 | 80.17 | 27.85 | 2.36 | 90.86 | 31.42 | 2.65 | 101.66 | 35.30 | 6 | | |
| 0.38 | 14.53 | 5.04 | 0.44 | 16.84 | 5.85 | 0.49 | 19.03 | 6.69 | 0.56 | 21.36 | 7.42 | 0 | | |
| 0.47 | 17.89 | 6.23 | 0.54 | 20.73 | 7.20 | 0.61 | 23.40 | 8.13 | 0.69 | 26.20 | 9.12 | 6.7 | | |
| 0.16 | 13.47 | 4.85 | 0.42 | 16.19 | 5.62 | 0.48 | 18.28 | 6.35 | 0.53 | 20.53 | 7.13 | 2.5 | | |
| 0.17 | 6.84 | 2.31 | 0.20 | 7.70 | 2.67 | 0.23 | 8.69 | 3.02 | 0.25 | 9.76 | 3.39 | 28 | | |
| 0.10 | 3.85 | 1.34 | 0.12 | 4.46 | 1.55 | 0.13 | 5.04 | 1.75 | 0.15 | 5.66 | 1.97 | 108 | | |

| STORAGE UPGRADES REQUIRED | 2036 | NOTES |
|--|------------------------------|---|
| | Op. Storage Upgrade Required | |
| Emergency storage beyond volume within wet well required | | New pumps already ordered |
| Emergency storage beyond volume within wet well required | | In line to be upgraded already |
| Emergency storage beyond volume within wet well required | 2.03 | Augmented |
| Emergency storage beyond volume within wet well required | | Need to alert to RM, pumps to be installed 2020 |
| OK | | |
| OK | | depth in the wet well is not great enough to trigger the pump |
| OK | | |
| Emergency storage beyond volume within wet well required | | Already augmented |
| Emergency storage beyond volume within wet well required | | New pumps already ordered |
| Emergency storage beyond volume within wet well required | | |
| OK | | Augmented |
| OK | | depth in the wet well is not great enough to trigger the pump |
| Emergency storage beyond volume within wet well required | | Augmented pre-2021 |
| Emergency storage beyond volume within wet well required | | Augmented |
| Emergency storage beyond volume within wet well required | | depth in the wet well is not great enough to trigger the pump |
| Emergency storage beyond volume within wet well required | | |
| Emergency storage beyond volume within wet well required | | |
| Emergency storage beyond volume within wet well required | | |
| Emergency storage beyond volume within wet well required | | |

PROJECT: Whitsundays Sewer Network Modelling
 DOCUMENT NUMBER: 10027536-AAC-03
 DATE:

Project Engineer: M.C.V.H
 Software: InfoSWMM 5A

WHITSUNDAYS COLLINSVILLE SEWER NETWORK
 2020 LGIP OPTIMISATION - SEWER PUMP STATION ASSESSMENT SUMMARY



| PUMP ID (Char) | PUMP | | WET WELL | | CURRENT | |
|----------------|-----------------|-------------------|--------------------|--------------------|-------------|-------------|
| | Start Depth (m) | Shutoff Depth (m) | Wet Well Depth (m) | Wet Well Area (m²) | Op. Storage | Em. Storage |
| PROS_P1 | 0.7 | 0.3 | 5.2 | 1.58 | 1.58 | 15,715 |
| PROS_P51.1 | 2.6 | 1.1 | 8.1 | 7.31 | 10.97 | 38,012 |
| PROS_P510.1 | 0.78 | 0.38 | 3 | 2 | 0.80 | 3.84 |
| PROS_P511.1 | 0.61 | 0.21 | 3 | 2 | 0.80 | 4.18 |
| PROS_P512.1 | 0.8 | 0.4 | 6.1 | 2 | 0.80 | 10.4 |
| PROS_P52.1 | 1.1 | 0.5 | 7.85 | 7.31 | 5.85 | 43,495 |
| PROS_P53.1 | 0.8 | 0.4 | 7.65 | 7.11 | 2.62 | 47,880 |
| PROS_P54.1 | 1.8 | 0.8 | 6 | 1.33 | 1.33 | 4,167 |
| PROS_P55.1 | 1.4 | 0.5 | 5.77 | 1.33 | 1.20 | 5,413 |
| PROS_P59.1 | 0.7 | 0.3 | 7 | 2.32 | 0.93 | 13,92 |
| PUMP_11 | 0.8 | 0.4 | 1 | 2 | 0.80 | 3.8 |

| PWWF FACTORS | 0.030041152 | | | | 0.0547325 | | | | 0.079423868 | | | | PWWF | With final pump curves |
|--------------|-------------|-------------|-----------|-------------|-------------|------|-------------|-------------|-------------|-------------|-------------|--------|------|------------------------|
| | Op. Storage | Em. Storage | 2021 LGIP | Op. Storage | Em. Storage | 2026 | Op. Storage | Em. Storage | 2031 | Op. Storage | Em. Storage | 2036 | | |
| 0.02 | 35.21 | 22.21 | 0.94 | 36.14 | 22.55 | 0.97 | 37.08 | 22.81 | 12.87 | 0.99 | 37.86 | 13,145 | -25 | |
| 3.94 | 151.40 | 92.57 | 4.05 | 155.42 | 53.97 | 4.15 | 159.44 | 65.36 | 4.24 | 162.79 | 56,525 | 42 | | |
| 0.10 | 3.95 | 1.37 | 0.11 | 4.08 | 1.41 | 0.11 | 4.16 | 1.44 | 0.11 | 4.25 | 1,475 | 2 | | |
| 0.10 | 3.94 | 1.37 | 0.11 | 4.04 | 1.40 | 0.11 | 4.15 | 1.44 | 0.11 | 4.23 | 1,427 | 1.5 | | |
| 1.03 | 39.06 | 13.77 | 1.06 | 40.71 | 14.13 | 1.09 | 41.76 | 14.50 | 1.11 | 42.64 | 14,805 | 52 | | |
| 3.72 | 143.87 | 49.64 | 3.81 | 146.46 | 50.85 | 3.91 | 149.25 | 52.17 | 3.99 | 153.40 | 53,865 | 22 | | |
| 1.84 | 70.57 | 24.50 | 1.89 | 72.44 | 25.15 | 1.94 | 74.31 | 25.80 | 1.98 | 75.87 | 26,345 | 42 | | |
| 0.56 | 21.51 | 7.47 | 0.57 | 22.08 | 7.67 | 0.59 | 22.65 | 7.86 | 0.60 | 23.13 | 8.03 | 4.6 | | |
| 0.55 | 21.37 | 7.35 | 0.57 | 21.74 | 7.55 | 0.58 | 22.30 | 7.74 | 0.59 | 22.77 | 7.905 | 10 | | |
| 0.76 | 23.25 | 10.16 | 0.78 | 23.03 | 10.43 | 0.80 | 23.80 | 10.70 | 0.82 | 21.45 | 10.52 | 16 | | |

| UPGRADES REQUIRED | 2036 |
|--|------------------------------|
| Op. Storage Upgrade Required | Op. Storage Upgrade Required |
| Emergency storage beyond volume within wet well required | |
| Emergency storage beyond volume within wet well required | 0.31 |
| Emergency storage beyond volume within wet well required | |
| Emergency storage beyond volume within wet well required | |
| Emergency storage beyond volume within wet well required | 0.02 |

NOTES
 PWWF quantity reduced with pipe augmentations (R8.02 to S6.52)

PROJECT: Whitsundays Sewer Network Modelling
 DOCUMENT NUMBER: D002 10027536.AAC.01
 DATE:

Project Engineer: M.C.S.H
 Software: INFOSWMM 5A

WHITSUNDAYS COLLINSVILLE SEWER NETWORK
 2020 LGIP OPTIMISATION - SEWER PUMP STATION ASSESSMENT SUMMARY



| Pump ID (Chart) | PUMP | Startup Depth (m) | Shutoff Depth (m) | WET WELL | | CURRENT | |
|-----------------|------|-------------------|-------------------|-----------|--------|-------------|--------------|
| | | | | Max Depth | Area | Op. Storage | Emr. Storage |
| I.C. PUMPS 1 | 1.15 | 0.61 | 5.11 | 15.8 | 8.532 | 47.828 | |
| I.C. PUMPS 2 | 1.75 | 0.61 | 5.11 | 15.8 | 18.012 | 48.348 | |
| I.C. PUMPS 1 | 1.21 | 0.71 | 4.61 | 3.66 | 1.81 | 11.4102 | |
| I.C. PUMPS 2 | 2.27 | 0.71 | 4.61 | 3.66 | 5.7096 | 7.5306 | |
| B.C. PUMPS 1 | 0.7 | 0.3 | 6.5 | 2.54 | 1.016 | 13.97 | |
| B.C. PUMPS 2 | 0.9 | 0.3 | 6.5 | 2.54 | 3.524 | 19.862 | |
| A.C. PUMPS 1 | 0.7 | 0.3 | 2.7 | 2.54 | 1.016 | 4.318 | |
| A.C. PUMPS 2 | 0.9 | 0.3 | 2.7 | 2.54 | 3.524 | 8.81 | |

| 0.9609375 | | | | | | 0.9661458 | | | | | | 0.9771958 | | | | | | PWW | | With ideal pump curves | |
|-------------|--------------|-----------|-------------|--------------|-------|-------------|--------------|-------|-------------|--------------|--------|-------------|--------------|------|-------------|--------------|------|-----------|--|------------------------|--|
| Op. Storage | Emr. Storage | 2021 8:00 | Op. Storage | Emr. Storage | 2026 | Op. Storage | Emr. Storage | 2031 | Op. Storage | Emr. Storage | 2036 | Op. Storage | Emr. Storage | 2036 | Op. Storage | Emr. Storage | 2036 | HEAD GAIN | | | |
| 3.33 | 127.03 | 44.42 | 3.35 | 128.62 | 44.66 | 3.46 | 132.78 | 46.10 | 3.47 | 133.13 | 46.225 | 28 | | | | | | | | | |
| 0.68 | 26.22 | 9.10 | 0.69 | 26.36 | 9.15 | 0.71 | 27.22 | 9.45 | 0.71 | 27.29 | 9.475 | 55 | | | | | | | | | |
| 0.68 | 26.22 | 9.10 | 0.69 | 26.36 | 9.15 | 0.71 | 27.22 | 9.45 | 0.71 | 27.29 | 9.475 | | | | | | | | | | |
| 0.02 | 0.84 | 0.29 | 0.02 | 0.85 | 0.29 | 0.02 | 0.88 | 0.30 | 0.02 | 0.88 | 0.305 | 0 | | | | | | | | | |
| 0.02 | 0.84 | 0.29 | 0.02 | 0.85 | 0.29 | 0.02 | 0.88 | 0.30 | 0.02 | 0.88 | 0.305 | | | | | | | | | | |
| 0.07 | 2.71 | 0.94 | 0.07 | 2.73 | 0.95 | 0.07 | 2.82 | 0.98 | 0.07 | 2.82 | 0.98 | 4.8 | | | | | | | | | |
| 0.07 | 2.71 | 0.94 | 0.07 | 2.73 | 0.95 | 0.07 | 2.82 | 0.98 | 0.07 | 2.82 | 0.98 | | | | | | | | | | |

| STORAGE UPGRADES REQUIRED | 2036 | NOTES |
|--|------|----------------------------------|
| Op. Storage Upgrade Required | | |
| Emergency storage beyond volume within wet well required | | |
| Emergency storage beyond volume within wet well required | | Pump augmentation required |
| OK | | Not enough depth to trigger pump |
| OK | | |

APPENDIX E

POTABLE WATER & SEWER NETWORK MCA ASSESSMENTS

PROJECT:
DOCUMENT NUMBER:
DATE:

Whitsundays Water Network Modelling
0008-10027536-01
03.02.2020

Project Engineer: M.C/S.H
Software: Bentley WaterCAD v8i



WHITSUNDAYS POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT

| ID | Change from Initial 2020 LGIP Optimisation | Need to Service | Operational (Known Bursts/Construction Fail) | Redundancy/Management | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP |
|----------------------|---|-----------------|--|-----------------------|---|-------------|------|
| 2014_Aug_J-100_PR172 | | | | | | 0 | |
| 2014_Aug_J-119_PR128 | maintain Augment | 3 | 3 | 3 | 3 | 12 | |
| 2014_Aug_JP60_JP121 | | | | | | 0 | YES |
| 2014_Aug_PR5 PR9 | | | | | | 0 | |
| 2014_Aug_PR106 PR107 | | | | | | 0 | |
| 2014_Aug_PR108 J-126 | | | | | | 0 | |
| 2014_Aug_PR171 PR173 | | | | | | 0 | |
| 2014_Aug_PR236 PR239 | | | | | | 0 | |
| 2014_Aug_PR229 PR143 | | | | | | 0 | |
| 2014_Aug_PR24 PR25 | maintain Augments | 3 | 3 | 3 | 3 | 12 | |
| 2014_Aug_PR242 PR237 | remove augmentation - not needed as will loop line. | | | | | 0 | |
| 2014_Aug_PR244 PR243 | maintain augmentation | | | | | 0 | |
| 2014_Aug_PR25 PR27 | maintain augments | 2 | 3 | 2 | 2 | 9 | |
| 2014_Aug_PR54 J-116 | | | | | | 0 | |
| 2021_Aug_BS107_JP58 | | | | | | 0 | YES |
| 2021_Aug_BS116_JP77 | | | | | | 0 | |
| Air Iso Aug_01 | | | | | | 0 | |
| Air Iso Aug_02 | | | | | | 0 | |
| Air Iso Aug_03 | | | | | | 0 | |
| Air Iso Aug_04 | | | | | | 0 | |
| BS229_AB130 | | | | | | 0 | |
| CanW Iso Aug_06 | remove - new loop from further west. | | | | | 0 | |
| CanW Iso Aug_09 | possible additional tank and alternative alignment to manage head limits. | | | | | 0 | |
| CanW Iso Aug_17 | retain augment | | | | | 0 | |
| CanW Iso Aug_18 | | | | | | 0 | |
| CanW Iso Aug_19 | | | | | | 0 | |
| CanW Iso Aug_21 | | | | | | 0 | |
| CanW Iso Aug_25 | | | | | | 0 | |
| CanW Iso Aug_26 | retain augment | | | | | 0 | YES |
| CanW Iso Aug_27 | retain augment | | | | | 0 | YES |
| CanW378 | | | | | | 0 | |
| CanW76 | | | | | | 0 | |
| CanW76a | | | | | | 0 | |
| CanW76b | | | | | | 0 | |
| JubW Iso Aug_05 | | | | | | 0 | |
| JubW Iso Aug_06 | | | | | | 0 | |
| JubW Iso Aug_08 | | | | | | 0 | |
| JubW Iso Aug_09 | | | | | | 0 | |
| P-1012 | | | | | | 0 | |
| P-1022 | maintain Augment | 2 | 2 | 2 | 2 | 8 | |
| P-1023 | maintain Augment | 3 | 3 | 3 | 3 | 12 | |
| P-1030 | | | | | | 0 | |
| P-1039 | retain augment | 4 | 4 | 4 | 4 | 16 | |
| P-1045 | | | | | | 0 | |
| P-1048 | | | | | | 0 | |
| P-1049 | Maintain Augment | 2 | 2 | 2 | 2 | 8 | |
| P-1050 | Maintain Augment | 2 | 2 | 2 | 2 | 8 | YES |
| P-1059 | | | | | | 0 | YES |
| P-1060 | Maintain Augment | 4 | 4 | 4 | 4 | 16 | YES |
| P-1063 | | | | | | 0 | |
| P-1066 | Maintain Augment | 4 | 4 | 4 | 4 | 16 | YES |
| P-1069 | | | | | | 0 | YES |
| P-1072 | | | | | | 0 | YES |
| P-1076 | | | | | | 0 | |
| P-1088 | | | | | | 0 | YES |
| P-1090 | | | | | | 0 | |
| P-1093 | Maintain Augment | 3 | 3 | 3 | 3 | 12 | |
| P-1094 | Maintain Augments | 2 | 3 | 2 | 2 | 9 | |
| P-1096 | | | | | | 0 | |
| P-1105 | maintain augment | 1 | 1 | 1 | 0 | 3 | |
| P-1110 | Maintain Augment | 3 | 3 | 3 | 3 | 12 | |
| P-1111 | Maintain Augment | 1 | 1 | 2 | 1 | 5 | |
| P-1112 | Maintain Augment | 2 | 2 | 2 | 2 | 8 | |
| P-1115 | Built remove from LGIP list | | | | | 0 | |
| P-1124 | retain | | | | | 0 | |
| P-1125 | | | | | | 0 | |
| P-1126 | required. | | | | | 0 | |
| P-1130 | Required | | | | | 0 | |
| P-1133 | retain augment | 3 | 3 | 3 | 3 | 12 | |
| P-1134 | retain augment | | | | | 0 | |
| P-1135 | retain augment | | | | | 0 | |
| P-1137 | | | | | | 0 | |
| P-1141 | | | | | | 0 | |
| P-1149 | remove augment. | | | 0 | | 0 | |
| P-1150 | | | | | | 0 | |
| P-1154 | maintain | | | | | 0 | |
| P-1154 | | | | | | 0 | |
| P-1155 | retain | 1 | 1 | 2 | 1 | 5 | |
| P-1156 | | | | | | 0 | |
| P-1162 | | | | | | 0 | |
| P-1164 | | | | | | 0 | |
| P-1165 | | | | | | 0 | |
| P-1170 | | | | | | 0 | YES |
| P-1173 | | | | | | 0 | |
| P-1174 | | | | | | 0 | YES |
| P-1176 | | | | | | 0 | YES |
| P-1178 | | | | | | 0 | |
| P-1179 | | | | | | 0 | |
| P-1180 | | | | | | 0 | |
| P-1181 | | | | | | 0 | |
| P-1183 | | | | | | 0 | |
| P-1184 | | | | | | 0 | YES |
| P-1188 | | | | | | 0 | |
| P-1191 | | | | | | 0 | |
| P-1196 | | | | | | 0 | |
| P-1200 | | | | | | 0 | |
| P-1201 | | | | | | 0 | |
| P-1204 | | | | | | 0 | YES |
| P-1205 | | | | | | 0 | |
| P-1206 | | | | | | 0 | |
| P-1208 | | | | | | 0 | |
| P-1223 | | | | | | 0 | |
| P-1224 | | | | | | 0 | |
| P-1225 | remove augment | | | | | 0 | |
| P-1230 | | | | | | 0 | YES |
| P-1236 | | | | | | 0 | |
| P-1238 | | | | | | 0 | |
| P-1239 | | | | | | 0 | |
| P-1240 | remains | | | | | 0 | |
| P-1241 | remains | | | | | 0 | |
| P-1242 | remains | | | | | 0 | |
| P-1244 | | | | | | 0 | |
| P-1271 | | | | | | 0 | YES |
| P-1274 | | | | | | 0 | |
| P-1275 | | | | | | 0 | |
| P-1276 | | | | | | 0 | |
| P-1292 | | | | | | 0 | |
| P-1293 | | | | | | 0 | |
| P-1310 | | | | | | 0 | |
| P-1311 | | | | | | 0 | |
| P-1324 | | | | | | 0 | |
| P-1346 | | | | | | 0 | |
| P-1354 | | | | | | 0 | |
| P-1355 | | | | | | 0 | |
| P-1356 | | | | | | 0 | |
| P-1357 | | | | | | 0 | |
| P-1358 | | | | | | 0 | |
| P-1359 | | | | | | 0 | |
| P-1362 | | | | | | 0 | |
| P-1363 | | | | | | 0 | |
| P-1364 | | | | | | 0 | |
| P-1365 | | | | | | 0 | |
| P-1366 | | | | | | 0 | YES |
| P-1367 | | | | | | 0 | YES |
| P-1368 | | | | | | 0 | YES |
| P-1369 | | | | | | 0 | |
| P-1371 | | | | | | 0 | |
| P-1372 | | | | | | 0 | |
| P-1373 | | | | | | 0 | |
| P-1374 | | | | | | 0 | YES |
| P-1375 | | | | | | 0 | |
| P-1378 | | | | | | 0 | YES |
| P-1380 | | | | | | 0 | YES |
| P-1381 | | | | | | 0 | YES |
| P-1382 | | | | | | 0 | |
| P-1391 | | | | | | 0 | |
| P-1394 | | | | | | 0 | |
| P-1397 | | | | | | 0 | |
| P-1403 | | | | | | 0 | |
| P-1404 | | | | | | 0 | YES |
| P-1405 | | | | | | 0 | YES |
| P-1406 | | | | | | 0 | YES |

| | | | | | | | |
|-----------------|--|--|--|--|--|---|-----|
| P-1407 | | | | | | 0 | YES |
| P-1408 | | | | | | 0 | YES |
| P-1409 | | | | | | 0 | YES |
| P-1410 | | | | | | 0 | YES |
| P-1411 | | | | | | 0 | YES |
| P-1412 | | | | | | 0 | YES |
| P-1413 | | | | | | 0 | YES |
| P-1414 | | | | | | 0 | YES |
| P-1415 | | | | | | 0 | YES |
| P-1416 | | | | | | 0 | YES |
| P-1419 | | | | | | 0 | YES |
| P-1420 | | | | | | 0 | YES |
| P-1430 | | | | | | 0 | YES |
| P-1434 | | | | | | 0 | YES |
| P-1435 | | | | | | 0 | YES |
| P-1439 | | | | | | 0 | YES |
| P-1440 | | | | | | 0 | YES |
| P-1441 | | | | | | 0 | YES |
| P-1442 | | | | | | 0 | YES |
| P-1444 | | | | | | 0 | YES |
| P-1445 | | | | | | 0 | YES |
| P-1446 | | | | | | 0 | YES |
| P-1461 | | | | | | 0 | YES |
| P-1462 | | | | | | 0 | YES |
| P-1463 | | | | | | 0 | YES |
| P-1475 | | | | | | 0 | YES |
| P-1482 | | | | | | 0 | YES |
| P-1485 | | | | | | 0 | YES |
| P-1487 | | | | | | 0 | YES |
| P-1488 | | | | | | 0 | YES |
| P-1494 | | | | | | 0 | YES |
| P-1495 | | | | | | 0 | YES |
| P-1496 | | | | | | 0 | YES |
| P-1497 | | | | | | 0 | YES |
| P-1498 | | | | | | 0 | YES |
| P-1501 | | | | | | 0 | YES |
| P-682 | | | | | | 0 | YES |
| P-835 | | | | | | 0 | YES |
| P-836 | | | | | | 0 | YES |
| P-867 | | | | | | 0 | YES |
| P-901 | | | | | | 0 | YES |
| P-902 | | | | | | 0 | YES |
| P-903 | | | | | | 0 | YES |
| P-905 | | | | | | 0 | YES |
| P-945 | | | | | | 0 | YES |
| P-946 | | | | | | 0 | YES |
| P-978 | | | | | | 0 | YES |
| P-982 | | | | | | 0 | YES |
| P-987 | | | | | | 0 | YES |
| PHW 199 Aug 14 | | | | | | 0 | YES |
| SHHW 199 Aug 03 | | | | | | 0 | YES |

PROJECT: Whitsundays Water Network Modelling
DOCUMENT NUMBER: D008-10027536-01
DATE: 11.02.2020

Project Engineer: M.C/S.H
Software: Bentley WaterCAD v8i



**BOWEN POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT**

| ID | Change from initial 2019 LGIP | Need to Service | Operational (Known Bursts/Construction Fails) | Redundancy/Management | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP |
|------------------------|---|-----------------|---|-----------------------|---|-------------|------|
| 2014_Aug_BNA160_BNA303 | No change, needed | 2 | 1 | 0 | 0 | 3 | |
| 2014_Aug_BNA581_BNA582 | Change to DN200, abandon AC pipe | 4 | 2 | 0 | 0 | 6 | YES |
| 2014_Aug_BNA669_BNA466 | | | | | | | |
| 2014_Aug_BNA87_BNA581 | | | | | | | |
| 2014_Aug_BNB074_J-1260 | | | | | | | |
| 2014_Aug_J-1253_BNB092 | | | | | | | |
| P-1940 | | | | | | | |
| P-1941 | | | | | | | |
| P-1946 | | 1 | 1 | 1 | 0 | 3 | |
| P-1948 | No change, needed | 3 | 1 | 3 | 1 | 8 | |
| P-1950 | No change, needed | 3 | 1 | 2 | 1 | 7 | |
| P-1956 | No change, needed | 2 | 1 | 0 | 0 | 3 | |
| P-1959 | No change, needed | 3 | 1 | 2 | 1 | 7 | |
| P-1964 | No change, needed | 2 | 1 | 0 | 0 | 3 | |
| P-1973 | Needed, changed length.. | 3 | 1 | 2 | 1 | 7 | |
| P-1981 | No change, needed | 3 | 1 | 2 | 1 | 7 | |
| P-1983 | Needed, verify construction status | 3 | 2 | 2 | 0 | 7 | |
| P-1987 | No change, needed | 3 | 2 | 2 | 0 | 7 | |
| P-1988 | No change, needed | 3 | 2 | 2 | 0 | 7 | |
| P-1990 | No change, needed | 3 | 2 | 2 | 0 | 7 | |
| P-1991 | No change, needed | 3 | 2 | 2 | 0 | 7 | |
| P-1993 | | | | | | | |
| P-1994 | | | | | | | |
| P-1995 | | | | | | | |
| P-2016 | No change, needed | 3 | 1 | 2 | 1 | 7 | |
| P-2018 | No change, needed | 1 | 0 | 0 | 0 | 1 | |
| P-2019 | No change, needed, low priority | 2 | 1 | 0 | 0 | 3 | |
| P-2020 | No change, needed, low priority | 2 | 1 | 0 | 0 | 3 | |
| P-2021 | No change, needed, low priority | 2 | 1 | 0 | 0 | 3 | |
| P-2030 | No change, needed | 3 | 0 | 2 | 0 | 5 | |
| P-2032 | No change, needed | 3 | 0 | 2 | 0 | 5 | |
| P-2057 | Needed however Native Title area risk. Construction approval risk | 2 | 0 | 2 | 1 | 5 | |
| P-2069 | No change, needed | 3 | 1 | 3 | 2 | 9 | |
| P-2071 | | 3 | 0 | 2 | 0 | 5 | |
| P-2077 | | | | | | | |
| P-2078 | | | | | | | |
| P-2081 | | | | | | | |
| P-2082 | | | | | | | |
| P-2087 | | | | | | | |
| P-2088 | New DN150, only needed for 500 2036 | 3 | 0 | 2 | 0 | 5 | |
| P-2089 | New DN150 aug for cross connection | 3 | 2 | 2 | 0 | 7 | |
| P-2090 | New DN100 aug for cross connection | 3 | 2 | 2 | 0 | 7 | |
| P-2091 | New DN200 to replace bad pipe | 5 | 5 | 5 | 0 | 15 | YES |
| P-2124 | | | | | | | |
| P-2125 | | | | | | | |
| P-2126 | | | | | | | |
| P-2127 | | | | | | | |
| | 2 isolation valves added | 3 | 0 | 2 | 0 | 5 | |
| | Augments to area, pipes deactivated | | | | | | |
| | Augs needed and tank change for Herenvale | | | | | | |
| | Council policy changes needed for Whitsunday Shores - Yeaston to apply policy change for elevated are and low pressure | | | | | | |
| | New aug to bulk water treatment pump - 230L/s | | | | | | |
| | Koorelah Farms - Inverdon Road - Farming irrigation, nodes removed | | | | | | |
| | Expired ag contracts, loading reduced/removed | | | | | | |
| | No fireflow provision made to rural domestic supply/standard flow performance up to DSS only. Bottom of Smith Road, changes made. | | | | | | |

PROJECT: Whitsundays Water Network Modelling
DOCUMENT NUMBER: D008-10027536-01
DATE: 03.02.2020

Project Engineer: M.C/S.H
Software: Bentley WaterCAD v8i



**COLLINSVILLE POTABLE WATER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT**

| ID | Change from initial 2019 LGIP | Need to Service | Operational (Known Bursts/Construction Fails) | Redundancy/Management | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP |
|---|-------------------------------|-----------------|---|-----------------------|---|-------------|------|
| P-23 | Needed, no change | 4 | 1 | 2 | 0 | 7 | No |
| P-30 | Needed, no change | 3 | 1 | 2 | 0 | 6 | No |
| P-35 | Needed, no change | 3 | 1 | 2 | 0 | 6 | No |
| P-42 | Needed, no change | 0 | 0 | 0 | 0 | 0 | No |
| P-43 | New DN150 | 2 | 1 | 2 | 0 | 5 | No |
| P-44 | New DN150 | 2 | 1 | 2 | 0 | 5 | No |
| P-47 | New DN150 | 2 | 1 | 2 | 0 | 5 | No |
| P-50 | New DN150 | 4 | 1 | 2 | 0 | 7 | No |
| P-53 | New DN100 | 4 | 1 | 2 | 0 | 8 | No |
| P-56 | New DN150 | 4 | 1 | 2 | 0 | 7 | No |
| P-57 | New DN150 | 4 | 1 | 2 | 0 | 7 | No |
| P-58 | New DN100 | 4 | 1 | 2 | 0 | 7 | No |
| P-61 | New DN 100 | 3 | 1 | 2 | 0 | 6 | No |
| P-629 | Needed, no change | 4 | 1 | 2 | 0 | 7 | No |
| P-630 | Needed, no change | 4 | 1 | 2 | 0 | 7 | No |
| P-630 | New DN160 | | | | | | No |
| P-662 | No change, needed | 3 | 1 | 2 | 0 | 6 | No |
| Alternative hydrant position | | | | | | | |
| CLC82 removed to CLC81 for Collinsville Caravan Park | | | | | | | |
| Isolation Valves to be confirmed and changed. Pipes to be deactivated | | | | | | | |

PROJECT:
DOCUMENT NUMBER:
DATE:

Whitsundays Sewer Network Modelling
D009-10027536-01
03.02.2020

Project Engineer: M.C/S.H
Software: InfoSWMM SA



**CANNONVALE SEWER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT**

| ID | Change from initial 2019 LGIP | Need to Service | Operational (Known Bursts/Construction Fails) | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP | Notes |
|-------------|--|-----------------|---|---|-------------|------|---------------------|
| CDT-71 | upgrade as per priority | 2 | 2 | 5 | 9 | Yes | |
| CDT-77 | stays same | 2 | 2 | 5 | 9 | Yes | |
| CDT-79 | upgrade as per priority | 2 | 1 | 5 | 8 | Yes | |
| CDT-81 | upgrade as per priority | 2 | 1 | 5 | 8 | No | |
| CDT-85 | upgrade as per priority | 2 | 1 | 5 | 8 | Yes | |
| CDT-113 | maintain upgrade as per priority | 3 | 2 | 5 | 10 | Yes | |
| CDT-115 | maintain upgrade as per priority | 3 | 2 | 5 | 10 | Yes | |
| CDT-117 | maintain upgrade as per priority | 3 | 2 | 5 | 10 | Yes | |
| CDT-119 | adopt as required. | 2 | 2 | 5 | 12 | No | |
| CDT-121 | adopt as required. | 2 | 2 | 5 | 12 | Yes | |
| CDT-123 | adopt as required. | 2 | 2 | 5 | 12 | Yes | |
| CDT-131 | adopt as required. | 2 | 2 | 5 | 12 | No | |
| CDT-135 | replace as a 200 gravity line note apply plastic to avoid HS2 impacts...note Hs2 | 2 | 1 | 1 | 4 | Yes | |
| CDT-139 | as required, additional existing alignment investigation may be required | 2 | 2 | 5 | 12 | Yes | |
| CDT-145 | adopt as required. | 2 | 2 | 5 | 12 | Yes | |
| CDT-147 | don't duplicate, replace with a 425 PVC. | 3 | 3 | 5 | 11 | No | |
| CDT-149 | | 3 | 3 | 5 | 11 | Yes | |
| CDT-151 | | 3 | 3 | 5 | 11 | Yes | |
| CDT-153 | | 3 | 3 | 5 | 11 | Yes | |
| CDT-155 | | 3 | 3 | 5 | 11 | Yes | |
| CDT-157 | | 3 | 3 | 5 | 11 | Yes | |
| CDT-159 | | 3 | 3 | 5 | 11 | Yes | |
| CDT-167 | | 3 | 3 | 5 | 11 | Yes | |
| SM_P_3076 | | | | | | Yes | |
| PUMP-JUBI1 | Augment - Pump in-line to be upgraded pre 2021 | 4 | 5 | 5 | 14 | Yes | PUMP CAPACITY |
| PUMP-JUBI2 | apply augment | 3 | 2 | 5 | 10 | | PUMP CAPACITY |
| PUMP-CANN12 | | 3 | 3 | 3 | 9 | | OPERATIONAL STORAGE |

PROJECT: Whitsundays Sewer Network Modelling
DOCUMENT NUMBER: D009-10027536-01
DATE: 03.02.2020

Project Engineer: M.C/S.H
Software: InfoSWMM SA



**BOWEN SEWER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT**

| ID | Change from initial 2019 LGIP | Need to Service | Operational (Known Bursts/Construction Fails) | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP |
|--------|--|-----------------|---|---|-------------|------|
| CDT-31 | No change, needed | 3 | 3 | 2 | 8 | No |
| CDT-39 | New DN pipe frpm PS3 towards PS1, existing pipe extremely worn | 2 | 5 | 2 | 9 | Yes |
| | | | | | | |

PROJECT:
DOCUMENT NUMBER:
DATE:

Whitsundays Sewer Network Modelling
D009-10027536-01
03.02.2020

Project Engineer: M.C/S.H
Software: InfoSWMM SA



**PROSERPINE SEWER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT**

| ID | Change from initial 2019 LGIP | Need to Service | Operational (Known Bursts/Construction Fails) | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP | Notes |
|-------------------|---|-----------------|---|---|-------------|------|---------------------|
| CDT-87 | Needed but don't parrallel. Go Replace | 5 | 4 | 3 | 12 | No | |
| CDT-91 | No change, needed augmenting but must be poly because of school/trees etc | 5 | 4 | 3 | 12 | Yes | |
| CDT-93 | No change, needed augmenting but must be poly because of school/trees etc | 5 | 4 | 3 | 12 | Yes | |
| DEACTIVATE CDT-15 | | 5 | 4 | 3 | 12 | Yes | |
| CDT-99 | New DN355 pipe aug to treatment plant to avoid Bruce Highway | 5 | 3 | 4 | 12 | Yes | |
| CDT-101 | New DN355 pipe aug to treatment plant to avoid Bruce Highway | 5 | 3 | 4 | 12 | Yes | |
| CDT-103 | Augment is needed | | | | | No | |
| PROS_PS1 | | 3 | 3 | 3 | 9 | Yes | PUMP CAPACITY |
| PROS_PS12 | | 3 | 3 | 3 | 9 | Yes | PUMP CAPACITY |
| PROS_PS3 | | 3 | 3 | 3 | 9 | Yes | PUMP CAPACITY |
| PROS_6 | | 3 | 3 | 3 | 9 | Yes | PUMP CAPACITY |
| PROS_PS9 | Needed | 5 | 4 | 3 | 12 | Yes | PUMP CAPACITY |
| PROS_PS2 | Pump already updated, additional pump aug only required if augment CDT-103 does not take place. | 3 | 3 | 3 | 9 | Yes | PUMP CAPACITY |
| PROS_PS12 | | | | | | | OPERATIONAL STORAGE |
| PUMP-11 | | | | | | | OPERATIONAL STORAGE |

PROJECT:
DOCUMENT NUMBER:
DATE:

Whitsundays Sewer Network Modelling
D009-10027536-01
03.02.2020

Project Engineer: M.C/S.H
Software: InfoSWMM SA

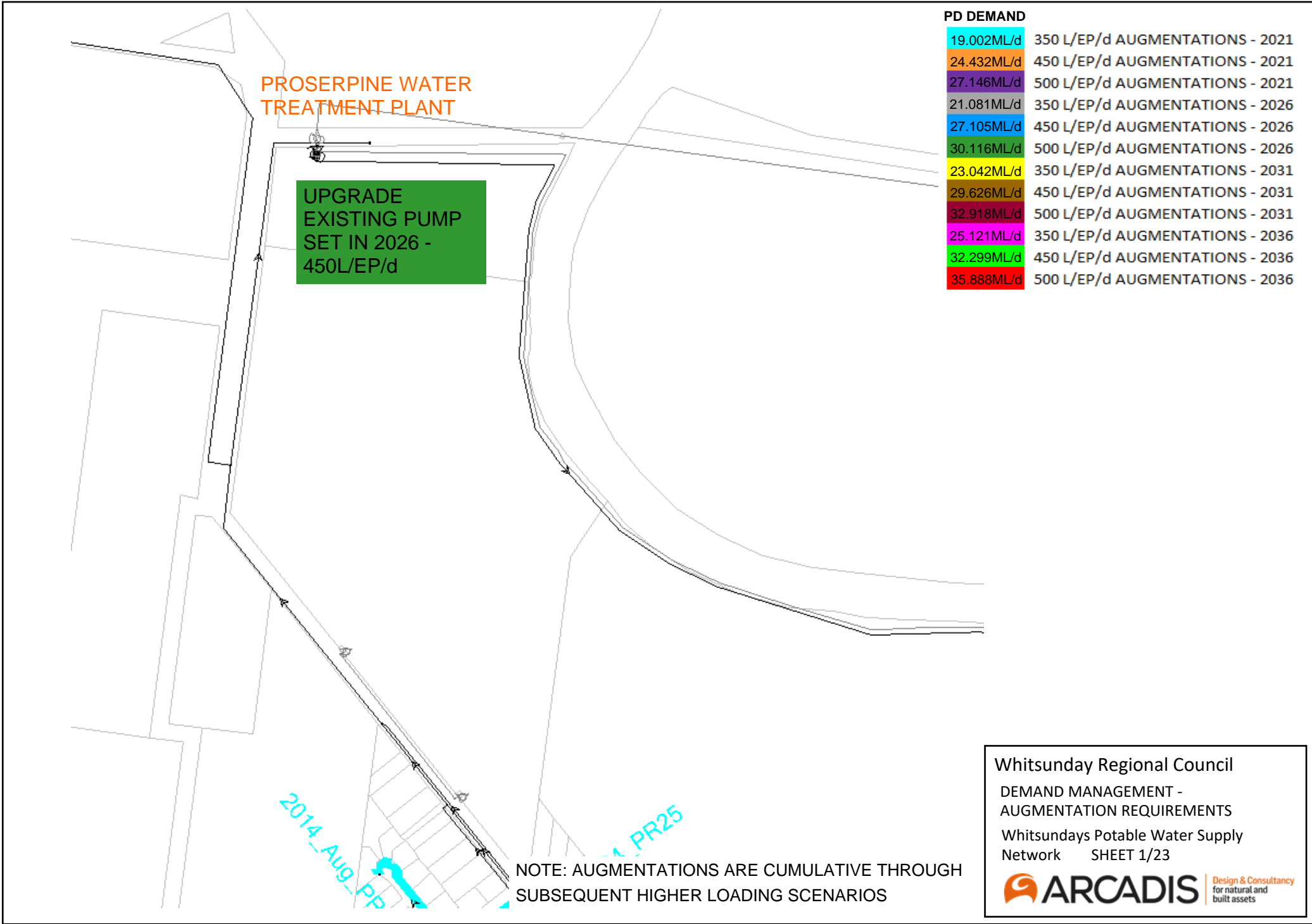


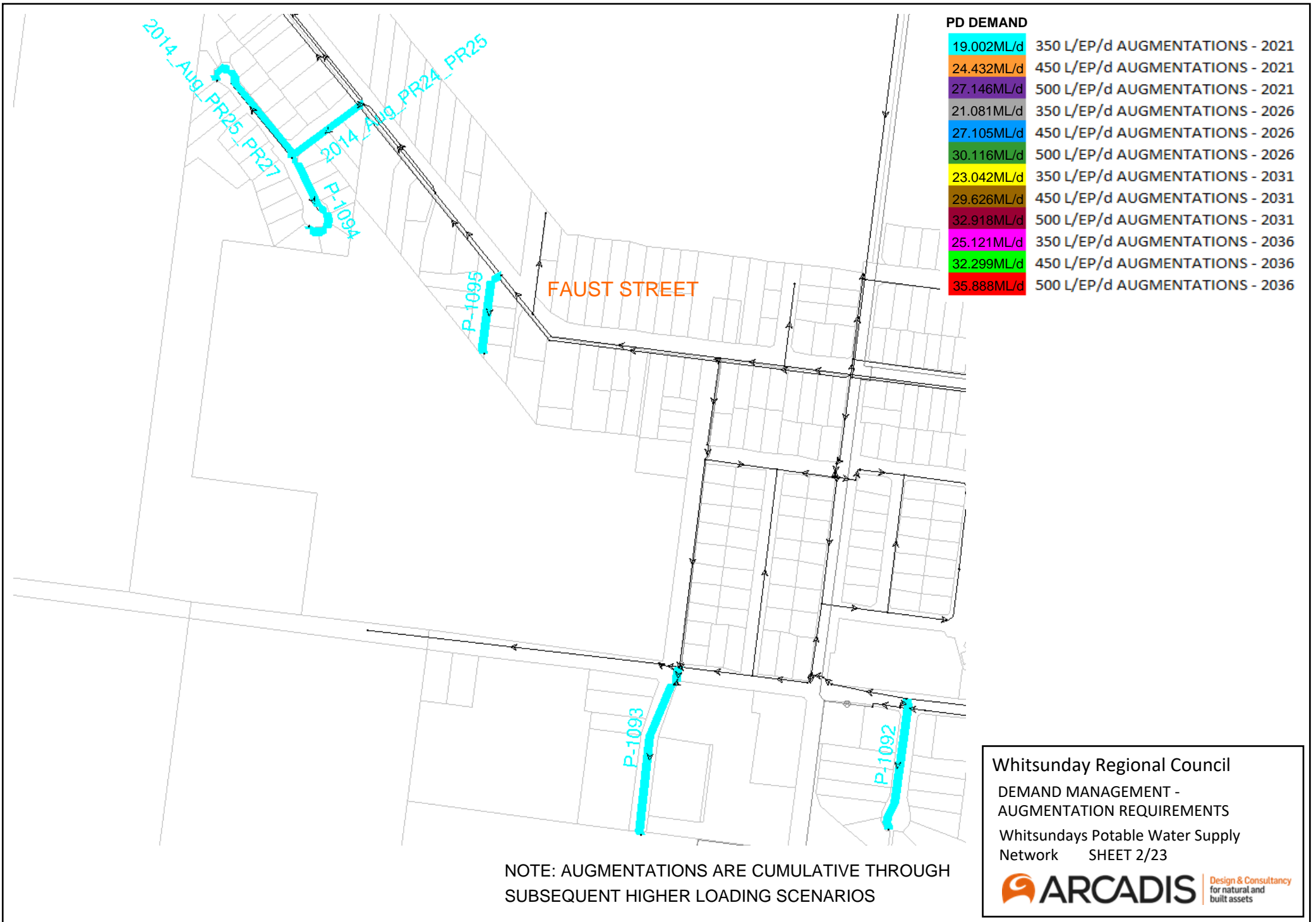
**COLLINSVILLE SEWER NETWORK
2020 LGIP OPTIMISATION - MCA ASSESSMENT**

| ID | Change from initial 2019 LGIP | Need to Service | Operational (Known Bursts/Construction Fails) | Realistic Growth/Ability to Service/Climate Adaption? | Total Score | LGIP |
|-----------|-------------------------------|-----------------|---|---|-------------|------|
| CDT-27 | | 1 | 1 | 0 | 2 | No |
| 2-C_PUMPS | | 2 | 2 | 0 | 4 | Yes |

APPENDIX F

INITIAL WATER NETWORK AUGMENTATION OUTPUTS PRE- WORKSHOP

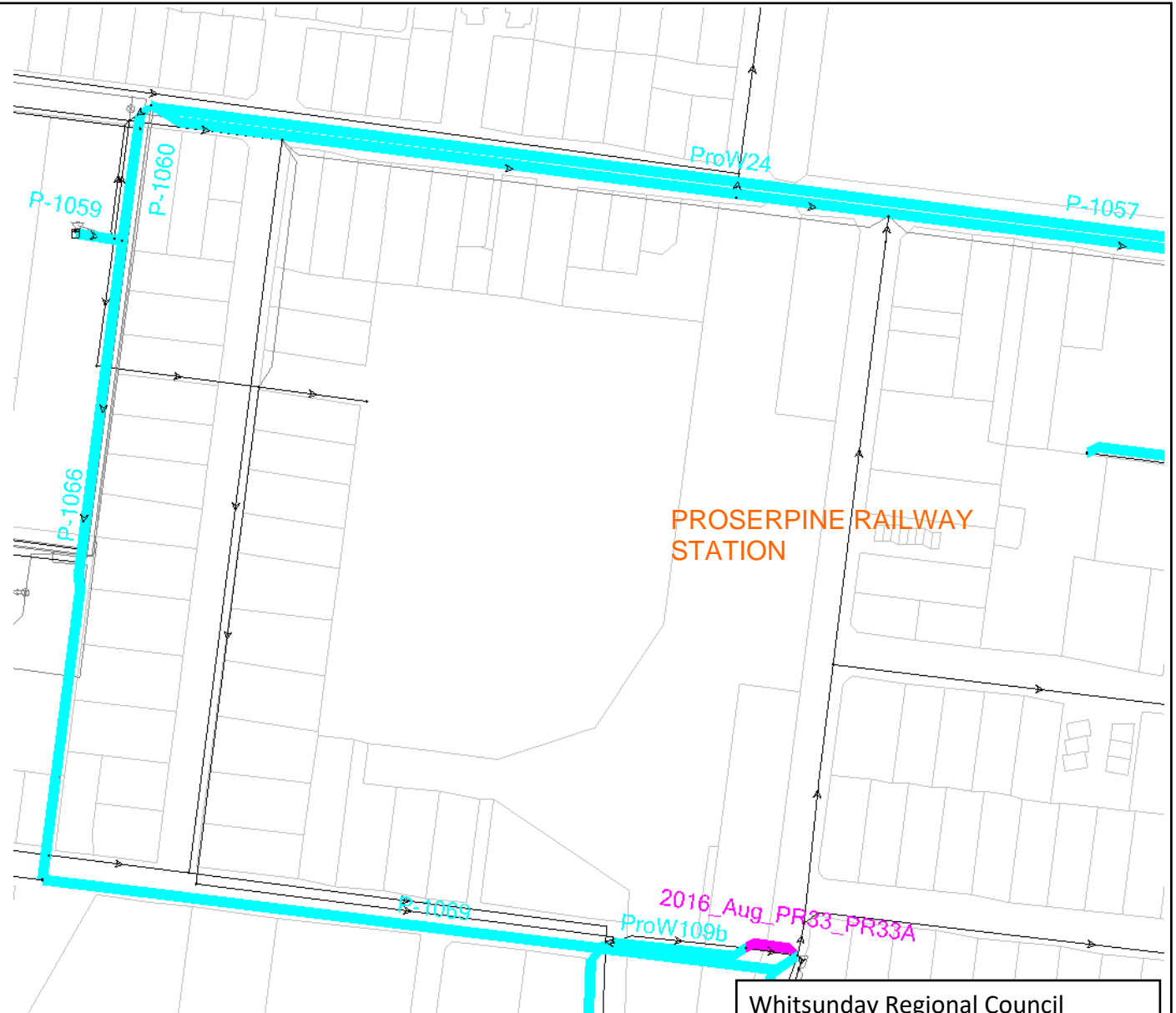




NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
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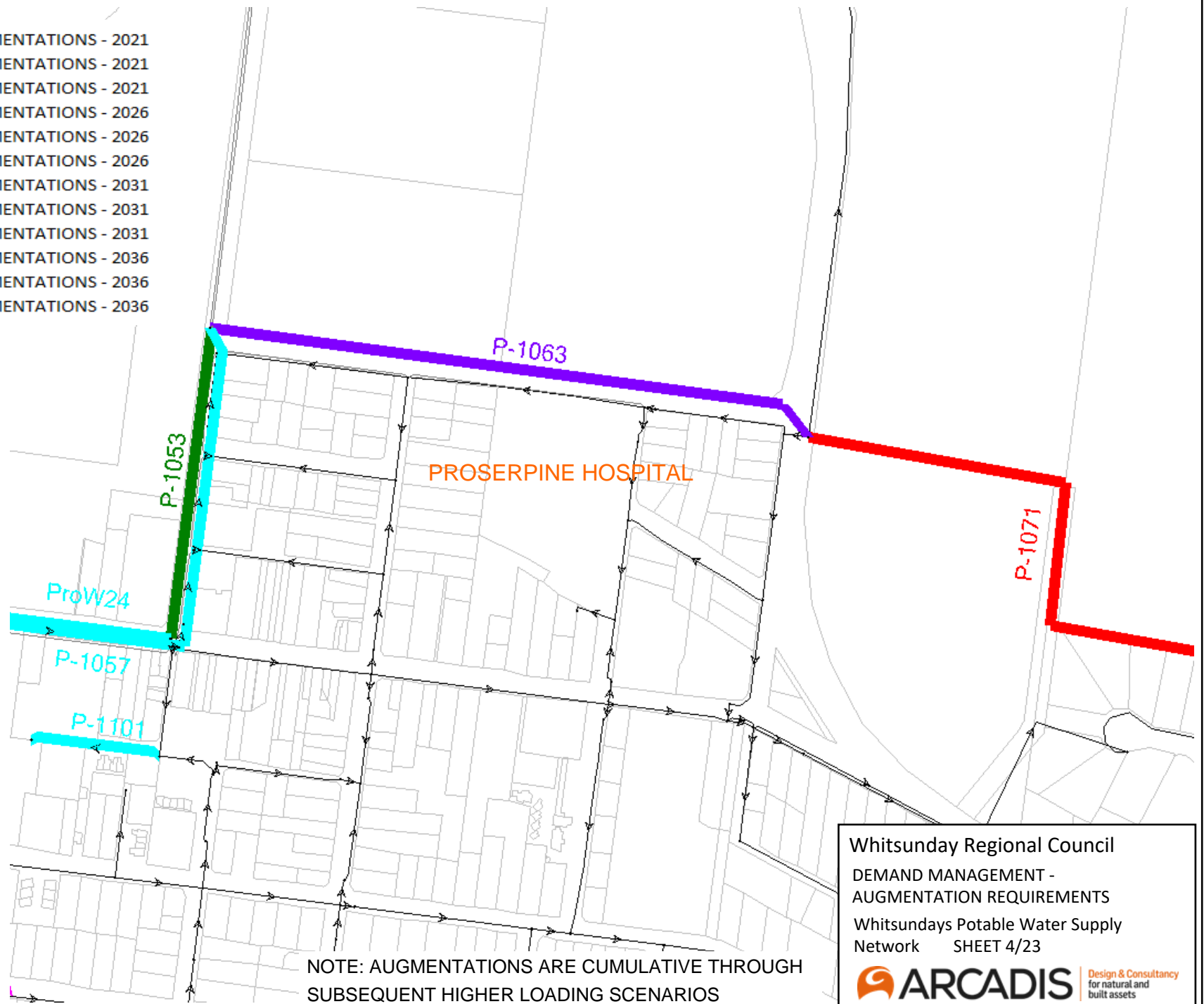
Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 3/23

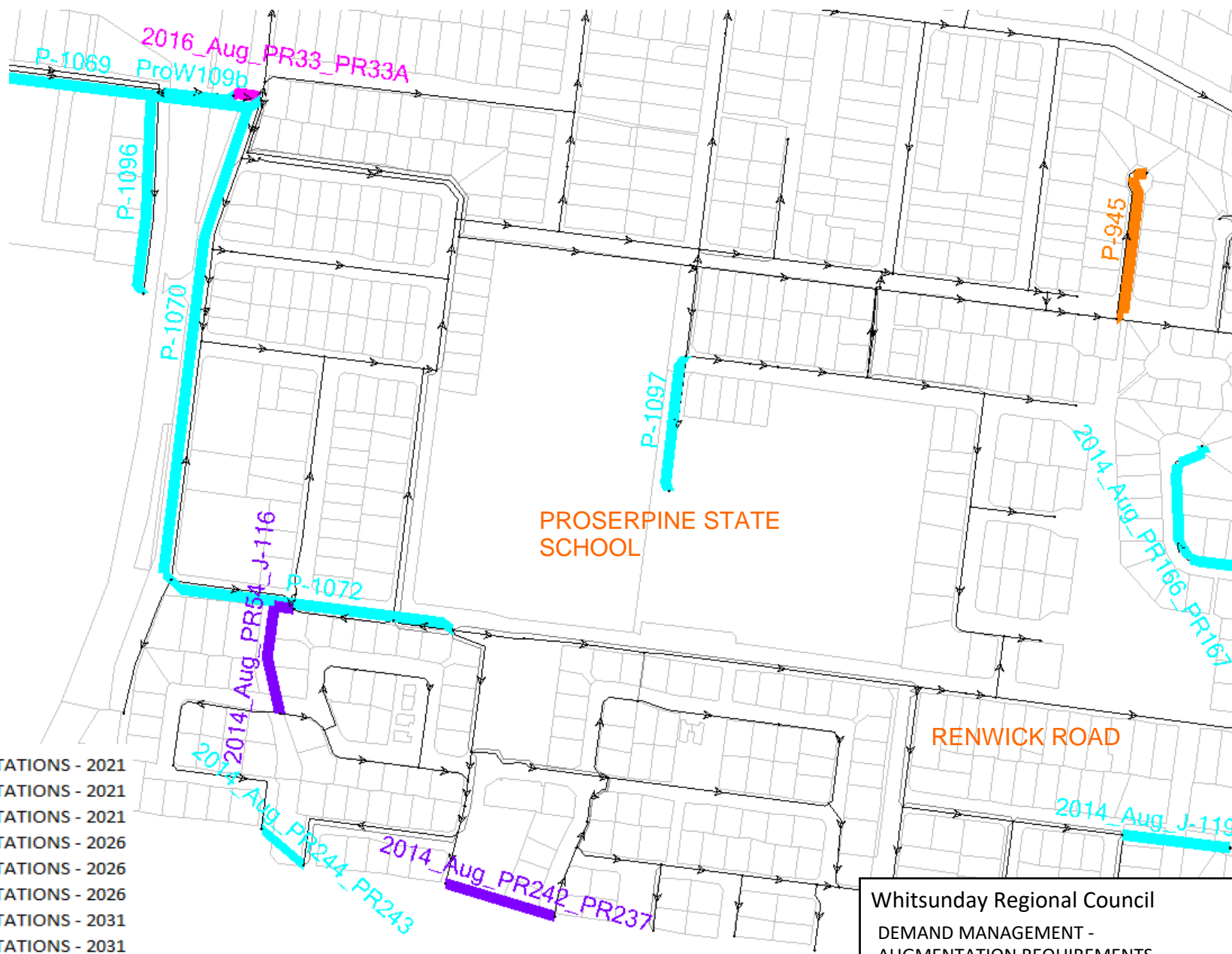


Design & Consultancy
for natural and
built assets

PD DEMAND

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PD DEMAND

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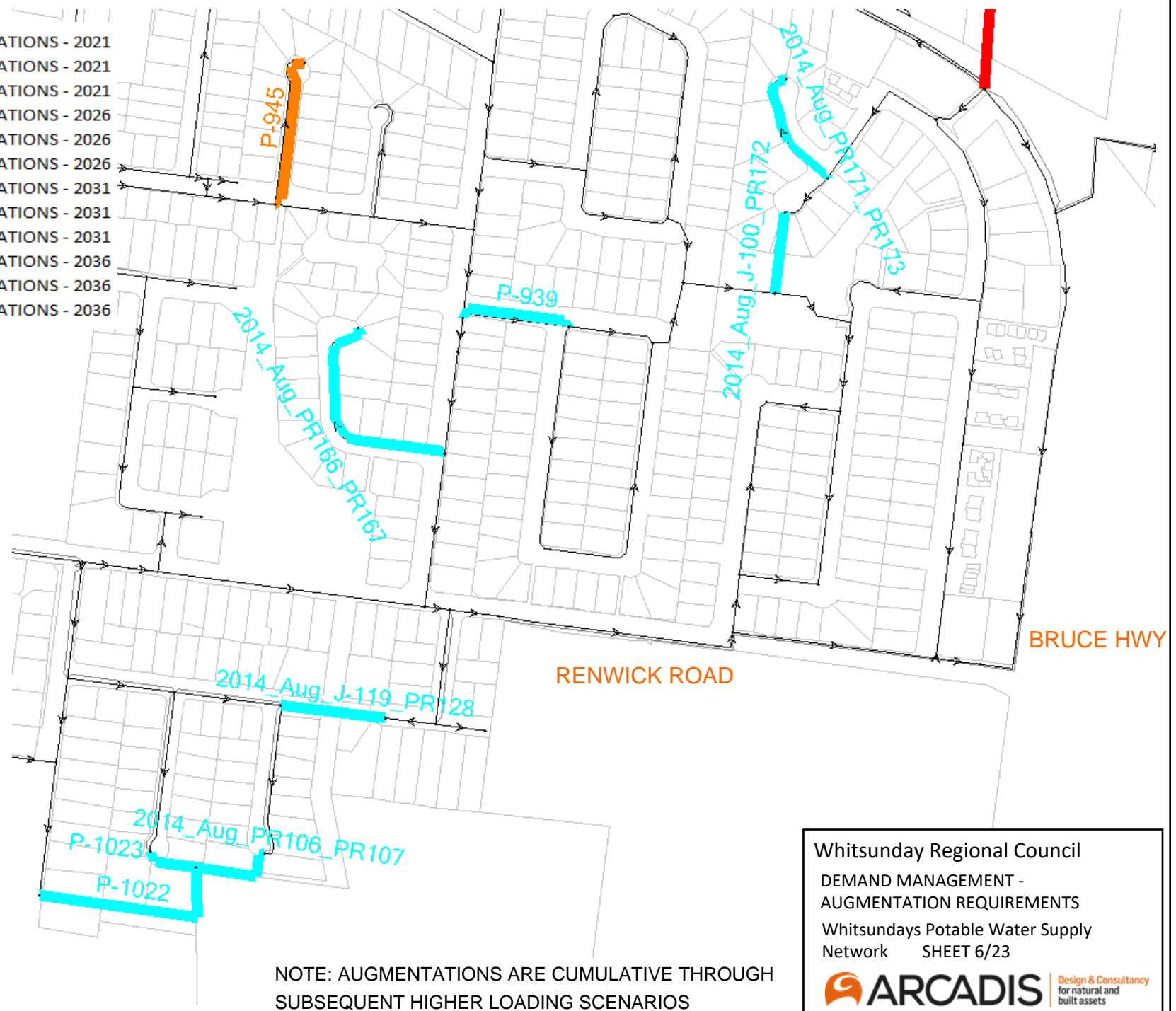
Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 5/23



Design & Consultancy
 for natural and
 built assets

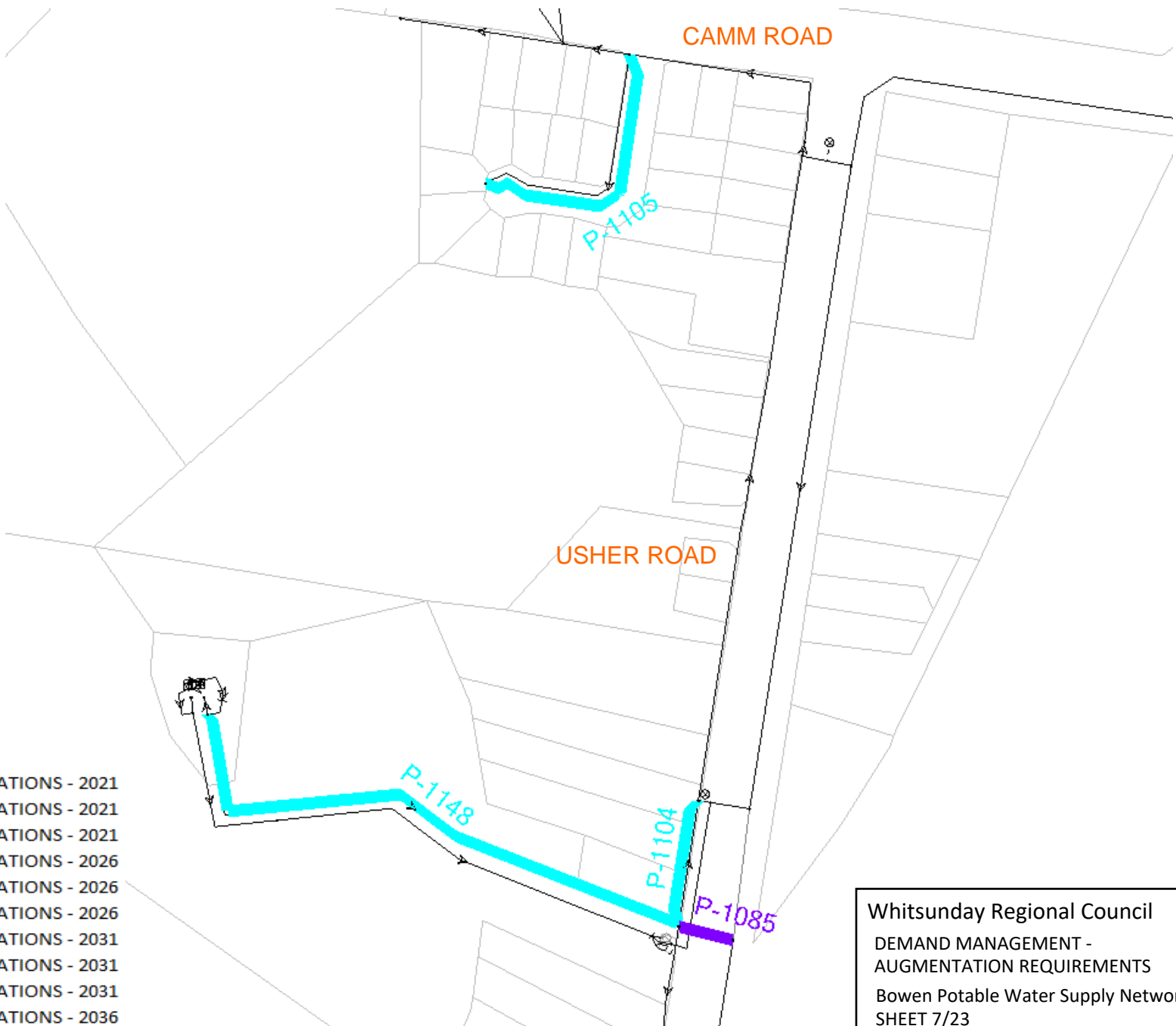
PD DEMAND

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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 6/23



PD DEMAND

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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 7/23

AITKEN ROAD

SHUTE HARBOUR ROAD

UPGRADE
EXISTING
BOOSTER PUMP
SET IN 2036 -
500L/EP/day

PD DEMAND

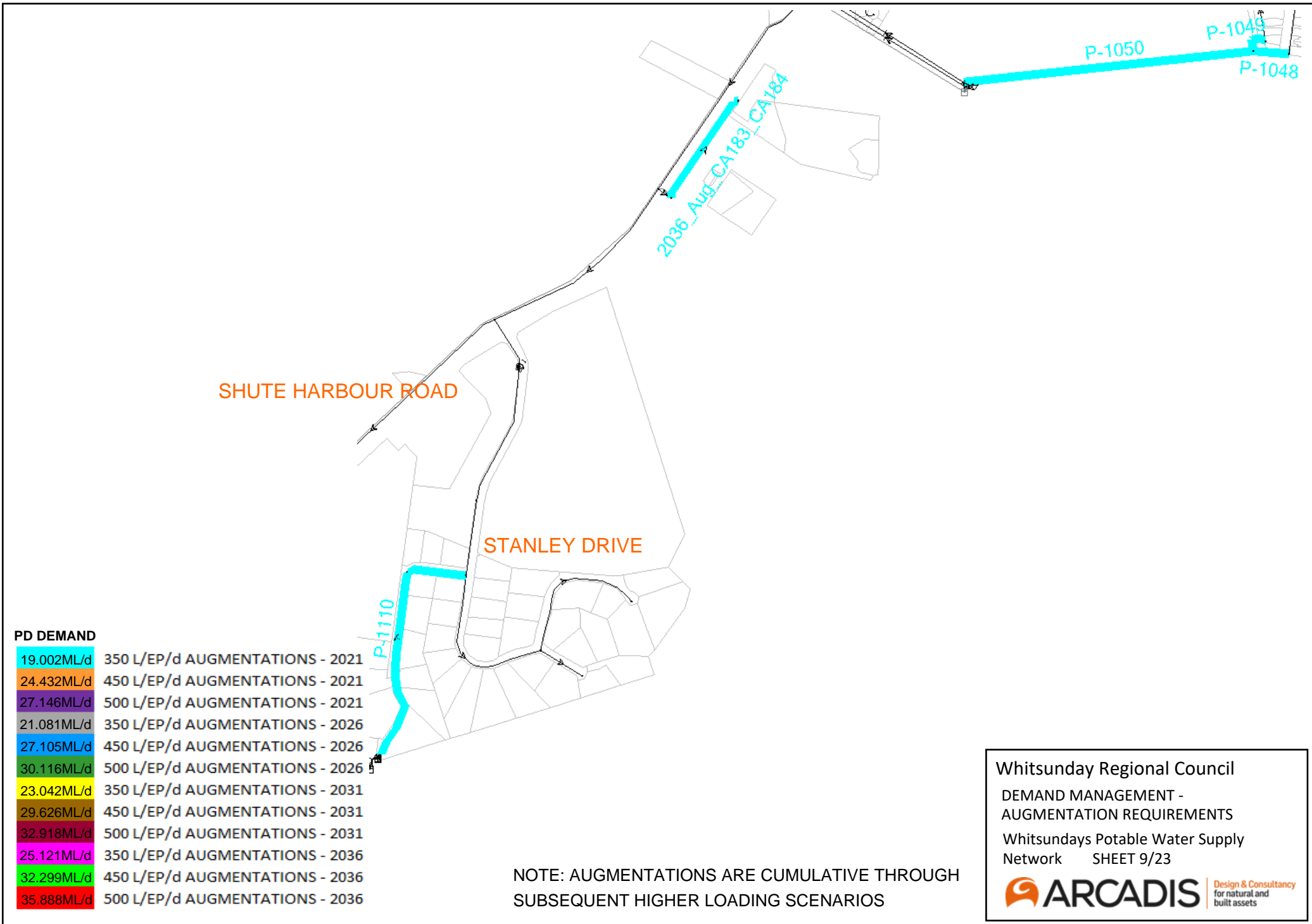
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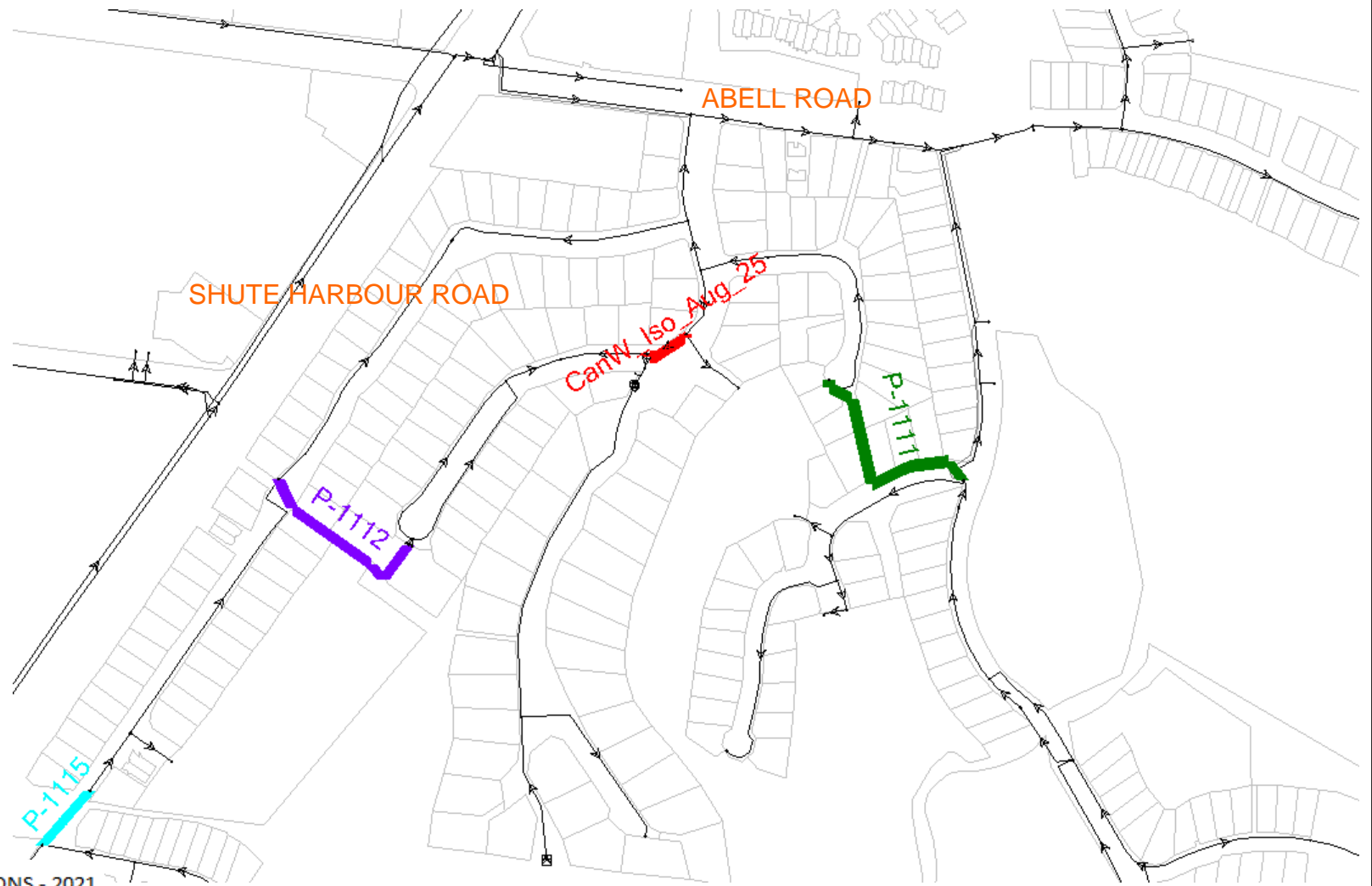
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Bowen Potable Water Supply Network
SHEET 8/23



Design & Consultancy
for natural and
built assets





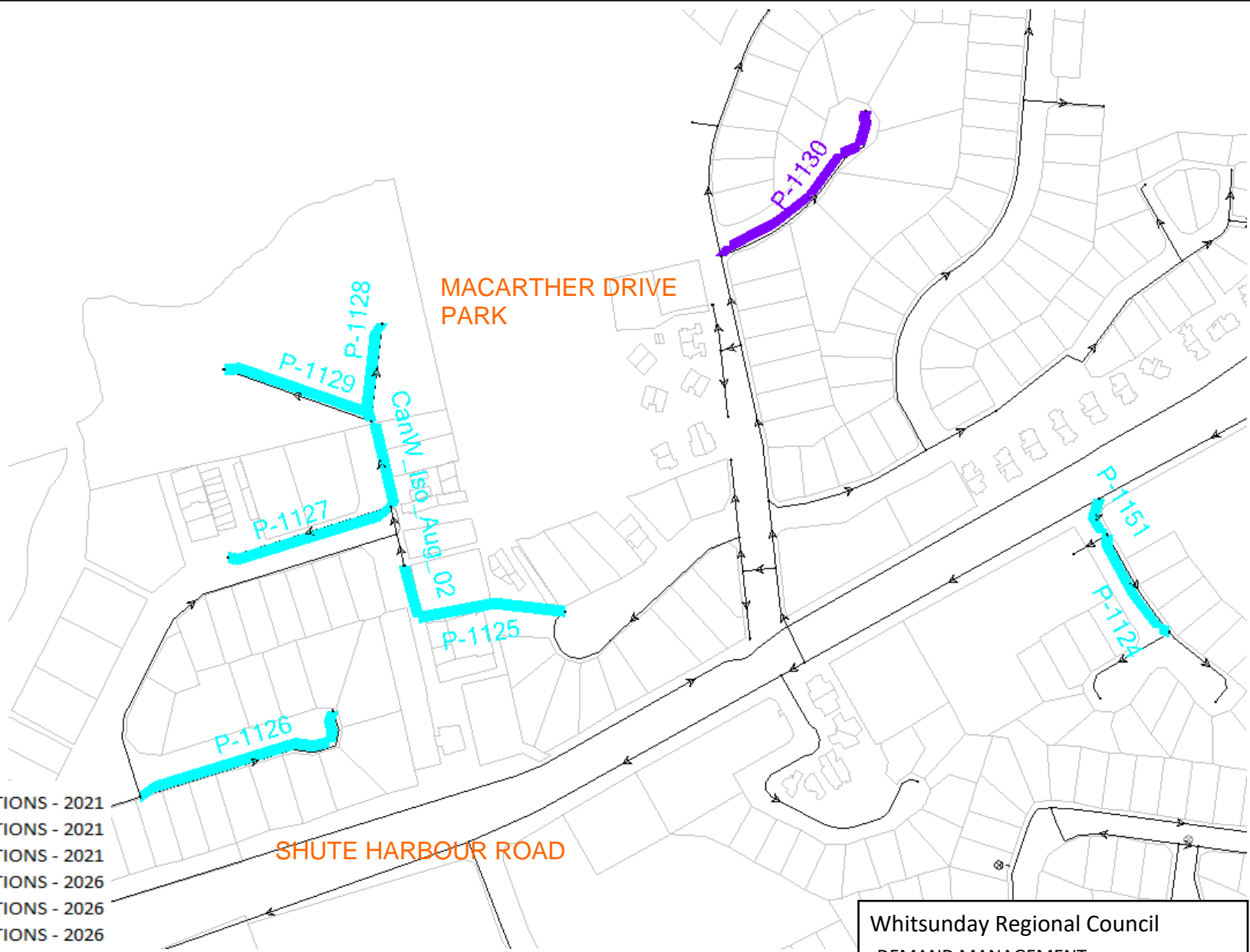
PD DEMAND

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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 10/23





PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
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SHUTE HARBOUR ROAD

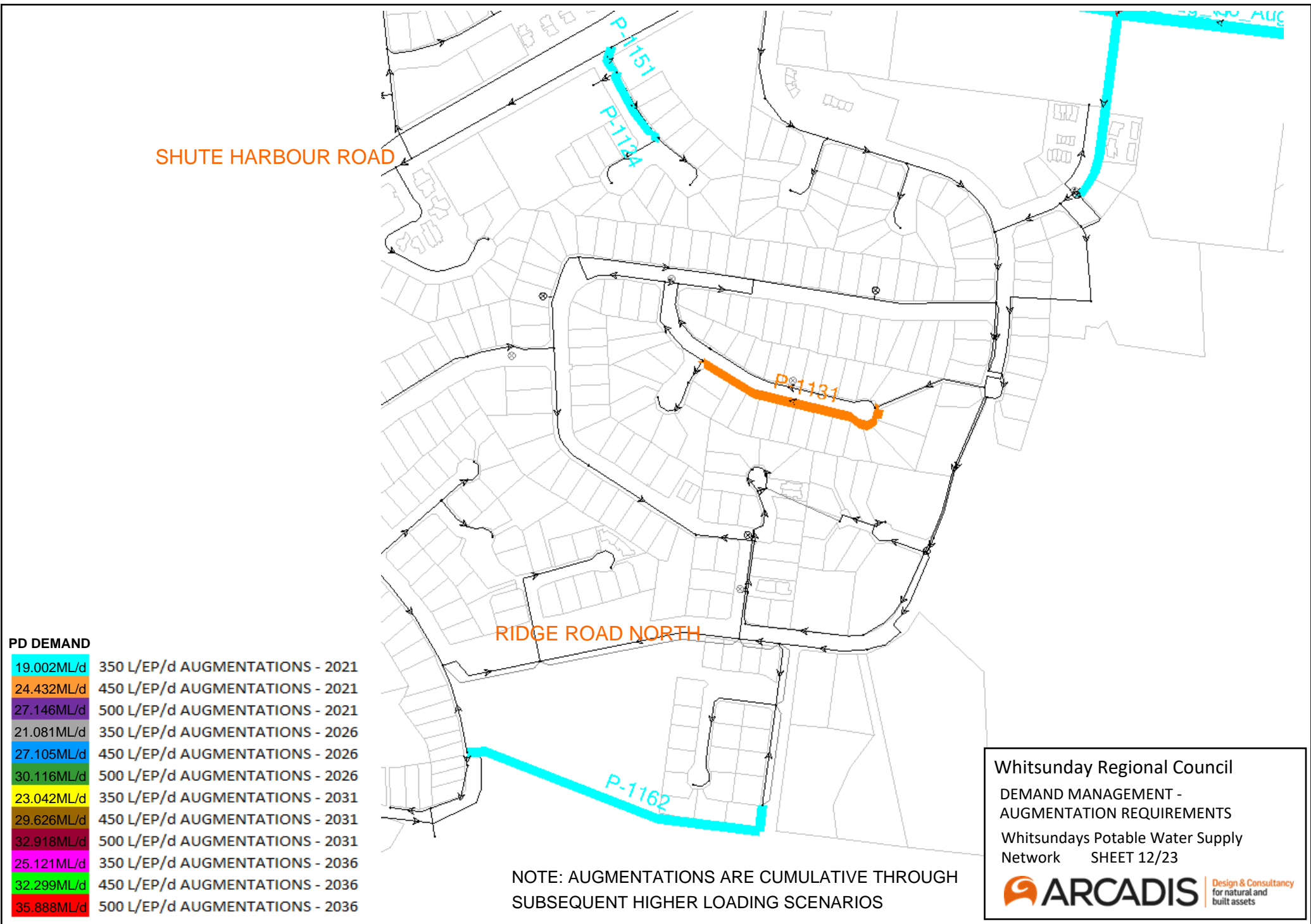
MACARTHER DRIVE PARK

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 11/23



Design & Consultancy for natural and built assets



| PD DEMAND | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
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NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 12/23



Design & Consultancy for natural and built assets

SHUTE HARBOUR ROAD

CanW_Iso_Aug_27

CanW_Iso_Aug_26

CanW_Iso_Aug_17


TROPIC ROAD NORTH

P-1135
P-1133
P-1134
P-867

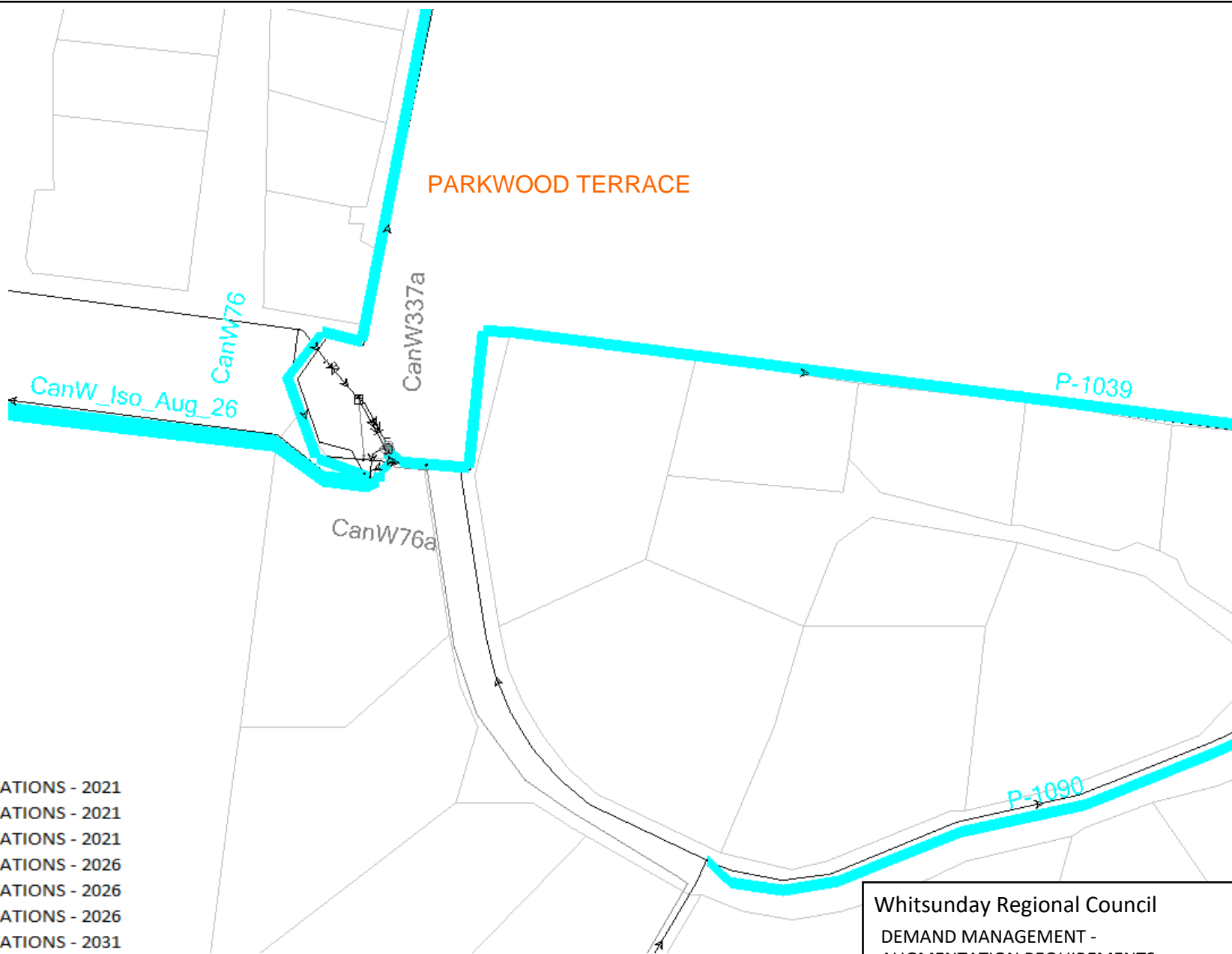
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|------------|---------------------------------|
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| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 13/23



Design & Consultancy
for natural and
built assets



PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

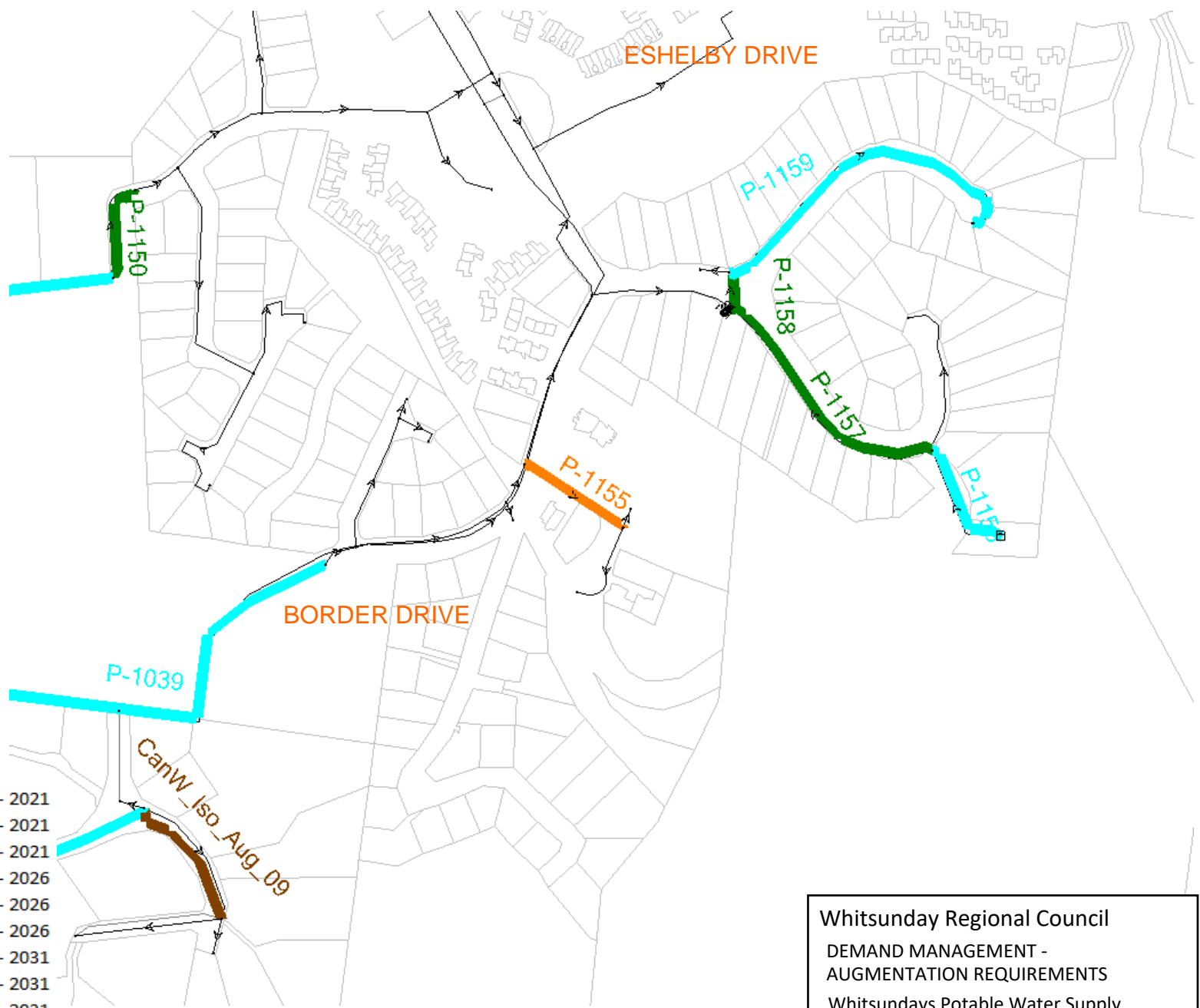
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 14/23





PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

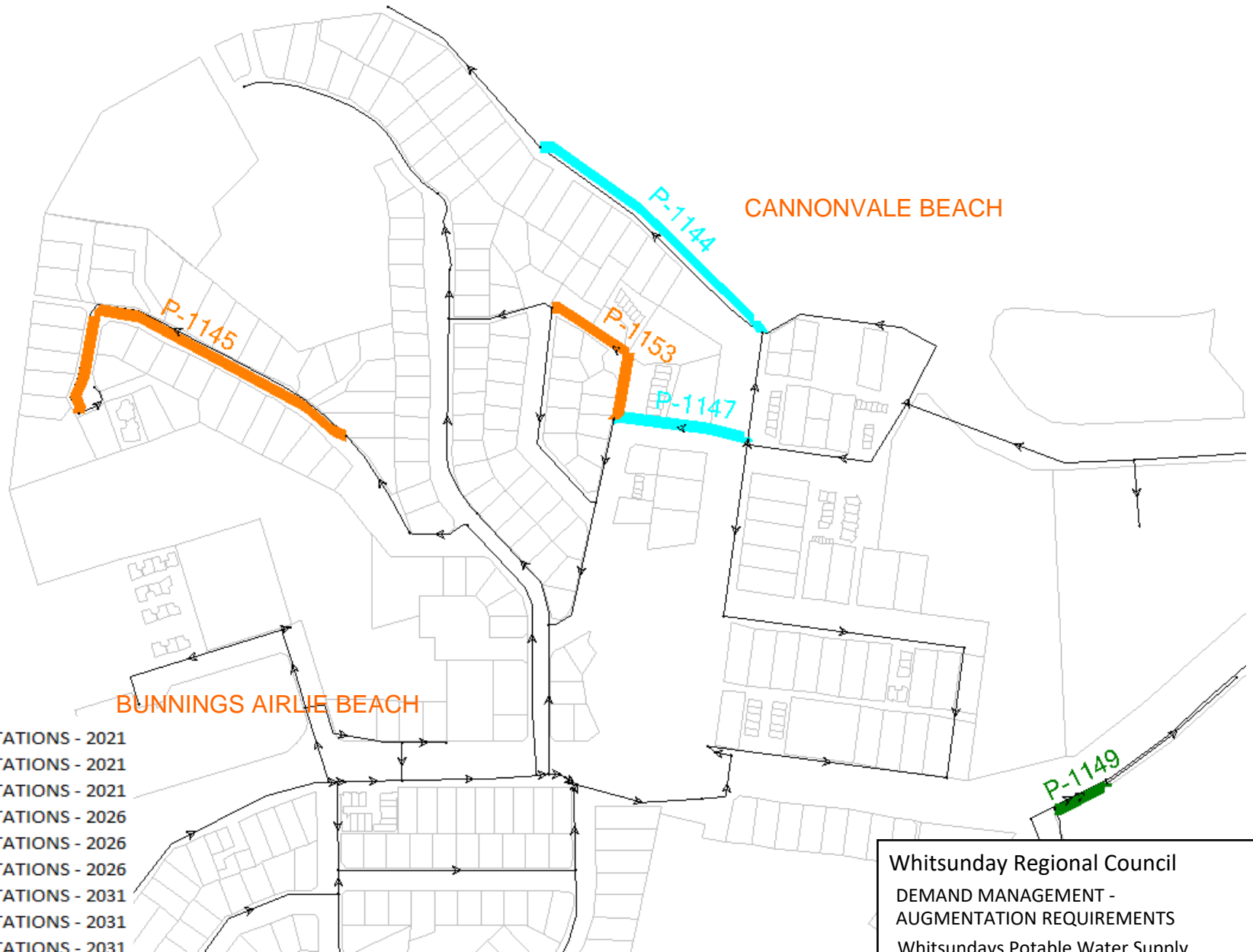
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 15/23





PD DEMAND

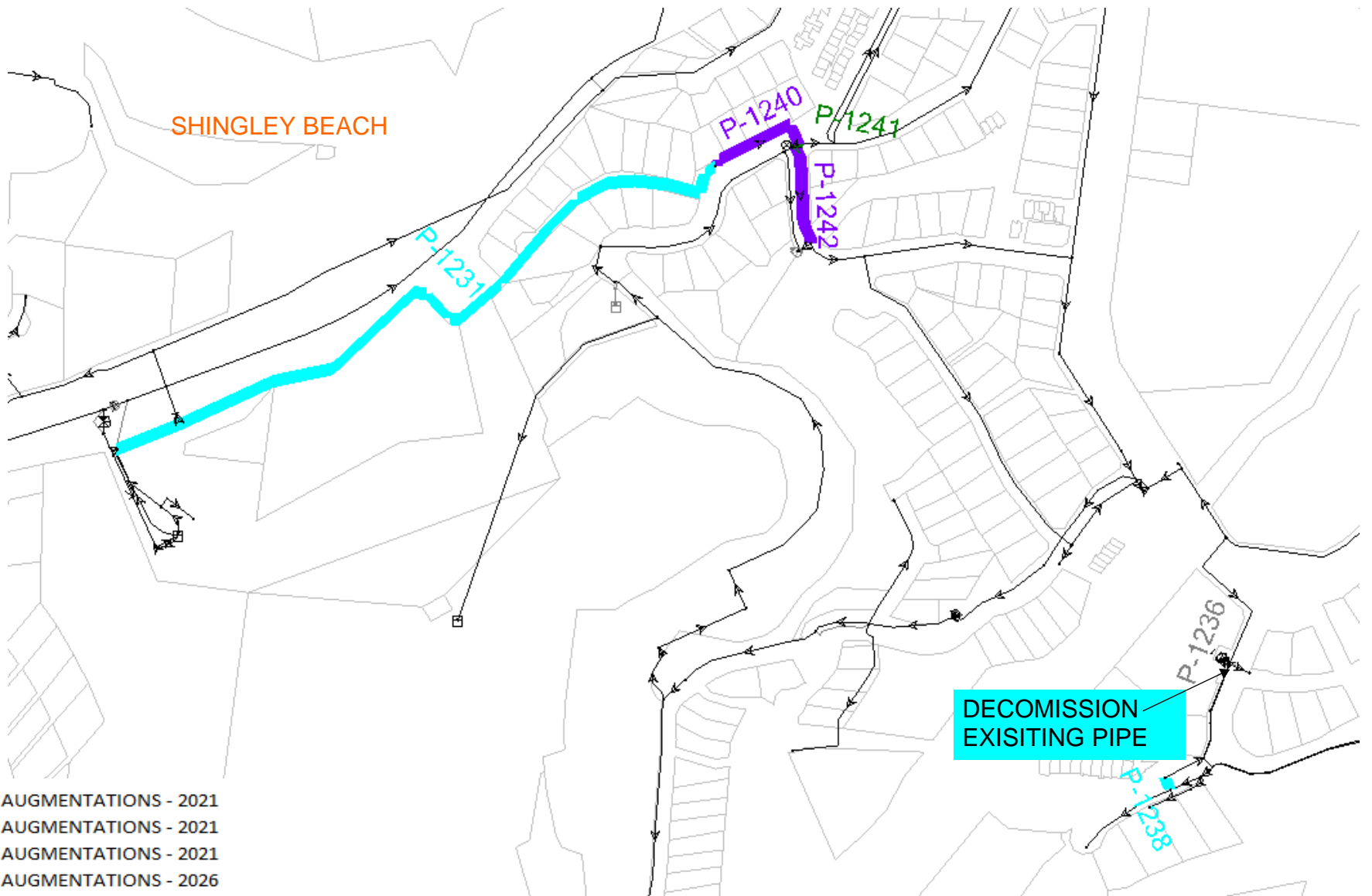
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|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 16/23



SHINGLEY BEACH




PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

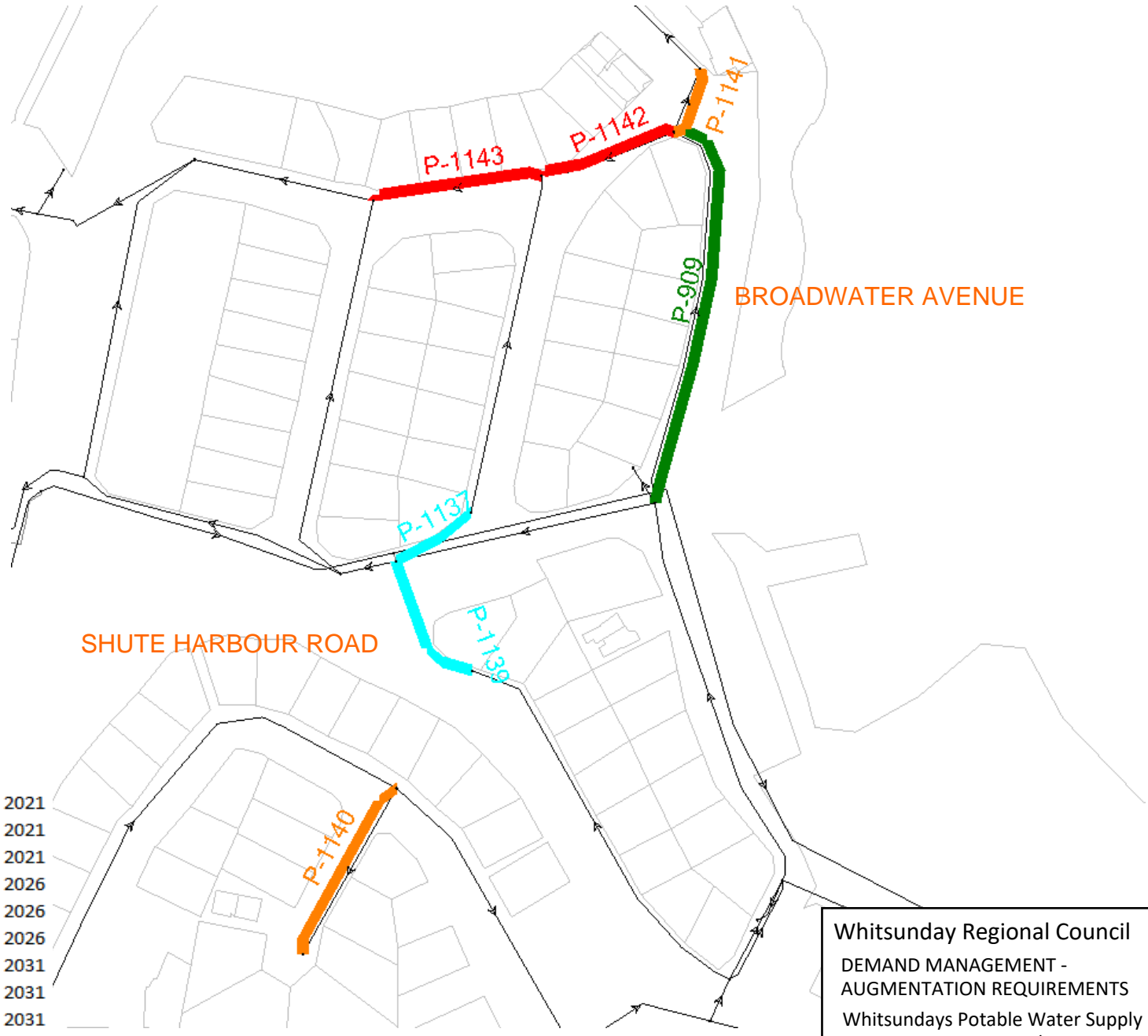
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

DECOMMISSION EXISTING PIPE

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Whitsundays Potable Water Supply
Network SHEET 17/23



Design & Consultancy
for natural and
built assets




PD DEMAND

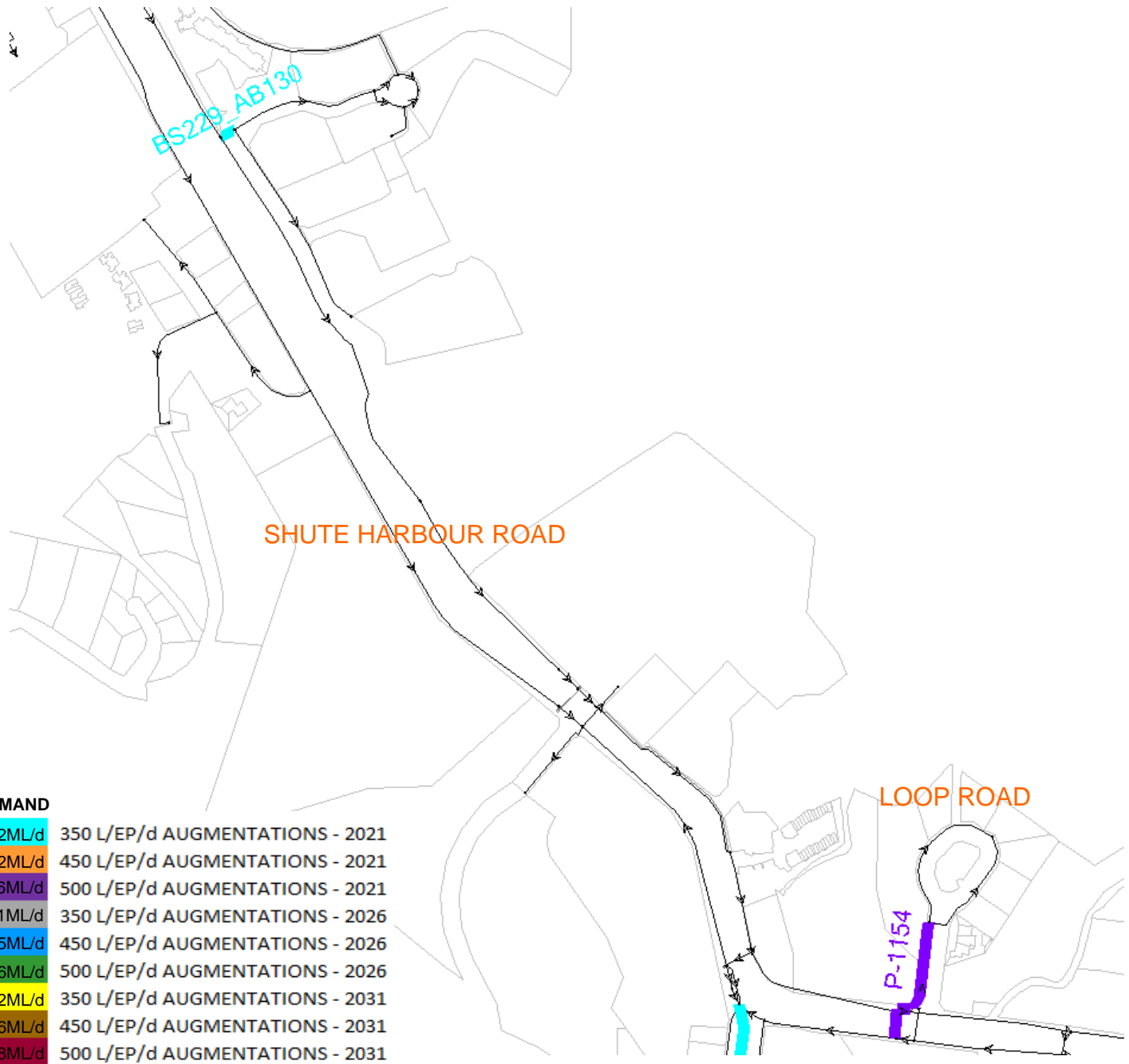
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|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 18/23



Design & Consultancy
for natural and
built assets



PD DEMAND

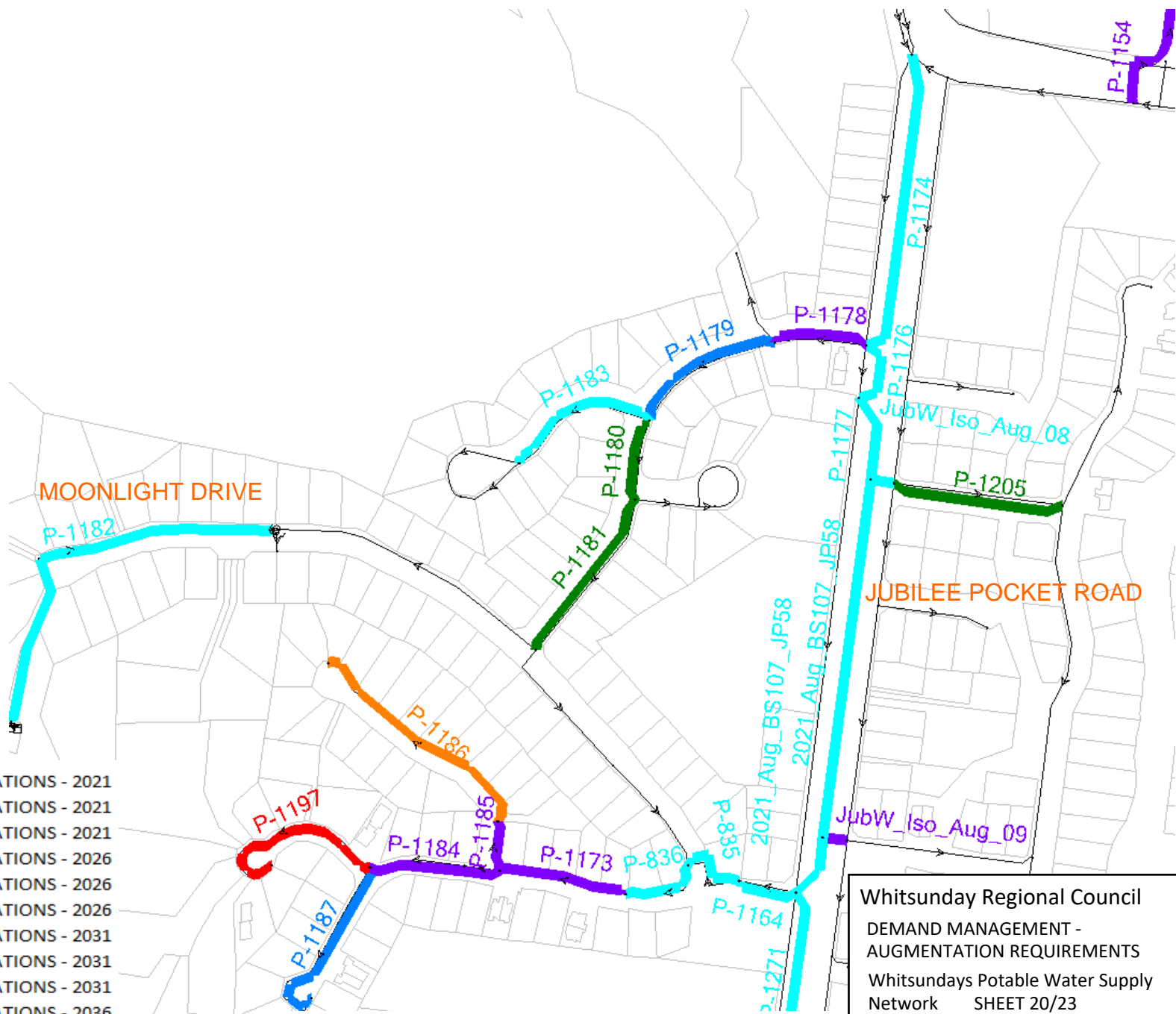
| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 19/23



Design & Consultancy
for natural and
built assets



PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
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| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

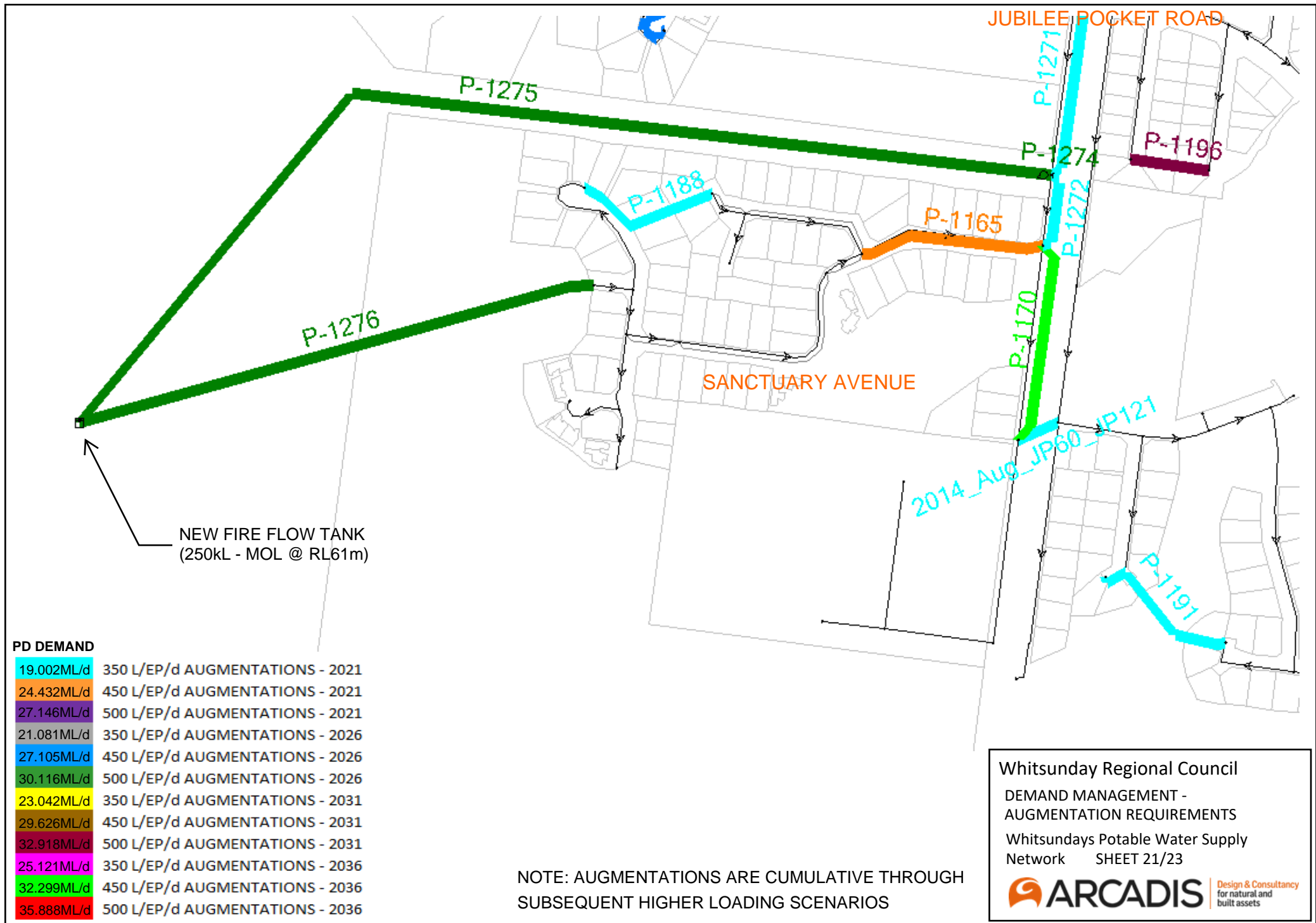
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS

Whitsundays Potable Water Supply
Network SHEET 20/23





AIRLIE BEACH SWIM CENTRE

SHUTE HARBOUR ROAD

| PD DEMAND | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

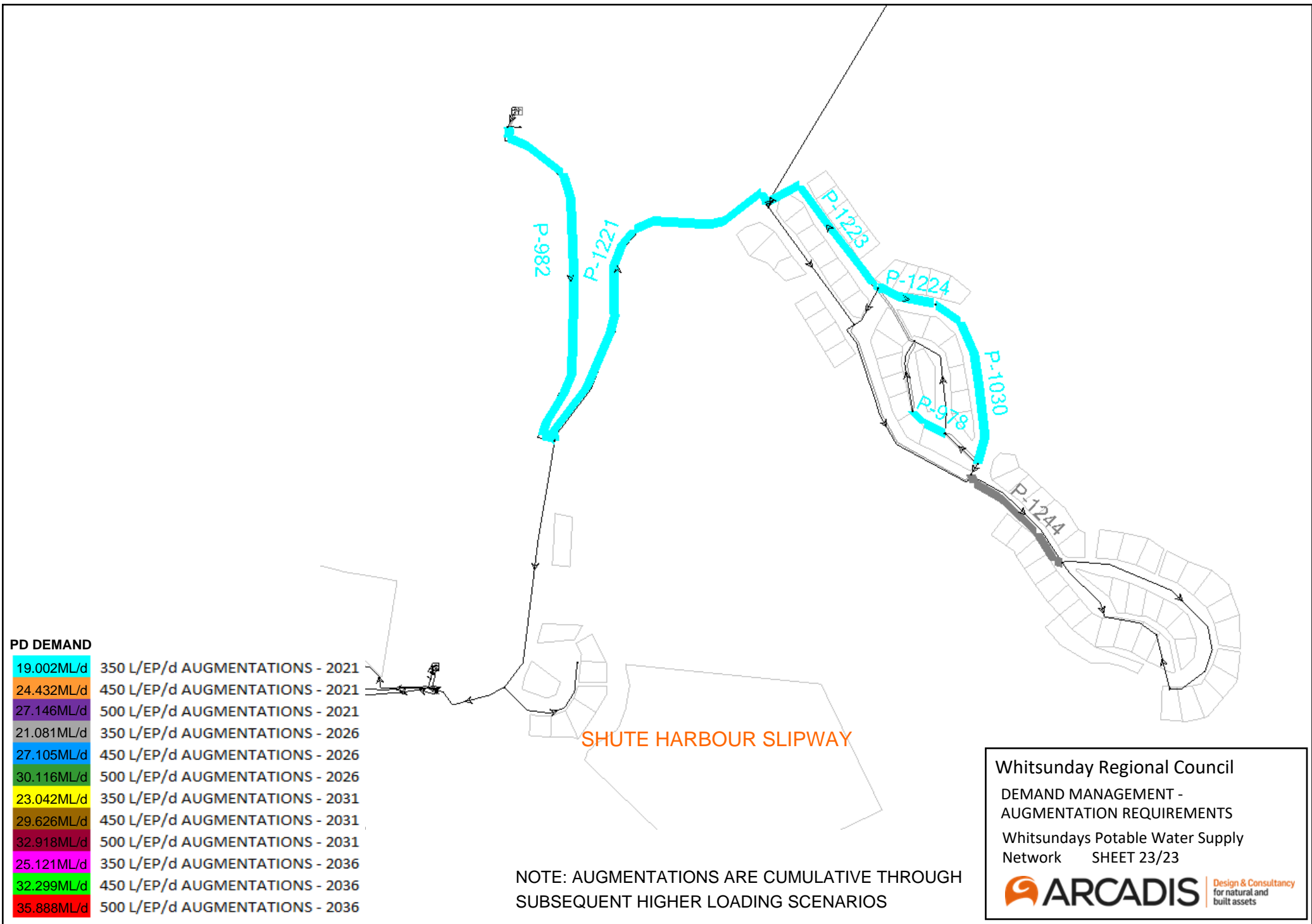
BOOSTED SUPPLY REQUIRED TO SERVICE CURLEW CT ELEVATED AREA

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT - AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply Network SHEET 22/23



Design & Consultancy for natural and built assets



PD DEMAND

| | |
|------------|---------------------------------|
| 19.002ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 24.432ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 27.146ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 21.081ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 27.105ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 30.116ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 23.042ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 29.626ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 32.918ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 25.121ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 32.299ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 35.888ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

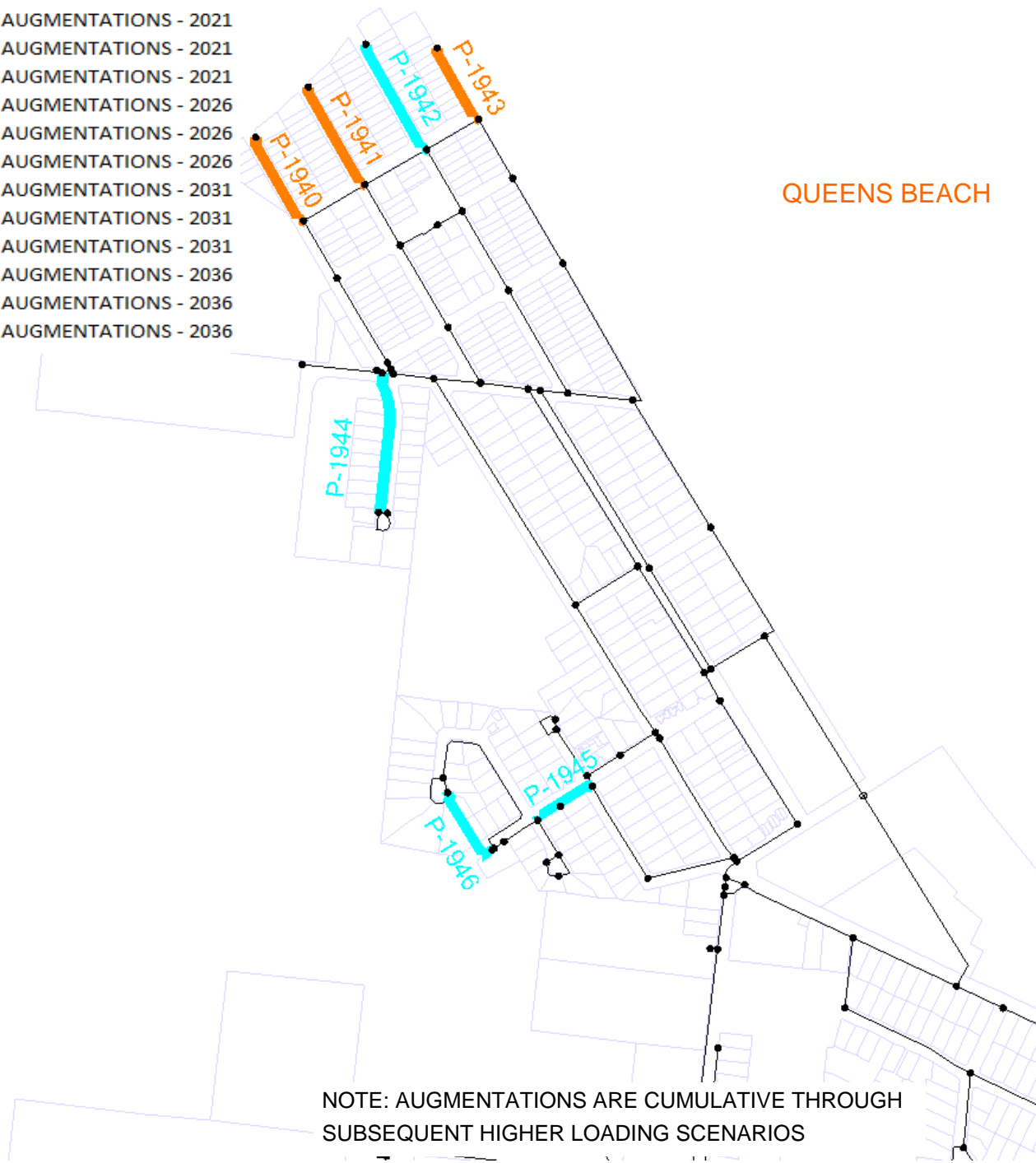
Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Whitsundays Potable Water Supply
 Network SHEET 23/23



Design & Consultancy
for natural and built assets

PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |



QUEENS BEACH

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

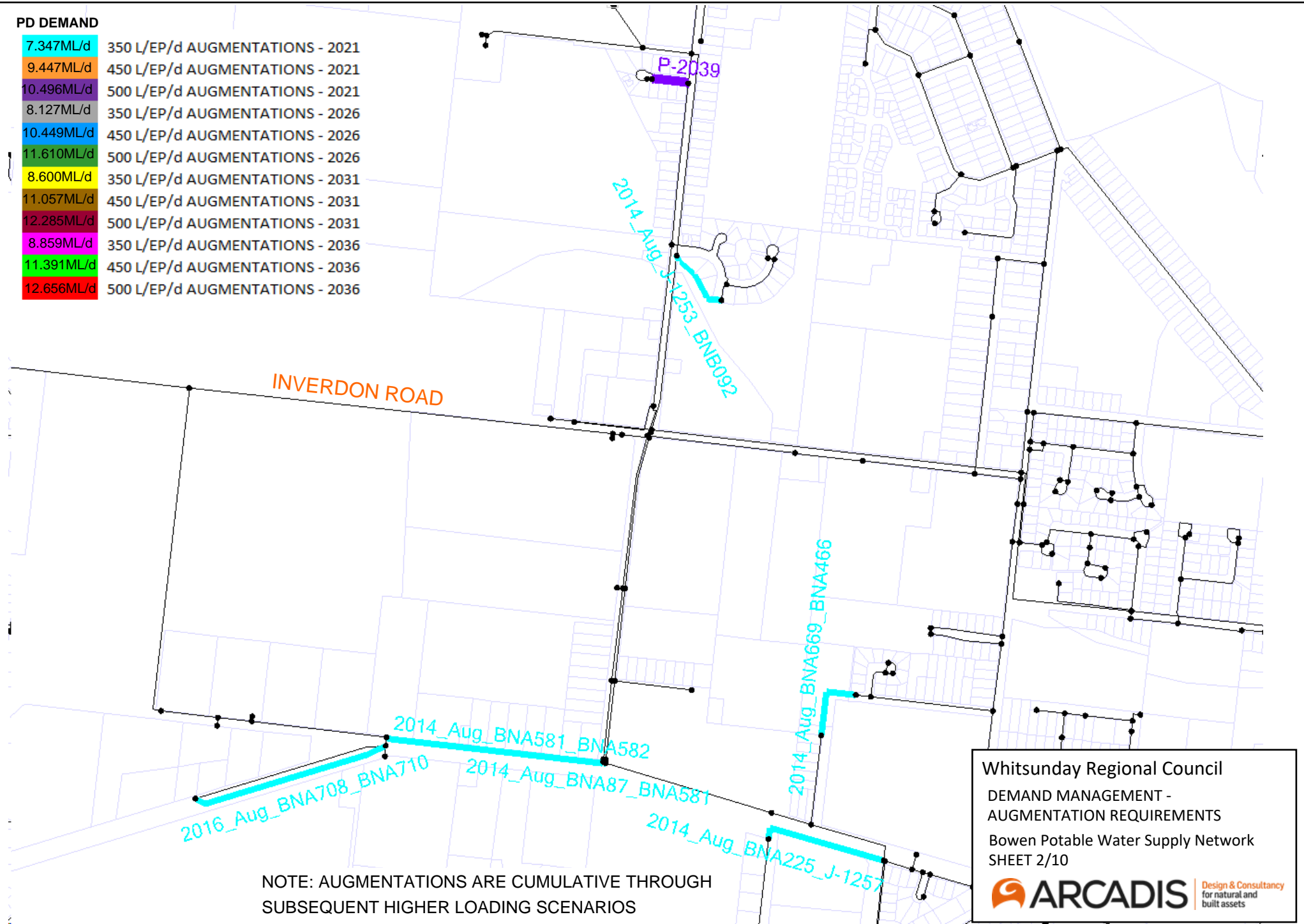
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS

Bowen Potable Water Supply Network
SHEET 1/10



PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

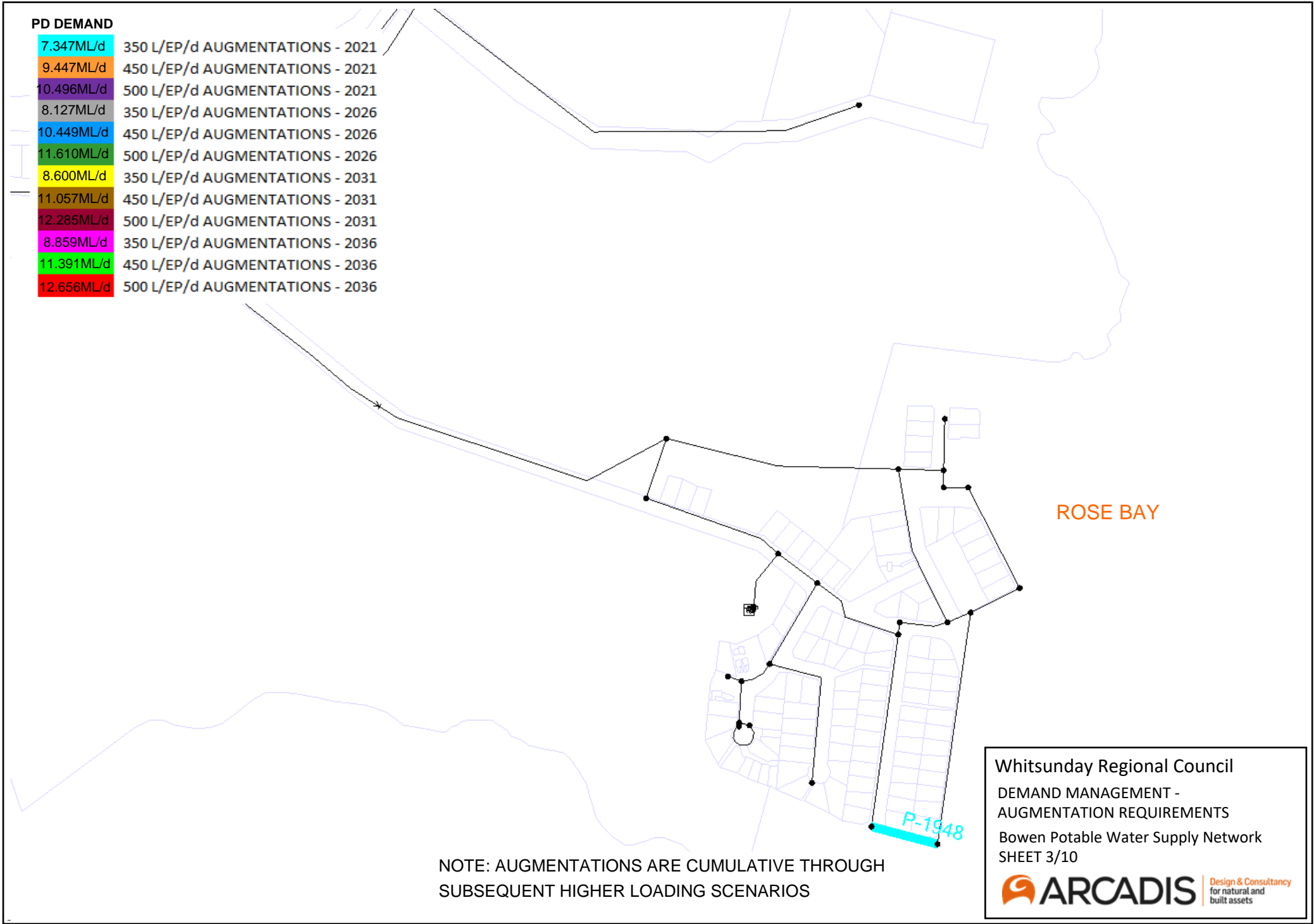


NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 2/10

PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
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| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |



ROSE BAY

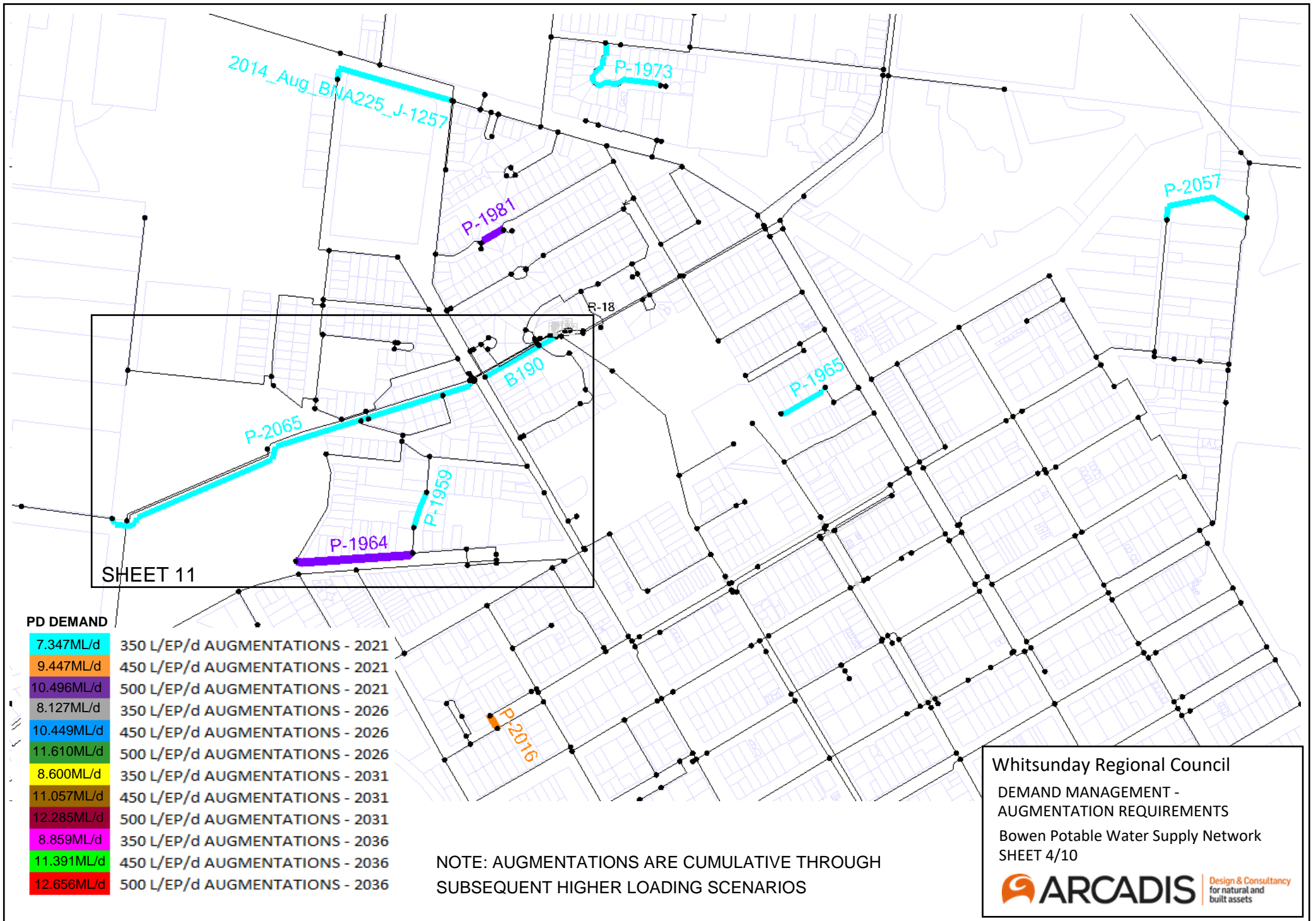
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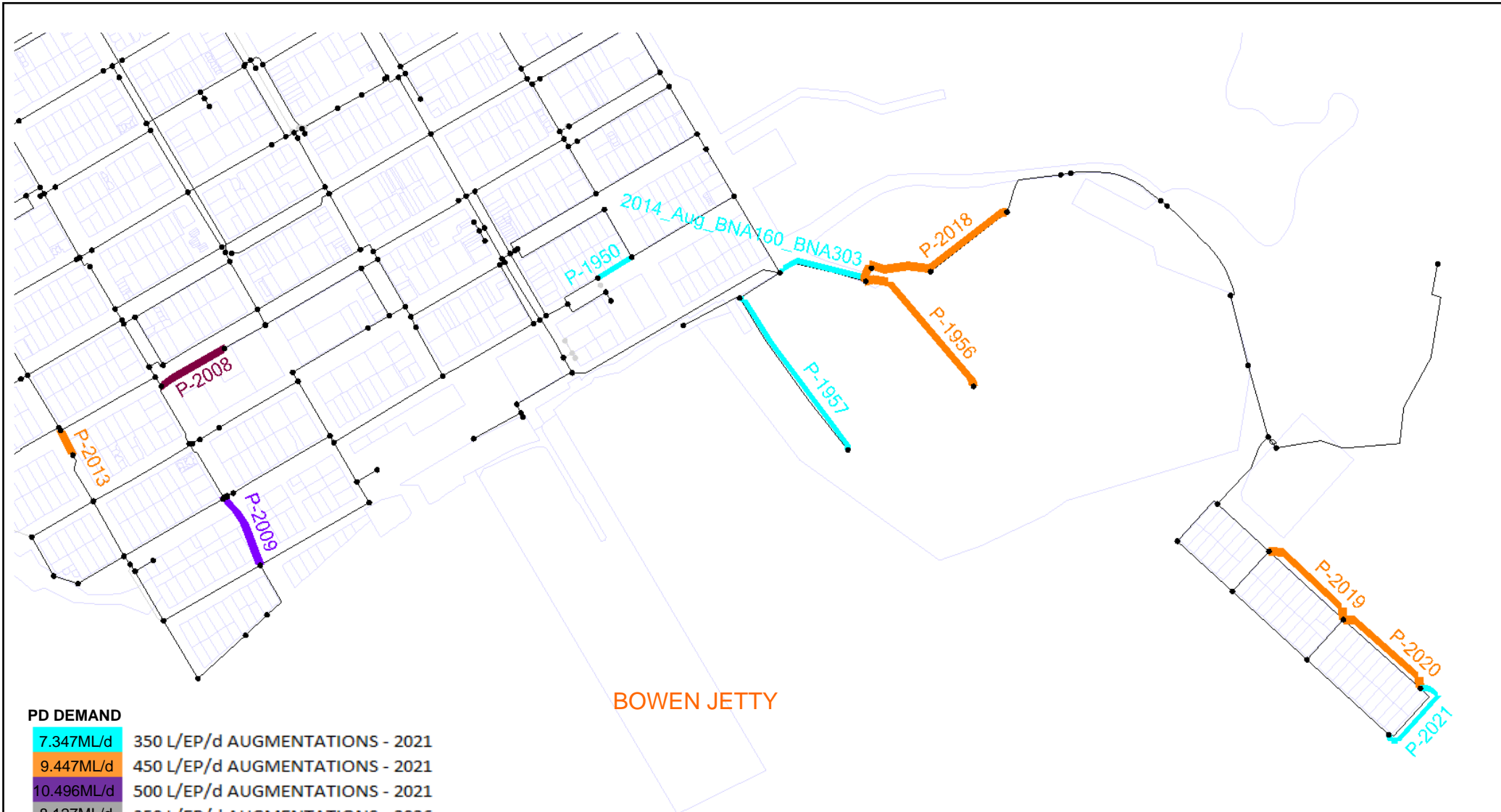
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Bowen Potable Water Supply Network
SHEET 3/10



Design & Consultancy
for natural and
built assets





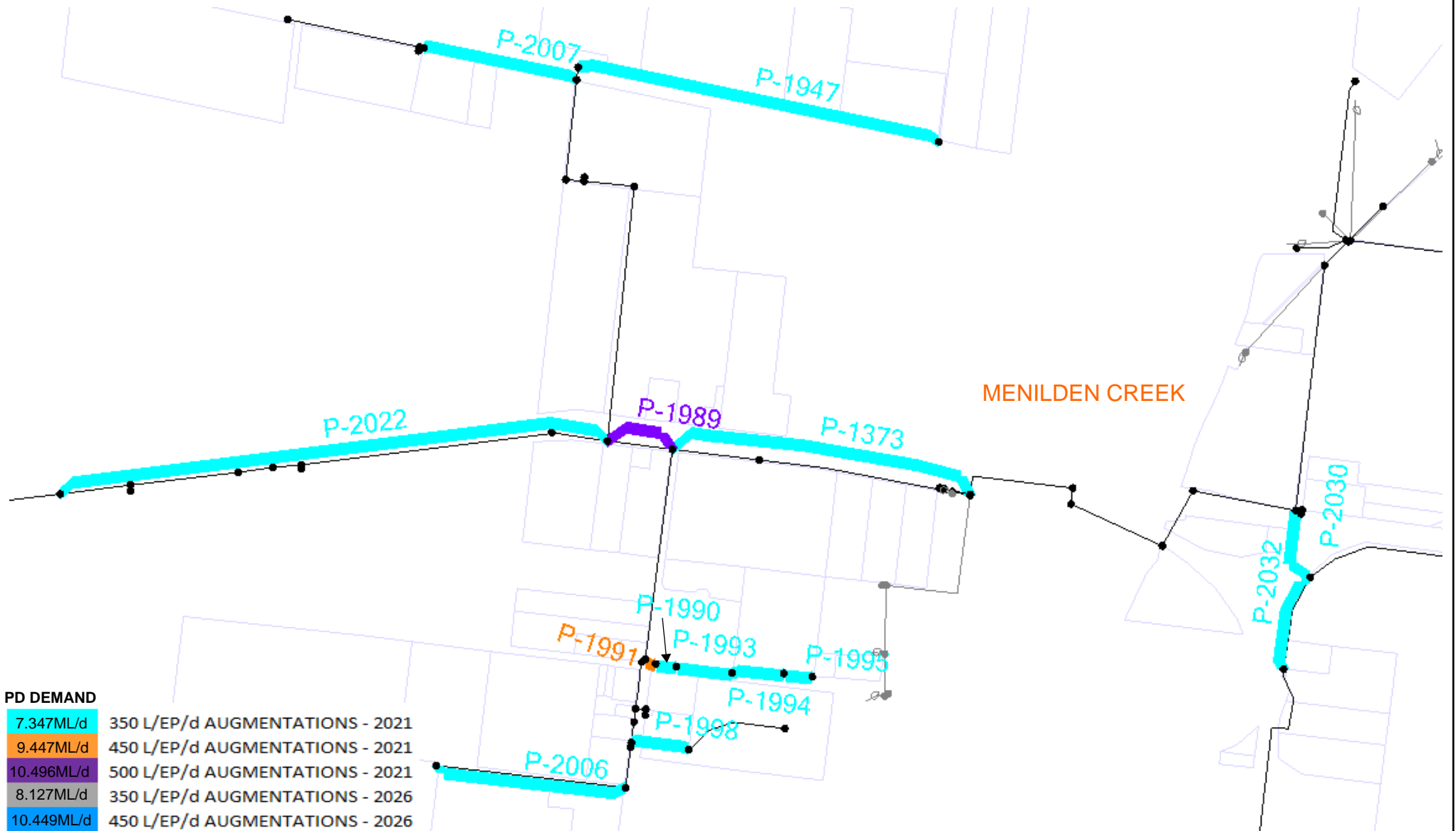
BOWEN JETTY

PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 5/10



MENILDEN CREEK

PD DEMAND

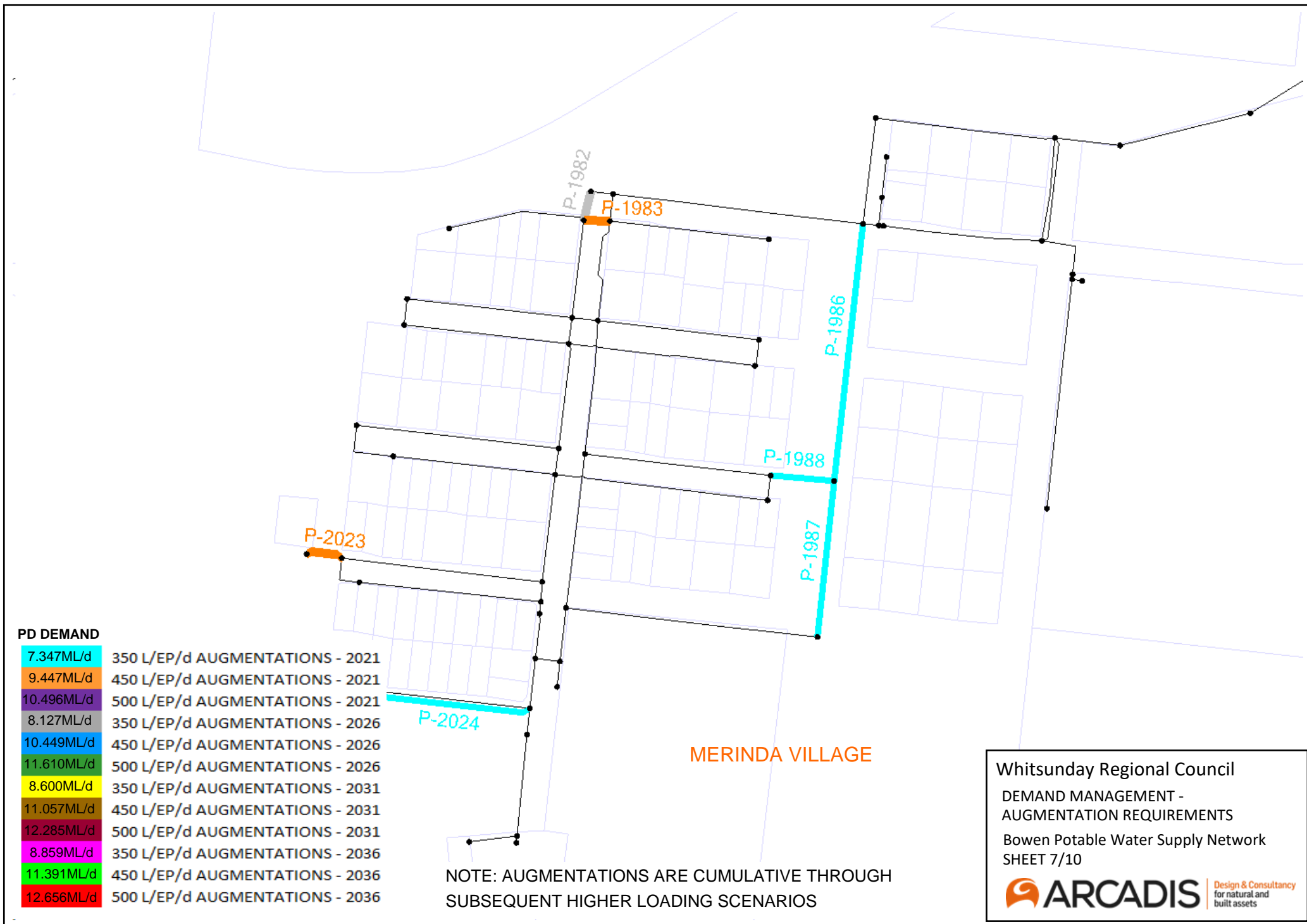
| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 6/10



Design & Consultancy
for natural and built assets



PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
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| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
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| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

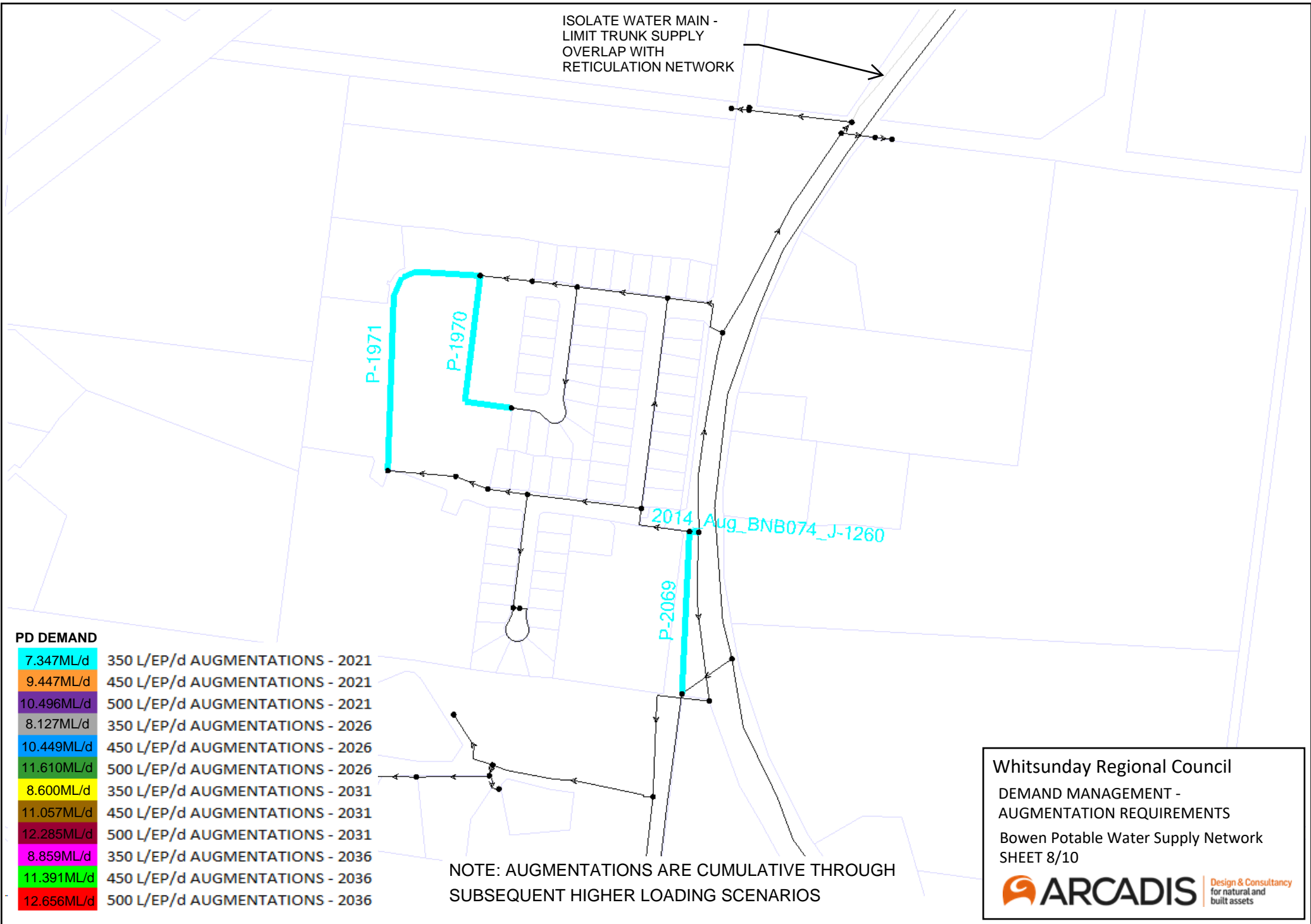
MERINDA VILLAGE

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 7/10



ISOLATE WATER MAIN -
LIMIT TRUNK SUPPLY
OVERLAP WITH
RETICULATION NETWORK



PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
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| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
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| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

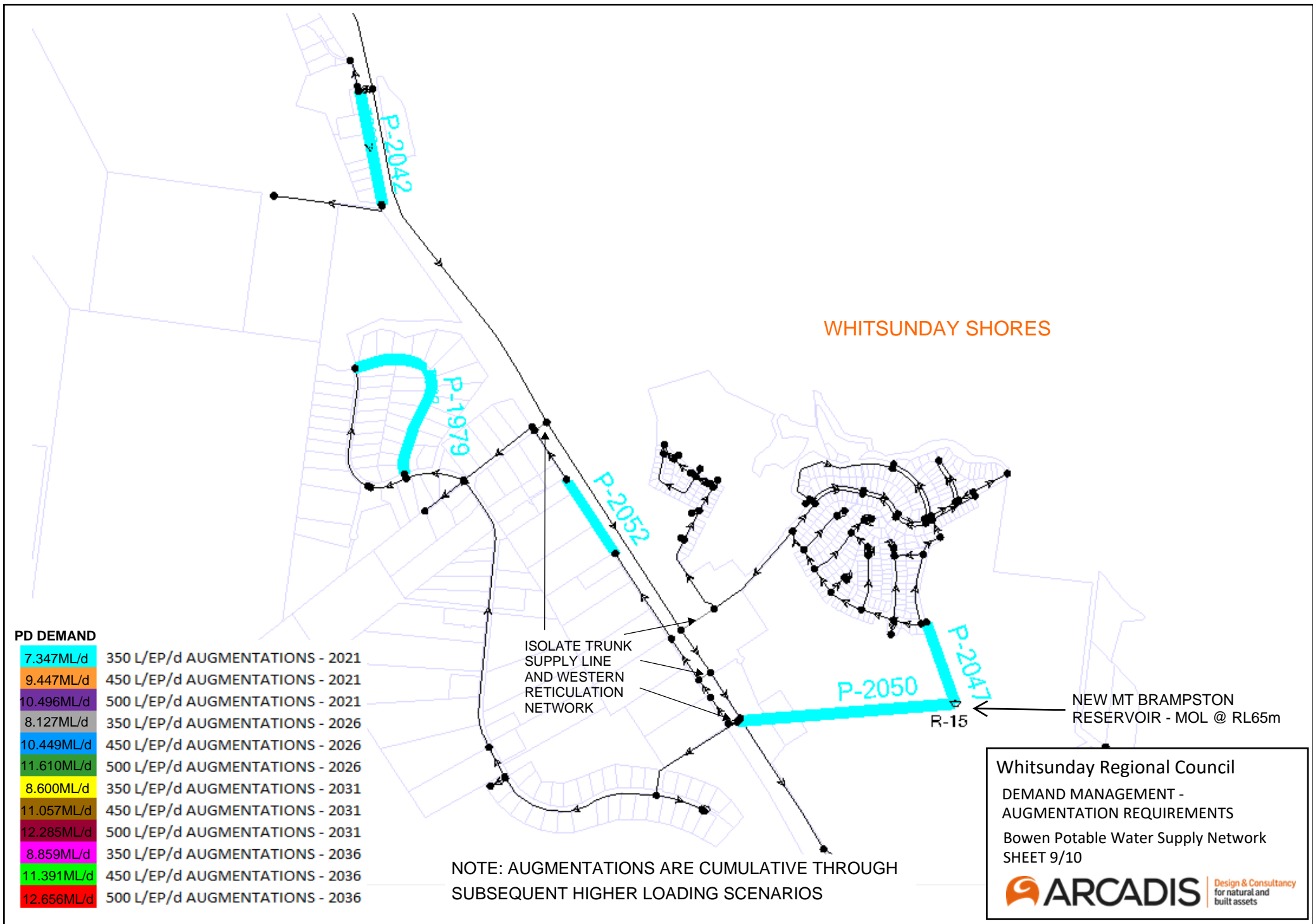
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS

Bowen Potable Water Supply Network
SHEET 8/10





PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 9/10

ARCADIS Design & Consultancy
 for natural and built assets

CONSTRUCT ELEVATED
HERONVALE WATER SUPPLY
TANK - MOL @ RL69m

BAXTER AVENUE

PD DEMAND

| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

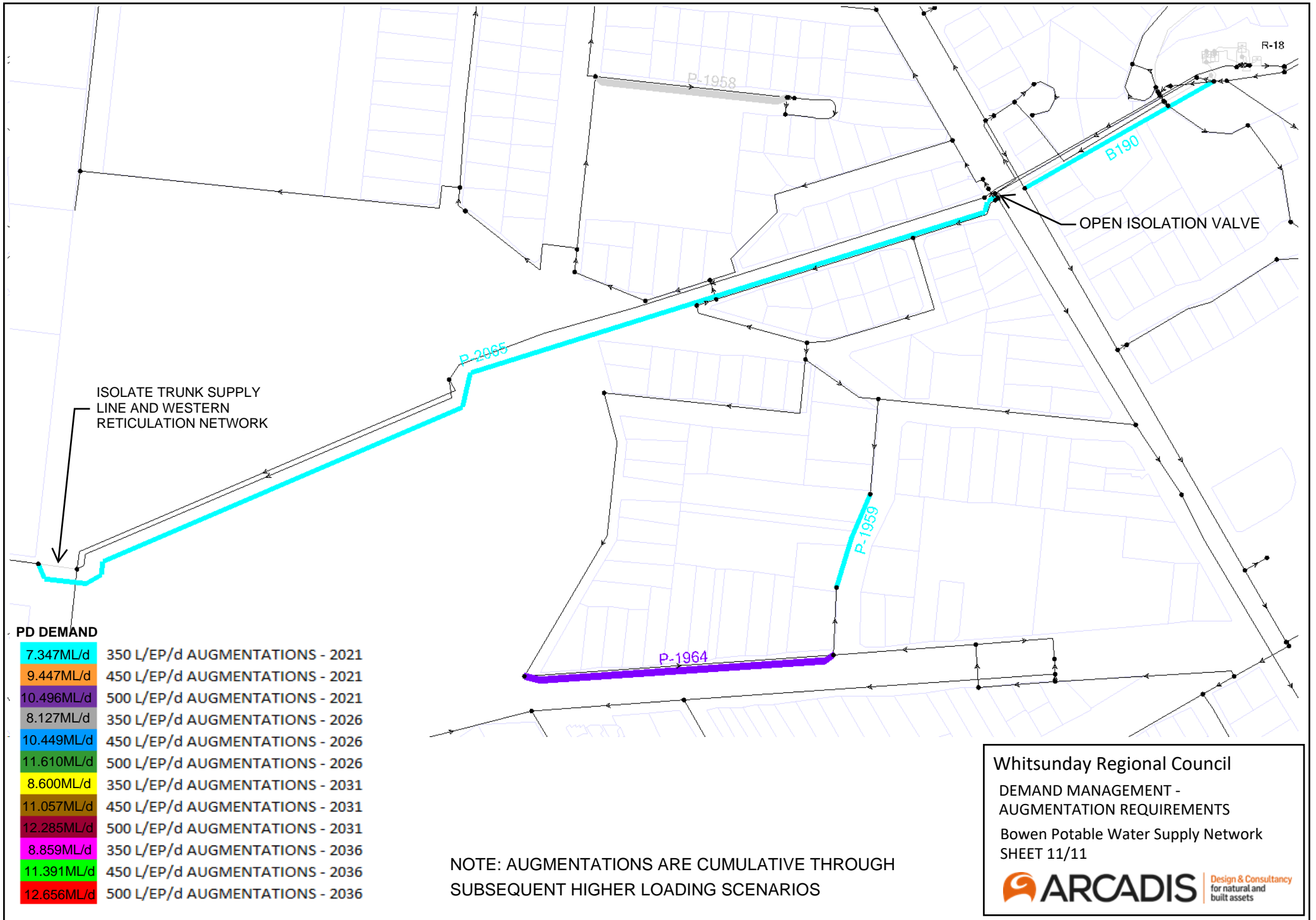
NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council

DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS

Bowen Potable Water Supply Network
SHEET 10/11





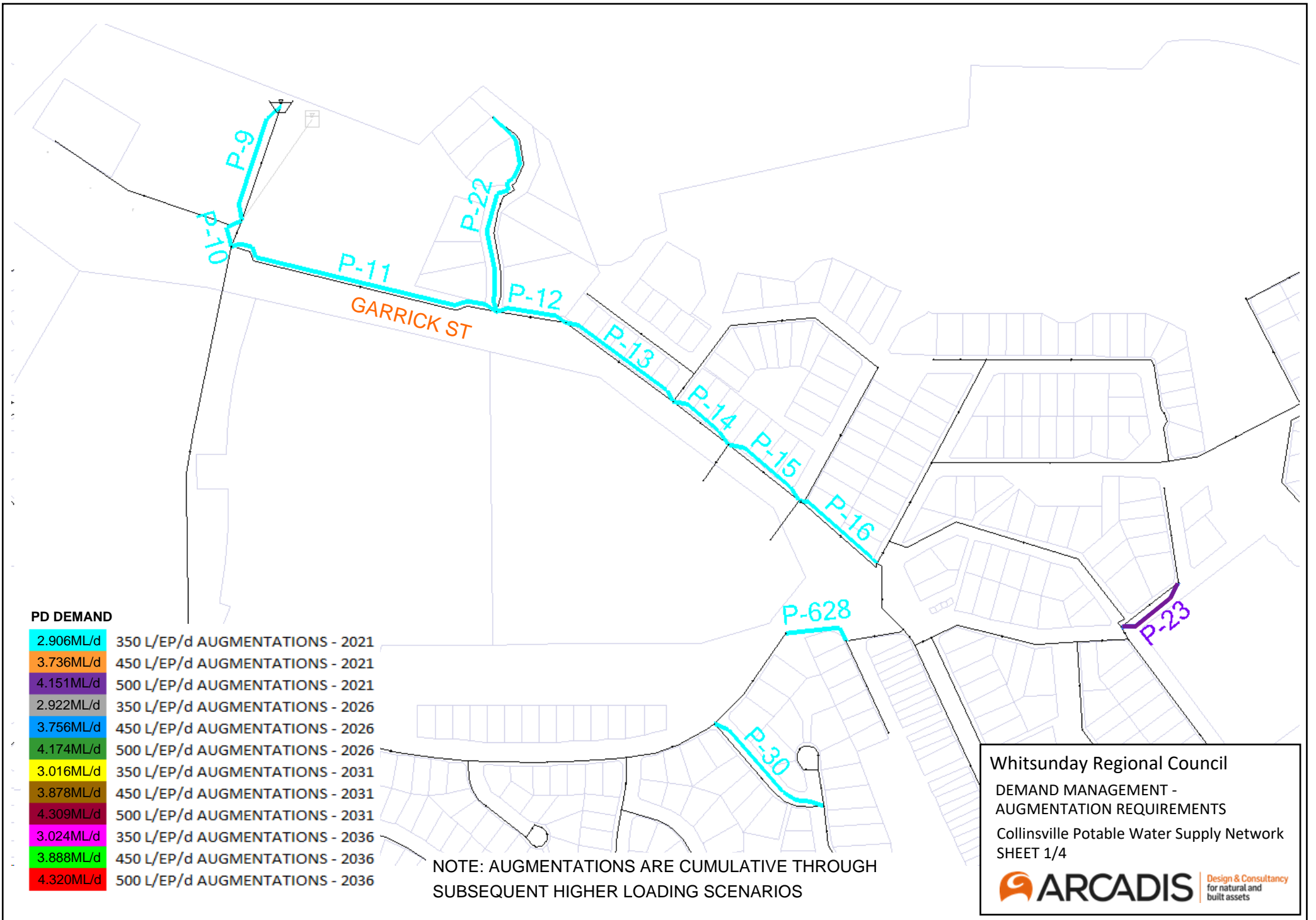
PD DEMAND

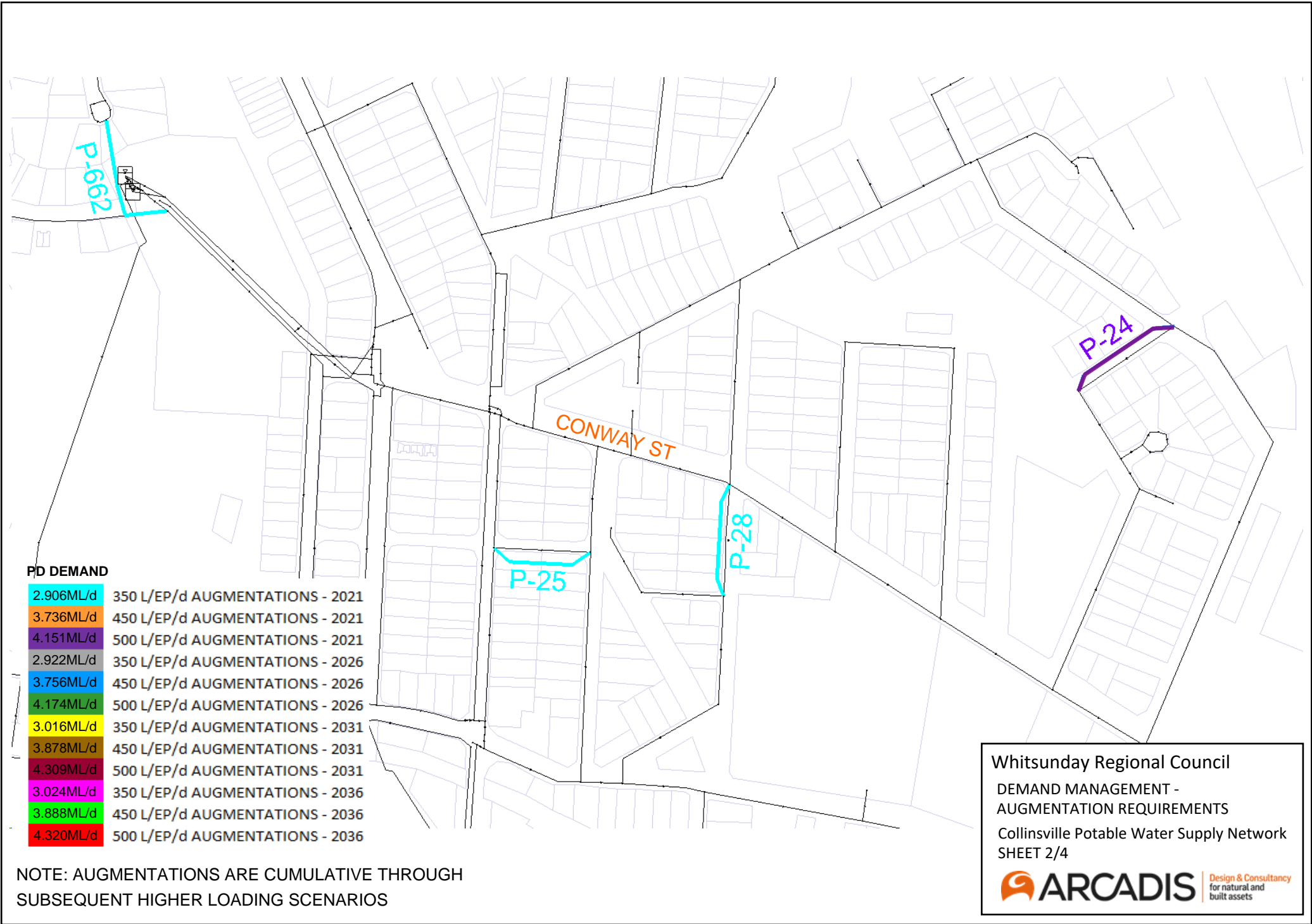
| | |
|------------|---------------------------------|
| 7.347ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 9.447ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 10.496ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 8.127ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 10.449ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 11.610ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 8.600ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 11.057ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 12.285ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 8.859ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 11.391ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 12.656ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Potable Water Supply Network
 SHEET 11/11

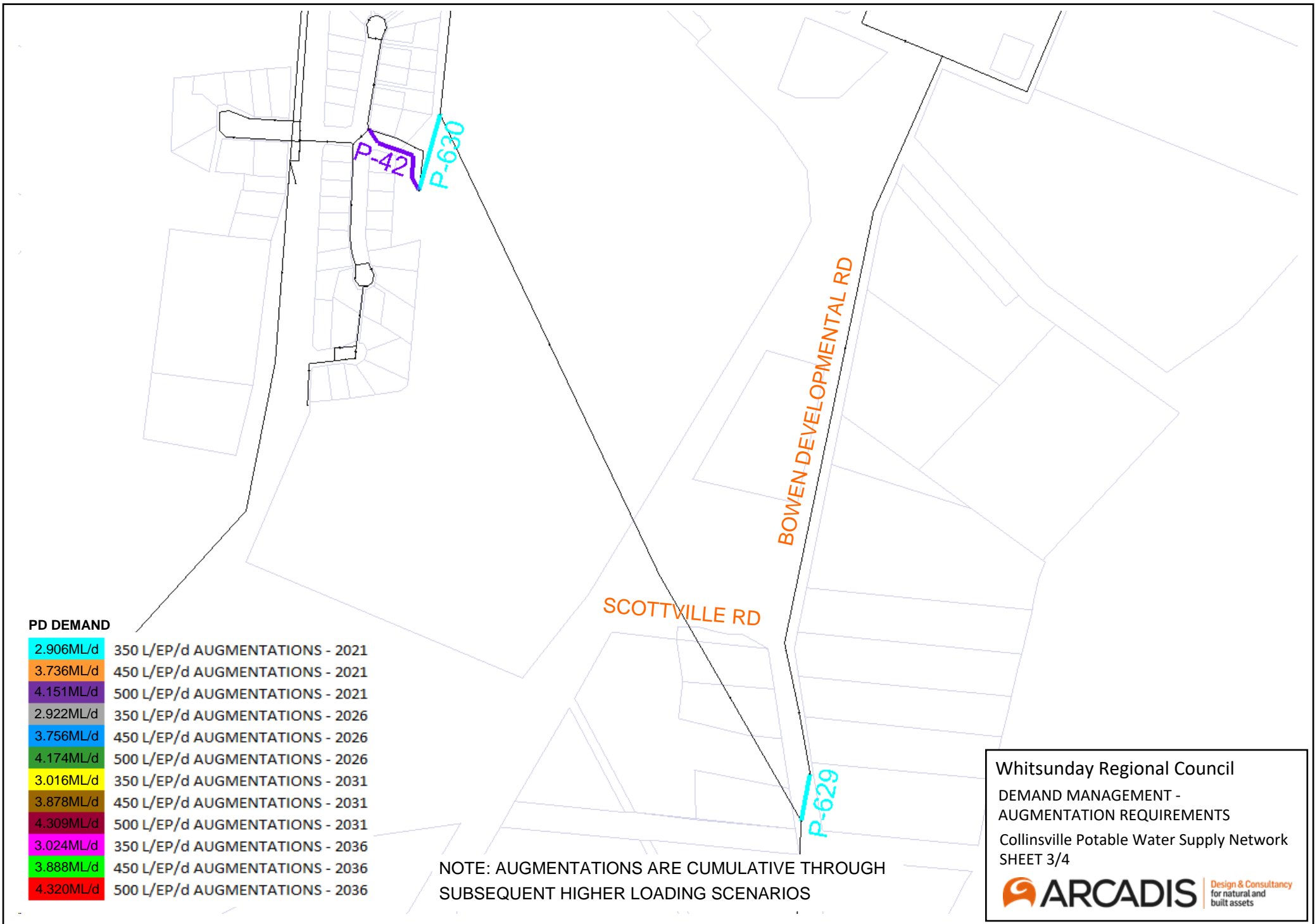






NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT - AUGMENTATION REQUIREMENTS
 Collinsville Potable Water Supply Network
 SHEET 2/4



PD DEMAND

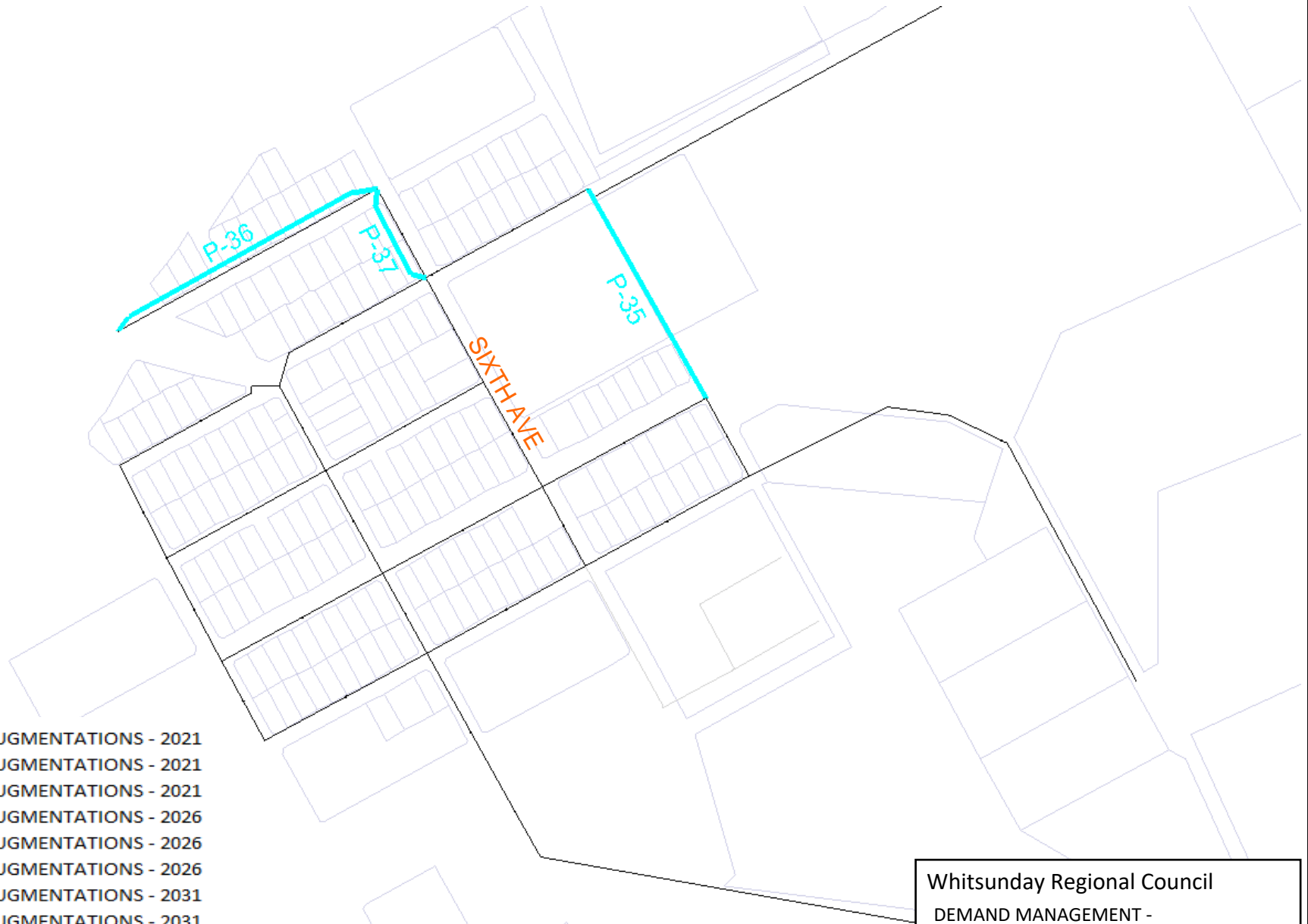
| | |
|-----------|---------------------------------|
| 2.906ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 3.736ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 4.151ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 2.922ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 3.756ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 4.174ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 3.016ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 3.878ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 4.309ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 3.024ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 3.888ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 4.320ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Collinsville Potable Water Supply Network
 SHEET 3/4



Design & Consultancy
for natural and
built assets



PD DEMAND

| | |
|-----------|---------------------------------|
| 2.906ML/d | 350 L/EP/d AUGMENTATIONS - 2021 |
| 3.736ML/d | 450 L/EP/d AUGMENTATIONS - 2021 |
| 4.151ML/d | 500 L/EP/d AUGMENTATIONS - 2021 |
| 2.922ML/d | 350 L/EP/d AUGMENTATIONS - 2026 |
| 3.756ML/d | 450 L/EP/d AUGMENTATIONS - 2026 |
| 4.174ML/d | 500 L/EP/d AUGMENTATIONS - 2026 |
| 3.016ML/d | 350 L/EP/d AUGMENTATIONS - 2031 |
| 3.878ML/d | 450 L/EP/d AUGMENTATIONS - 2031 |
| 4.309ML/d | 500 L/EP/d AUGMENTATIONS - 2031 |
| 3.024ML/d | 350 L/EP/d AUGMENTATIONS - 2036 |
| 3.888ML/d | 450 L/EP/d AUGMENTATIONS - 2036 |
| 4.320ML/d | 500 L/EP/d AUGMENTATIONS - 2036 |

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Collinsville Potable Water Supply Network
 SHEET 4/4



Design & Consultancy
for natural and
built assets

PROJECT: WRC Potable Water Network Modelling Project Engineer: M.C

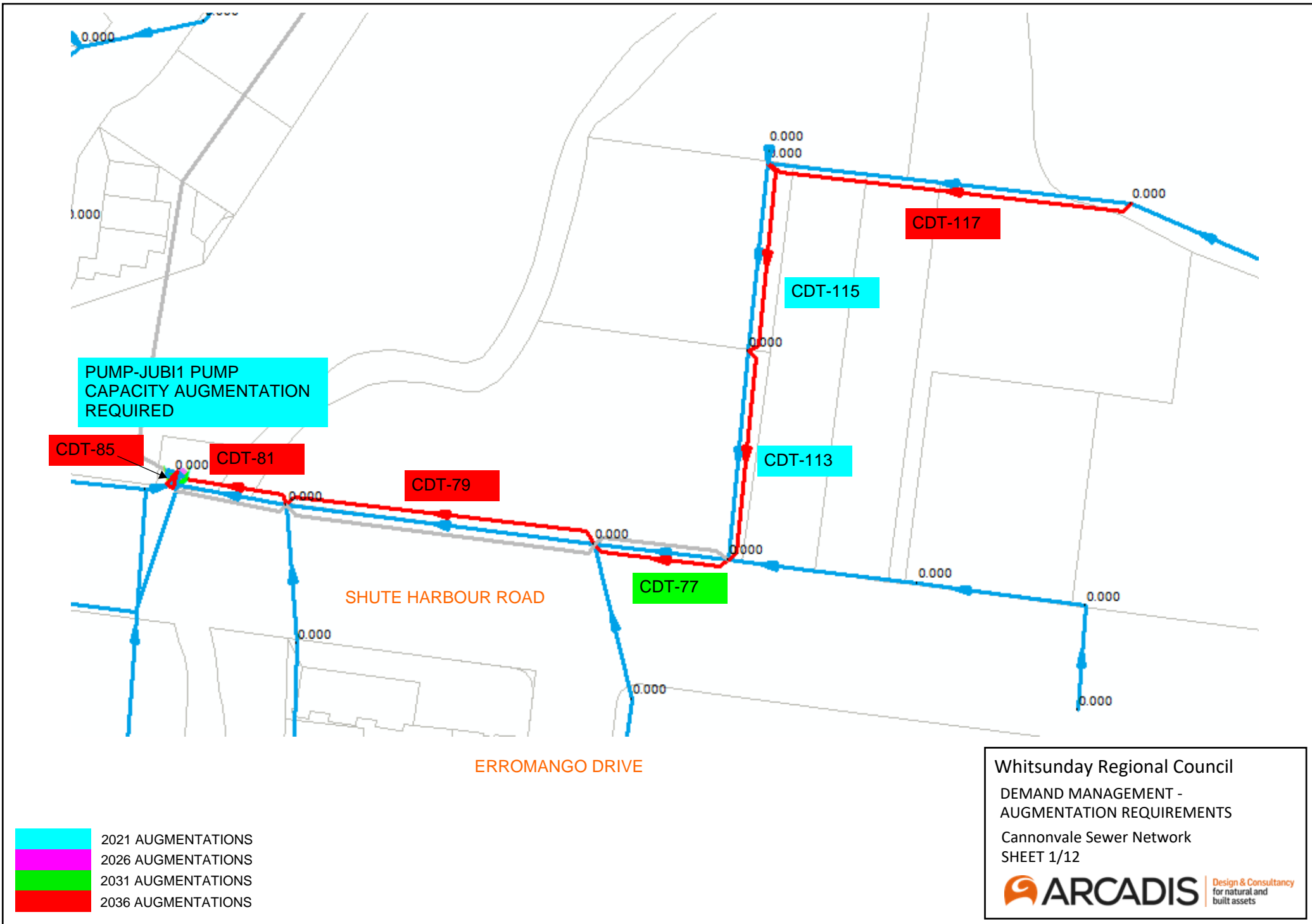
DOCUMENT NUMBER: DATE: 16.12.2019 Software: WaterCad v8i

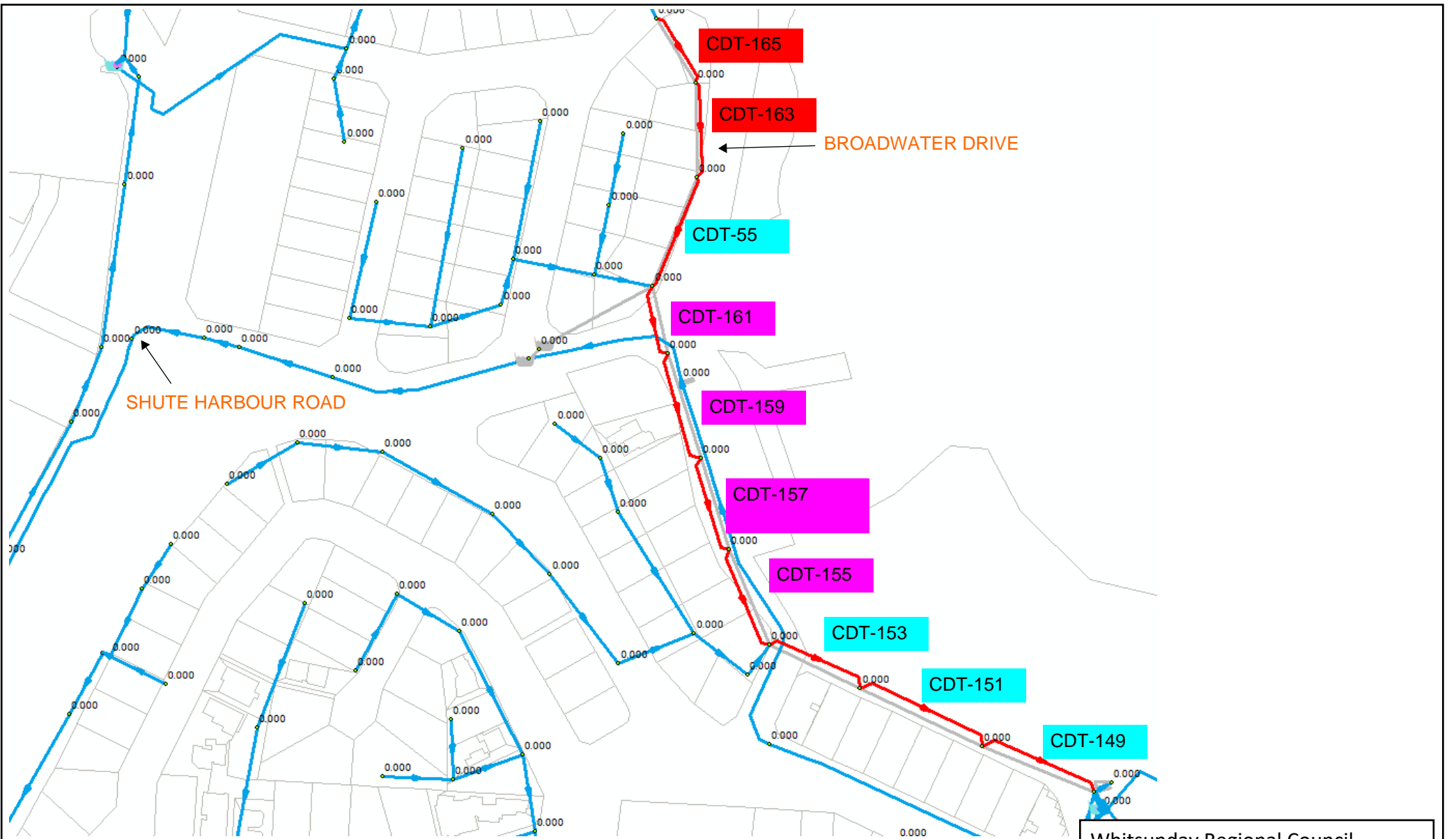
WHITSUNDAYS COLLINSVILLE POTABLE WATER NETWORK DEMAND MANAGEMENT ASSESSMENT - AUGMENTATION REQUIREMENT SUMMARY

| AUGMENTATION ID | LENGTH (m) | EXISTING PIPE SEGMENT | | | | NEW PIPE DN | COST - \$/m | ADJUSTMENT FACTOR FOR SOIL | 10% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW/DUPLICATION | AC PIPE | | | | | | | |
|-----------------|----------------|-----------------------|----------|----------------|----------------|-------------|-------------|----------------------------|-------------------------------|------------------------|-------------------------|----------------|-------|----------------|-------------|----------------------|---|---------------------------------------|---------------------------------------|
| | | START NODE | END NODE | DUPLICATION DN | DUPLICATION DN | | | | | | | YEAR INSTALLED | AC DN | REPLACEMENT DN | COST - \$/m | CAP & GROUT @ \$20/M | TOTAL COST OF DECOMMISSIONING AND REPLACEMENT | COST INCREASE COMPARED TO DULPICATION | % OF INCREASE COMPARED TO DUPLICATION |
| P-9 | 145 | R-1 | CLC258 | 250 | | \$ 384.00 | 1.26 | 10% | 30% | \$ 98,219.52 | | | | | | | | | |
| P-10 | 43 | CLC258 | CLC151 | 250 | | \$ 384.00 | 1.26 | 10% | 30% | \$ 29,127.17 | | | | | | | | | |
| P-11 | 328 | CLC151 | CLC2 | 200 | | \$ 310.00 | 1.26 | 10% | 30% | \$ 179,363.52 | | | | | | | | | |
| P-12 | 84 | CLC2 | CLC263 | 200 | | \$ 310.00 | 1.26 | 10% | 30% | \$ 45,934.56 | | | | | | | | | |
| P-13 | 158 | CLC263 | CLC3 | 200 | | \$ 310.00 | 1.26 | 10% | 30% | \$ 86,400.72 | | | | | | | | | |
| P-14 | 84 | CLC3 | CLC4 | 200 | | \$ 310.00 | 1.26 | 10% | 30% | \$ 45,934.56 | | | | | | | | | |
| P-15 | 108 | CLC4 | CLC5 | 200 | | \$ 310.00 | 1.26 | 10% | 30% | \$ 59,058.72 | | | | | | | | | |
| P-16 | 118 | CLC5 | CLC6 | 200 | | \$ 310.00 | 1.26 | 10% | 30% | \$ 64,527.12 | | | | | | | | | |
| P-22 | 257 | CLC2 | CLC9 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 92,936.34 | | | | | | | | | |
| P-23 | 89 | CLC20 | CLC24 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 32,184.18 | | 100 | 150 | \$ 262.00 | \$ 5,140.00 | \$ 132,401.26 | \$ 39,464.92 | 30% | |
| P-24 | 135 | CLC72 | CLC73 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 48,818.70 | | 100 | 150 | \$ 262.00 | \$ 2,700.00 | \$ 69,549.30 | \$ 20,730.60 | 30% | |
| P-25 | 118 | CLC97 | CLC98 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 42,671.16 | | 100 | 150 | \$ 262.00 | \$ 2,360.00 | \$ 60,791.24 | \$ 18,120.08 | 30% | |
| P-28 | 128 | CLC67 | CLC212 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 46,287.36 | | 100 | 150 | \$ 262.00 | \$ 2,560.00 | \$ 65,943.04 | \$ 19,655.68 | 30% | |
| P-30 | 165 | CLC164 | CLC166 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 59,667.30 | | 100 | 150 | \$ 262.00 | \$ 3,300.00 | \$ 85,004.70 | \$ 25,337.40 | 30% | |
| P-35 | 287 | CLC195 | CLC179 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 132,642.22 | | | | | | | | | |
| P-36 | 357 | CLC182 | CLC183 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 129,098.34 | | 100 | 150 | 262 | \$ 7,140.00 | \$ 183,919.26 | \$ 54,820.92 | 30% | |
| P-37 | 131 | CLC181 | CLC182 | 100 | | \$ 205.00 | 1.26 | 10% | 30% | \$ 47,372.22 | | 100 | 150 | 262 | \$ 2,620.00 | \$ 67,488.58 | \$ 20,116.36 | 30% | |
| P-42 | 105 | CLC120 | CLC121 | | 150 | \$ 205.00 | 1.26 | 10% | 30% | \$ 37,970.10 | | | 150 | 262 | \$ 2,100.00 | \$ 54,093.90 | \$ 16,123.80 | 30% | |
| P-628 | 80 | CLC165 | CLC54 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 36,973.44 | | | | | | | | | |
| P-629 | 55 | CLC205 | CLC267 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 25,419.24 | | | | | | | | | |
| P-630 | 91 | CLC172 | CLC121 | | 150 | \$ 262.00 | 1.26 | 10% | 30% | \$ 42,057.29 | | | | | | | | | |
| P-662 | 155 | CLC170 | CLC147 | | 100 | \$ 205.00 | 1.26 | 10% | 30% | \$ 56,051.10 | | | | | | | | | |
| | 3221.00 | | | | | | | TOTAL | | \$ 1,438,714.87 | | | | | | \$ 765,042.30 | \$ 228,036.60 | 29.81% | |

APPENDIX G


INITIAL SEWER NETWORK AUGMENTATION OUTPUTS PRE- WORKSHOP



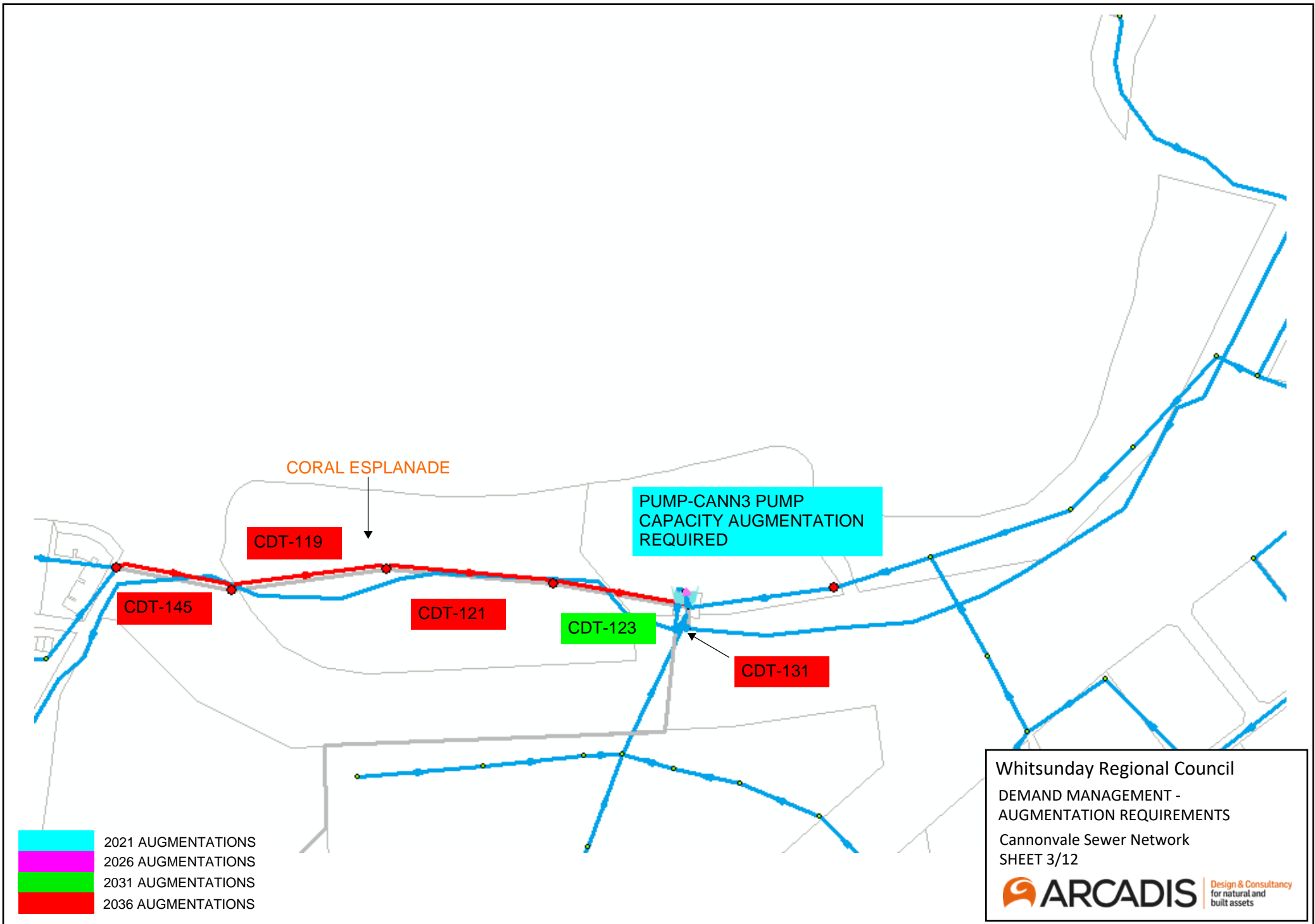


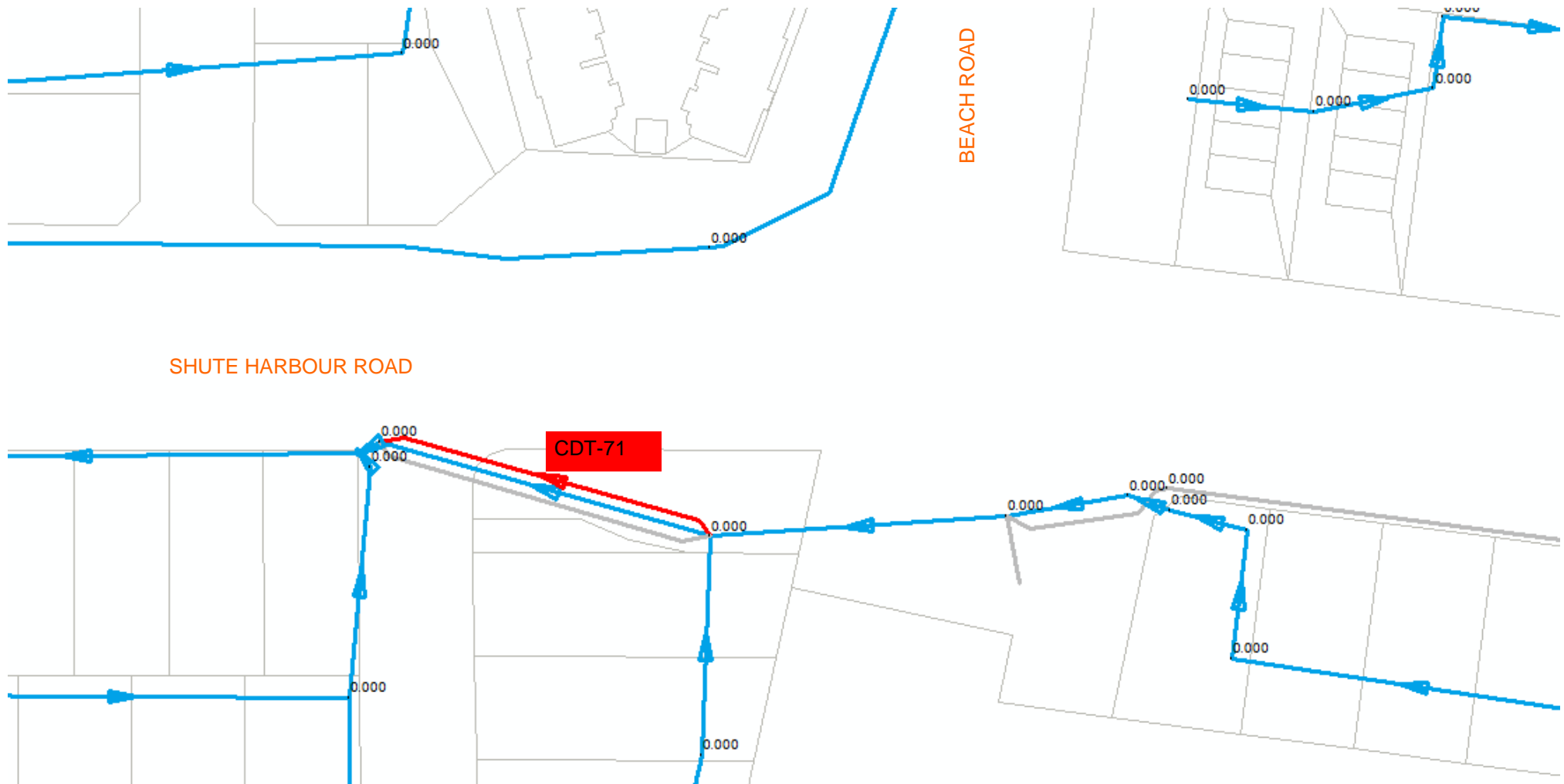
- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 2/12




Design & Consultancy
for natural and
built assets

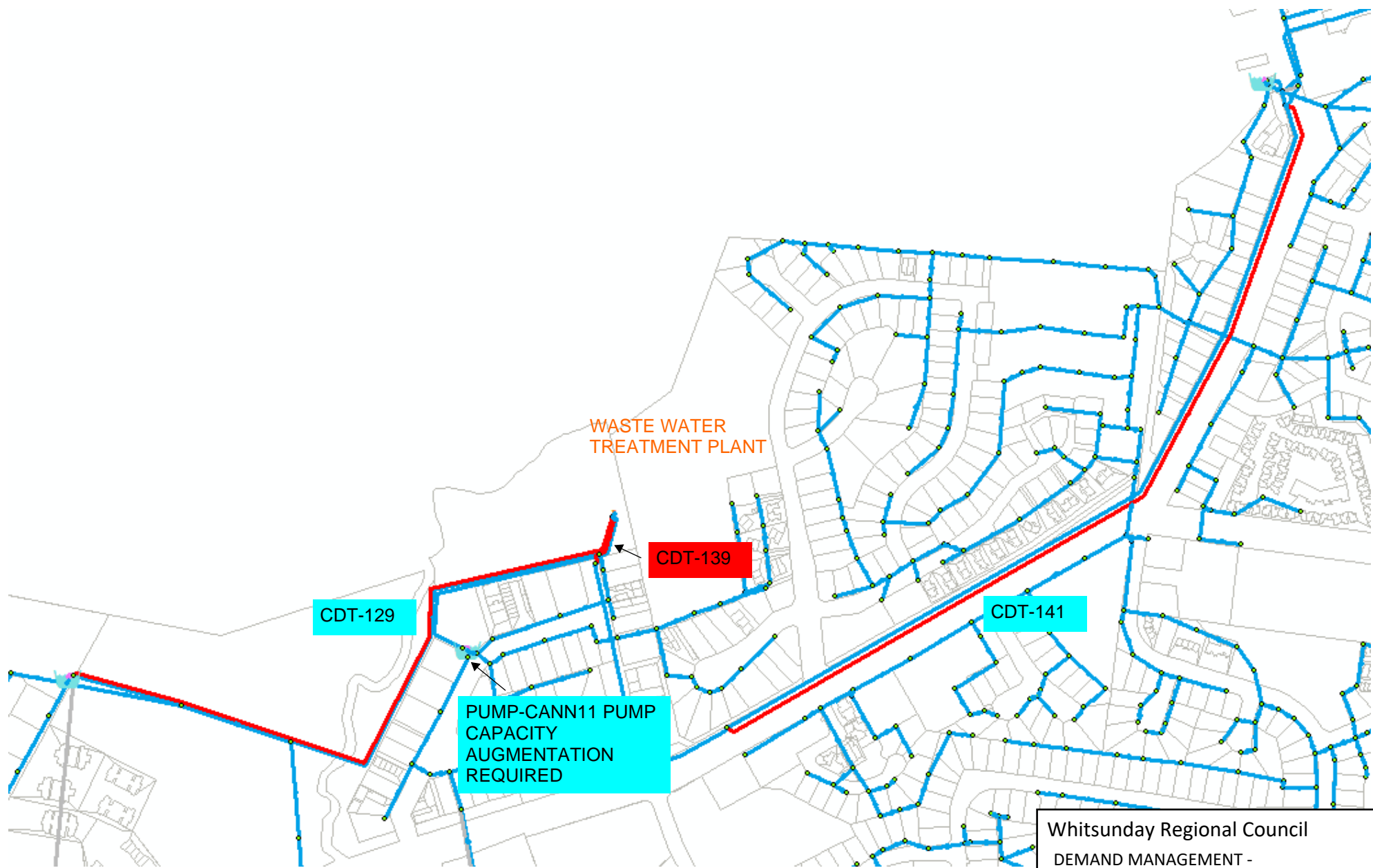




- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 4/12





- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 5/12



Design & Consultancy
for natural and
built assets

MANDALAY ROAD

CDT-135

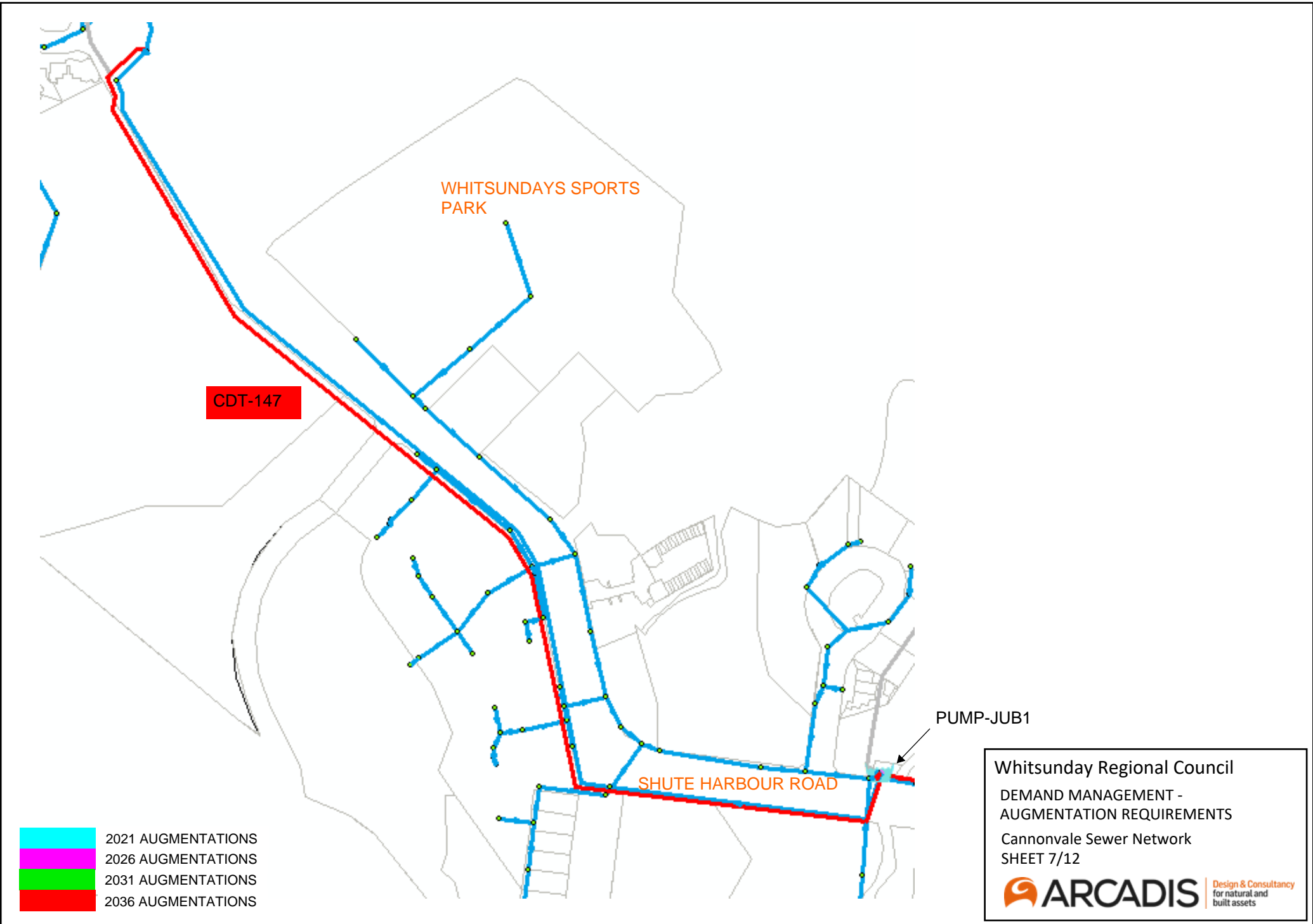
SHUTE HARBOUR ROAD

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 20231 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Cannonvale Sewer Network
SHEET 6/12



Design & Consultancy
for natural and
built assets



CDT-147

WHITSUNDAYS SPORTS PARK

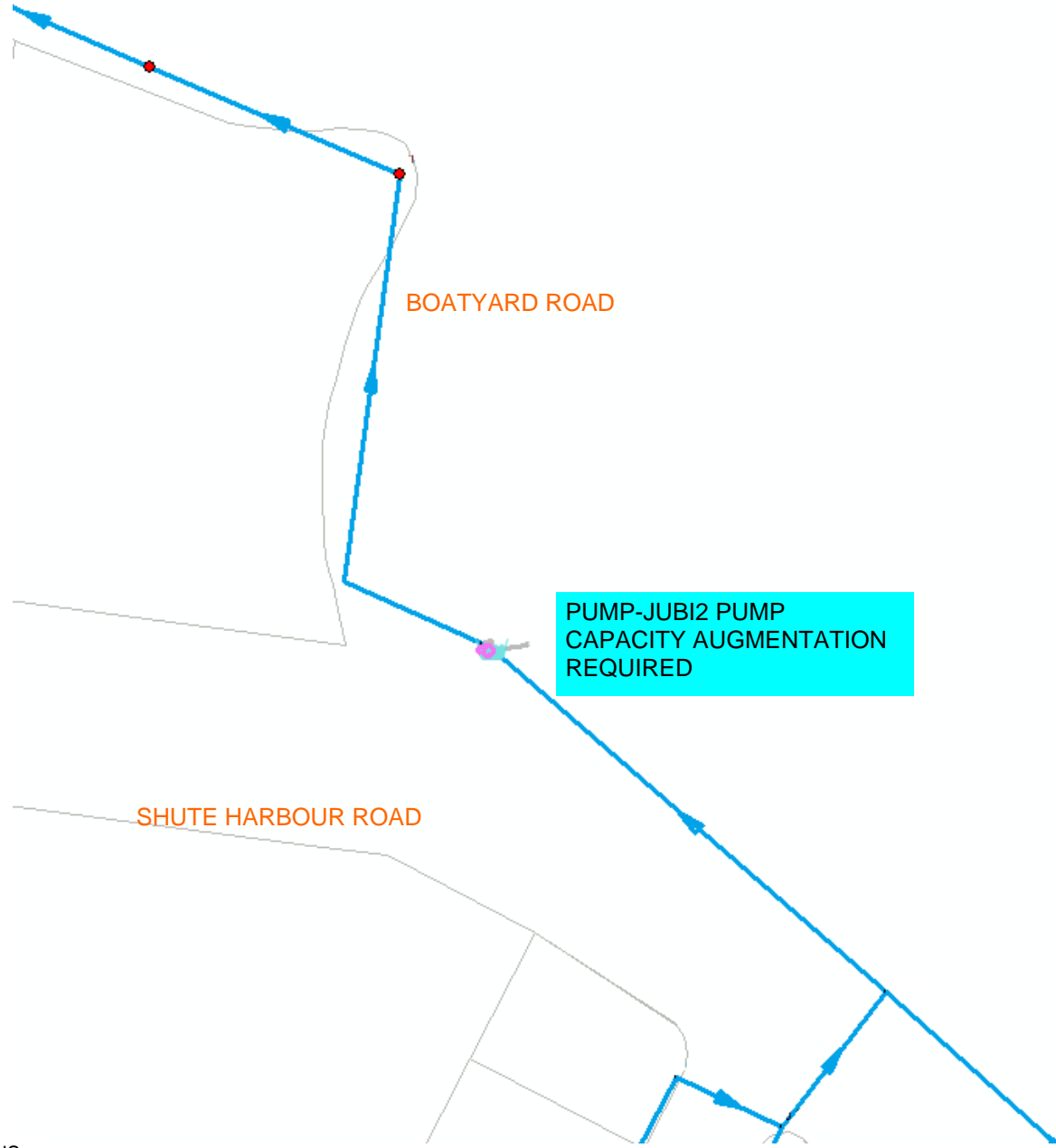
SHUTE HARBOUR ROAD

PUMP-JUB1

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS


Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 7/12

ARCADIS Design & Consultancy
for natural and
built assets

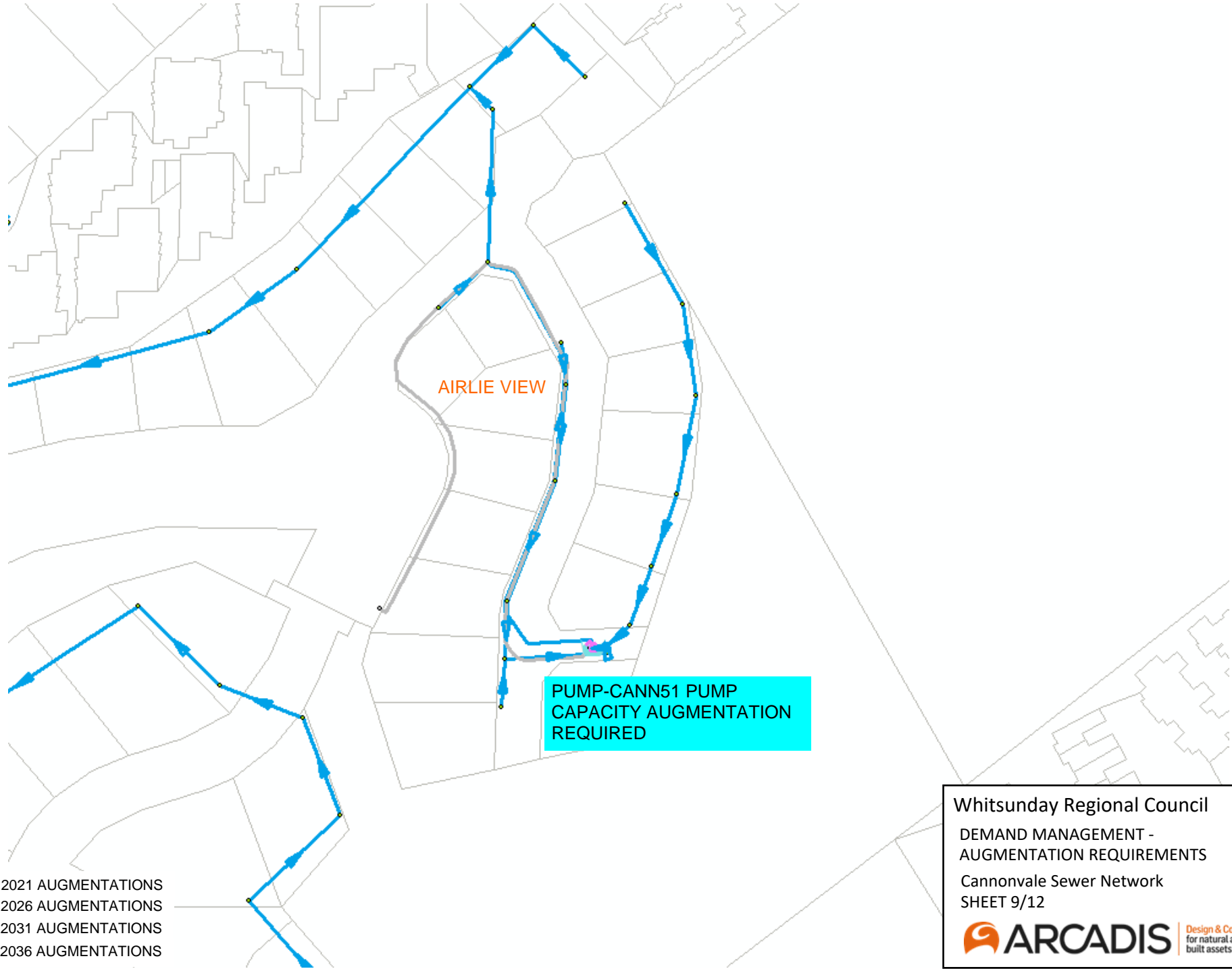


- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Cannonvale Sewer Network
SHEET 8/12



Design & Consultancy
for natural and
built assets

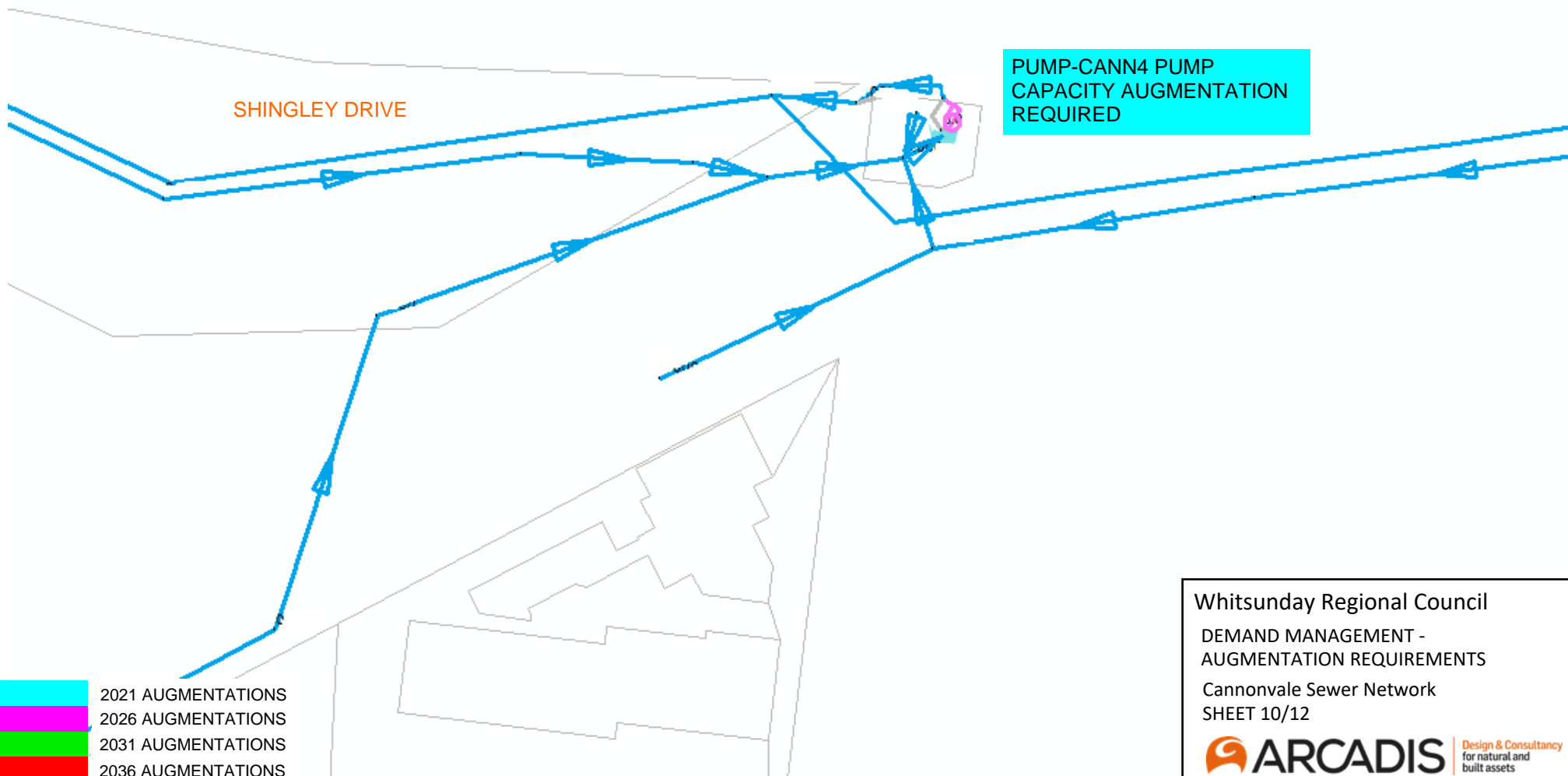


- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Cannonvale Sewer Network
SHEET 9/12



Design & Consultancy
for natural and
built assets



SHINGLEY DRIVE

PUMP-CANN4 PUMP
CAPACITY AUGMENTATION
REQUIRED

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Cannonvale Sewer Network
 SHEET 10/12



Design & Consultancy
for natural and
built assets

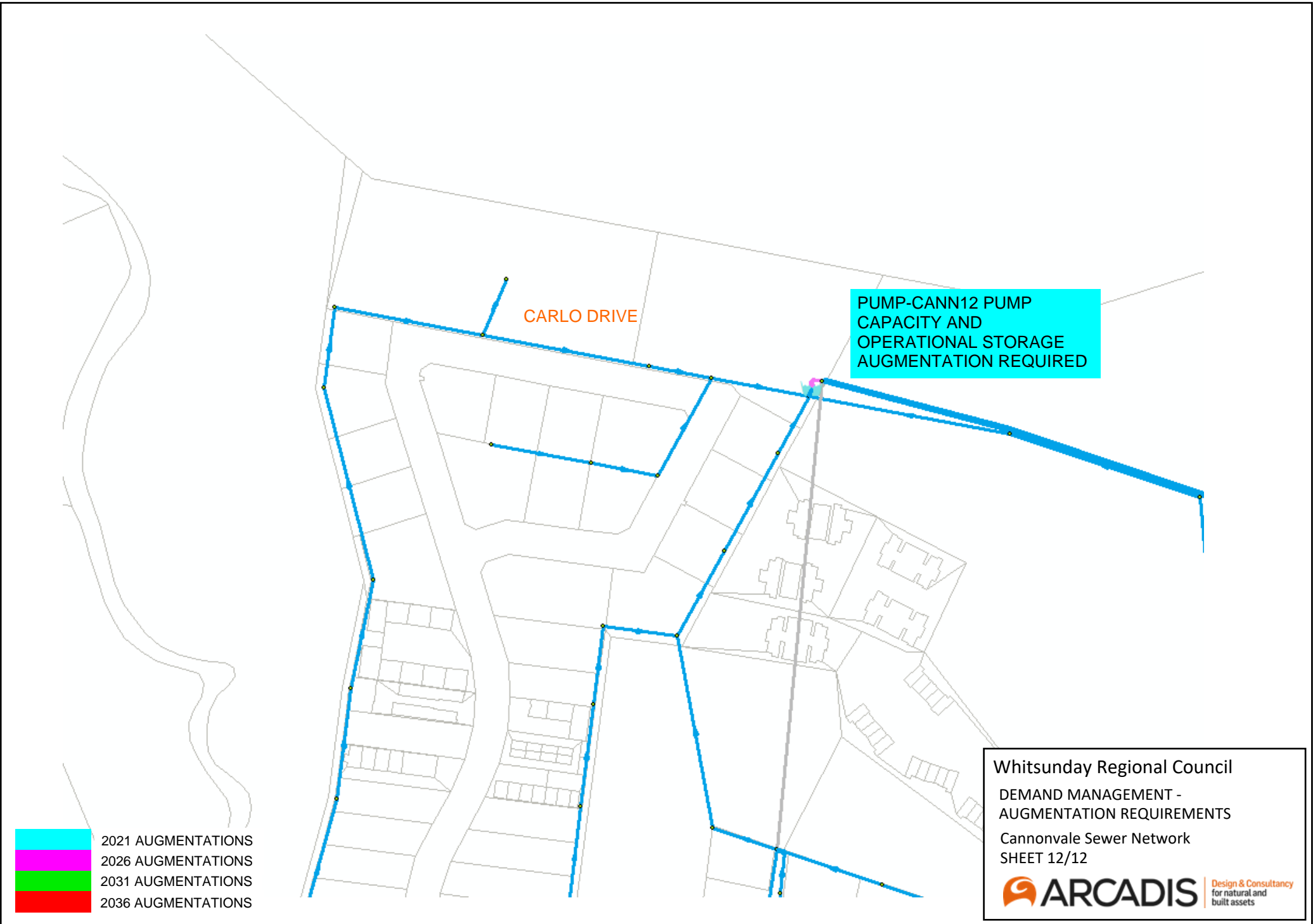
PUMP-CANN1 PUMP
CAPACITY AUGMENTATION
REQUIRED

SHUTE HARBOUR ROAD

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Cannonvale Sewer Network
SHEET 11/12





- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Cannonvale Sewer Network
SHEET 12/12



Design & Consultancy
for natural and
built assets

PROJECT: Whitsundays Sewer Network Modelling
 DOCUMENT N006-10027538-01
 DATE:

Project Engineer: M.C
 Software: InfoSWMM 5A

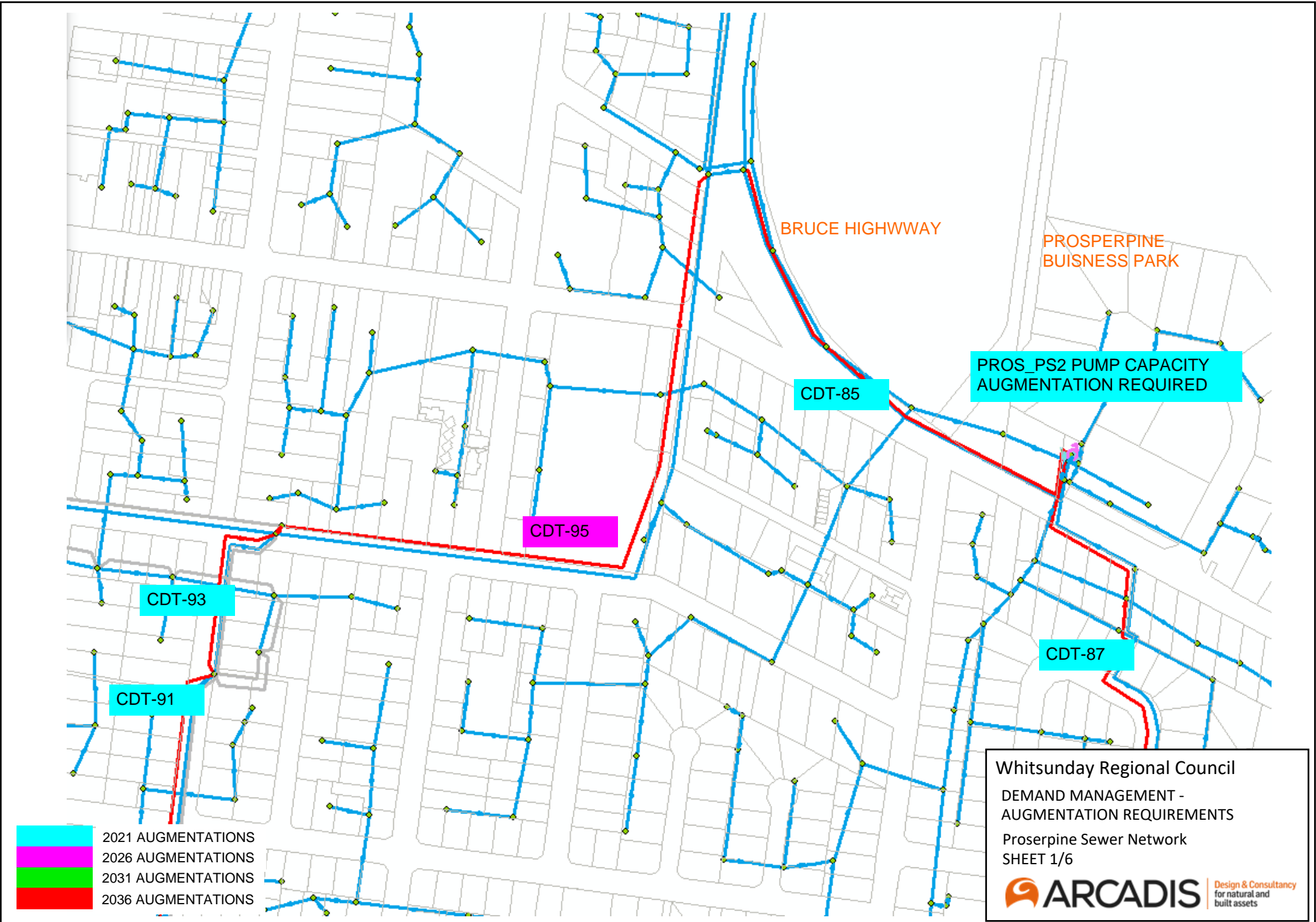
WHITSUNDAYS CANNONVALE SEWER NETWORK
 DEMAND MANAGEMENT ASSESSMENT - SEWER PUMP STATION ASSESSMENT SUMMARY



| Pump ID (Chgr) | PUMP | Startup Depth (m) | Shutoff Depth (m) | Max Depth | Area | CURRENT | |
|----------------|------|-------------------|-------------------|-----------|------|-------------|-------------|
| | | | | | | Op. Storage | Em. Storage |
| PUMP-CANN1.2 | | 3.9 | 4.1 | 6.6 | 10 | 15.00 | 17.00 |
| PUMP-CANN1.1 | | 0.87 | 0.27 | 4.37 | 4.34 | 2.50 | 13.83 |
| PUMP-CANN12.1 | | 1.7 | 1.2 | 5.95 | 3.6 | 1.80 | 14.22 |
| PUMP-CANN14.2 | | 0.86 | 0.36 | 3.66 | 2.46 | 1.23 | 6.15 |
| PUMP-CANN15.2 | | 0.47 | 0.17 | 5.8 | 1.43 | 0.43 | 7.19 |
| PUMP-CANN17.1 | | 2.6 | 1 | 5 | 10 | 16.00 | 21.00 |
| PUMP-CANN18.1 | | 2.6 | 1 | 5 | 10 | 16.00 | 21.00 |
| PUMP-CANN1.2 | | 2.82 | 1.82 | 5.22 | 2.04 | 2.04 | 4.28 |
| PUMP-CANN1.1 | | 4.26 | 2.76 | 6.86 | 10 | 15.00 | 21.00 |
| PUMP-CANN1.4 | | 1.04 | 1.04 | 5.04 | 4.34 | 8.68 | 7.38 |
| PUMP-CANN5.2 | | 2.8 | 1.3 | 6.3 | 4.34 | 6.51 | 13.89 |
| PUMP-CANN1.1 | | 2.5 | 1.5 | 5 | 2 | 2.00 | 4.00 |
| PUMP-CANN6.2 | | 2.65 | 1.35 | 4.97 | 30 | 30.00 | 60.60 |
| PUMP-JUB1.1 | | 1.85 | | 8.1 | 4.34 | 8.68 | 17.14 |
| PUMP-JUB1.2 | | 0.6 | | 6.05 | 2.85 | 5.69 | 7.84 |
| PUMP-JUB1.2 | | 2.07 | 0.57 | 3.67 | 10 | 15.00 | 13.00 |
| PUMP-JUB1A.2 | | 1.11 | | 6.86 | 3.66 | 7.32 | 11.04 |
| PUMP-SHUT1.2 | | 3 | 2 | 6 | 2.54 | 2.54 | 4.32 |
| PUMP-SHUT1.1 | | 2.6 | 1 | 5 | 2.54 | 4.36 | 5.33 |
| PUMP-SHUT1.1 | | 2.6 | 1 | 3 | 10 | 16.00 | 21.00 |

| PWWF FACTORS | | 0.680 | | | | 0.780 | | | | 0.880 | | | | PWWF | | | | HEAD GAIN |
|--------------|---------------|------------|-------------|---------------|-------|-------------|---------------|-------|-------------|---------------|-------|-------------|---------------|-------|------|--|--|-----------|
| Op. Storage | Emer. Storage | 2011 (L/s) | Op. Storage | Emer. Storage | 2026 | Op. Storage | Emer. Storage | 2031 | Op. Storage | Emer. Storage | 2036 | Op. Storage | Emer. Storage | 2036 | 2036 | | | |
| 2.82 | 108.16 | 37.35 | 3.36 | 125.36 | 43.53 | 3.46 | 141.49 | 49.13 | 4.24 | 158.68 | 53.20 | 4.24 | 158.68 | 53.20 | 21 | | | |
| 1.14 | 43.65 | 15.15 | 1.32 | 50.59 | 17.56 | 1.49 | 57.09 | 19.82 | 1.67 | 64.15 | 22.28 | 1.67 | 64.15 | 22.28 | 22 | | | |
| 3.07 | 117.89 | 40.93 | 3.56 | 136.64 | 47.44 | 4.02 | 154.21 | 53.55 | 4.51 | 173.28 | 60.17 | 4.51 | 173.28 | 60.17 | 32 | | | |
| 0.12 | 4.79 | 1.66 | 0.14 | 5.55 | 1.91 | 0.16 | 6.37 | 2.18 | 0.18 | 7.04 | 2.45 | 0.18 | 7.04 | 2.45 | 22.5 | | | |
| 0.02 | 0.62 | 0.21 | 0.02 | 0.72 | 0.25 | 0.02 | 0.81 | 0.28 | 0.02 | 0.91 | 0.32 | 0.02 | 0.91 | 0.32 | 0 | | | |
| 0.17 | 6.57 | 2.28 | 0.20 | 7.62 | 2.60 | 0.22 | 8.40 | 2.99 | 0.25 | 9.66 | 3.36 | 0.25 | 9.66 | 3.36 | 33 | | | |
| 0.37 | 14.12 | 4.90 | 0.43 | 16.36 | 5.68 | 0.48 | 18.47 | 6.41 | 0.54 | 20.75 | 7.21 | 0.54 | 20.75 | 7.21 | 3.9 | | | |
| 2.08 | 79.85 | 27.73 | 2.41 | 92.54 | 32.33 | 2.72 | 104.65 | 36.21 | 3.06 | 117.86 | 40.75 | 3.06 | 117.86 | 40.75 | 38 | | | |
| 0.14 | 5.24 | 1.82 | 0.16 | 6.07 | 2.11 | 0.18 | 6.86 | 2.38 | 0.20 | 7.70 | 2.68 | 0.20 | 7.70 | 2.68 | 35 | | | |
| 2.55 | 97.97 | 34.02 | 2.06 | 113.55 | 39.43 | 3.34 | 128.16 | 44.50 | 3.75 | 144.00 | 50.00 | 3.75 | 144.00 | 50.00 | 51 | | | |
| 0.03 | 1.02 | 0.35 | 0.03 | 1.18 | 0.41 | 0.03 | 1.33 | 0.46 | 0.04 | 1.60 | 0.52 | 0.04 | 1.60 | 0.52 | 19 | | | |
| 0.80 | 30.62 | 10.63 | 0.92 | 35.48 | 12.32 | 1.04 | 40.05 | 13.91 | 1.17 | 45.00 | 15.63 | 1.17 | 45.00 | 15.63 | 0 | | | |
| 4.54 | 174.21 | 60.49 | 5.26 | 201.92 | 70.11 | 5.93 | 227.89 | 79.13 | 6.67 | 256.06 | 86.51 | 6.67 | 256.06 | 86.51 | 64 | | | |
| 1.80 | 69.17 | 24.02 | 2.09 | 80.17 | 27.81 | 2.36 | 90.48 | 31.42 | 2.65 | 101.66 | 35.30 | 2.65 | 101.66 | 35.30 | 5.6 | | | |
| 0.38 | 14.53 | 5.04 | 0.44 | 16.84 | 5.85 | 0.49 | 19.03 | 6.69 | 0.56 | 21.36 | 7.42 | 0.56 | 21.36 | 7.42 | 0 | | | |
| 0.47 | 17.89 | 6.23 | 0.54 | 20.33 | 7.20 | 0.61 | 23.40 | 8.13 | 0.69 | 26.20 | 9.12 | 0.69 | 26.20 | 9.12 | 6.7 | | | |
| 0.16 | 13.47 | 4.85 | 0.42 | 16.19 | 5.62 | 0.48 | 18.28 | 6.35 | 0.53 | 20.53 | 7.13 | 0.53 | 20.53 | 7.13 | 96 | | | |
| 0.17 | 6.84 | 2.31 | 0.20 | 7.70 | 2.67 | 0.23 | 8.69 | 3.02 | 0.25 | 9.76 | 3.39 | 0.25 | 9.76 | 3.39 | 28 | | | |
| 0.10 | 3.85 | 1.34 | 0.12 | 4.46 | 1.55 | 0.13 | 5.04 | 1.75 | 0.15 | 5.66 | 1.97 | 0.15 | 5.66 | 1.97 | 108 | | | |

| STORAGE UPGRADES REQUIRED | 2036 | NOTES |
|--|------------------------------|---|
| Emergency storage beyond volume within wet well required | Op. Storage Upgrade Required | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | 2.71 | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| OK | | |
| OK | | |
| Emergency storage beyond volume within wet well required | | depth in the wet well is not great enough to trigger the pump |
| OK | | |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| OK | | |
| Emergency storage beyond volume within wet well required | | depth in the wet well is not great enough to trigger the pump |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | depth in the wet well is not great enough to trigger the pump |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |
| Emergency storage beyond volume within wet well required | | Pump capacity augmentation required |

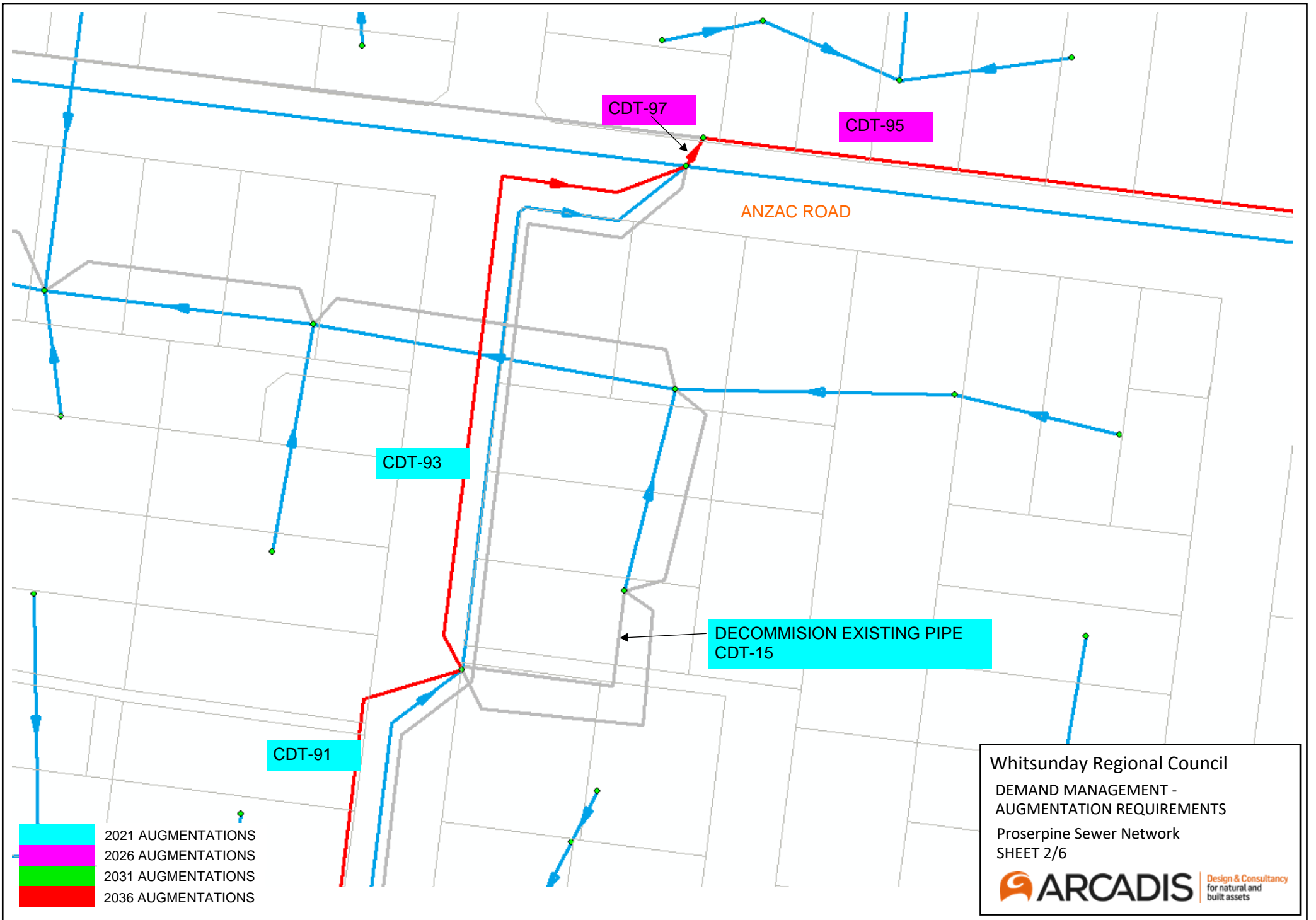


- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS


Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 1/6



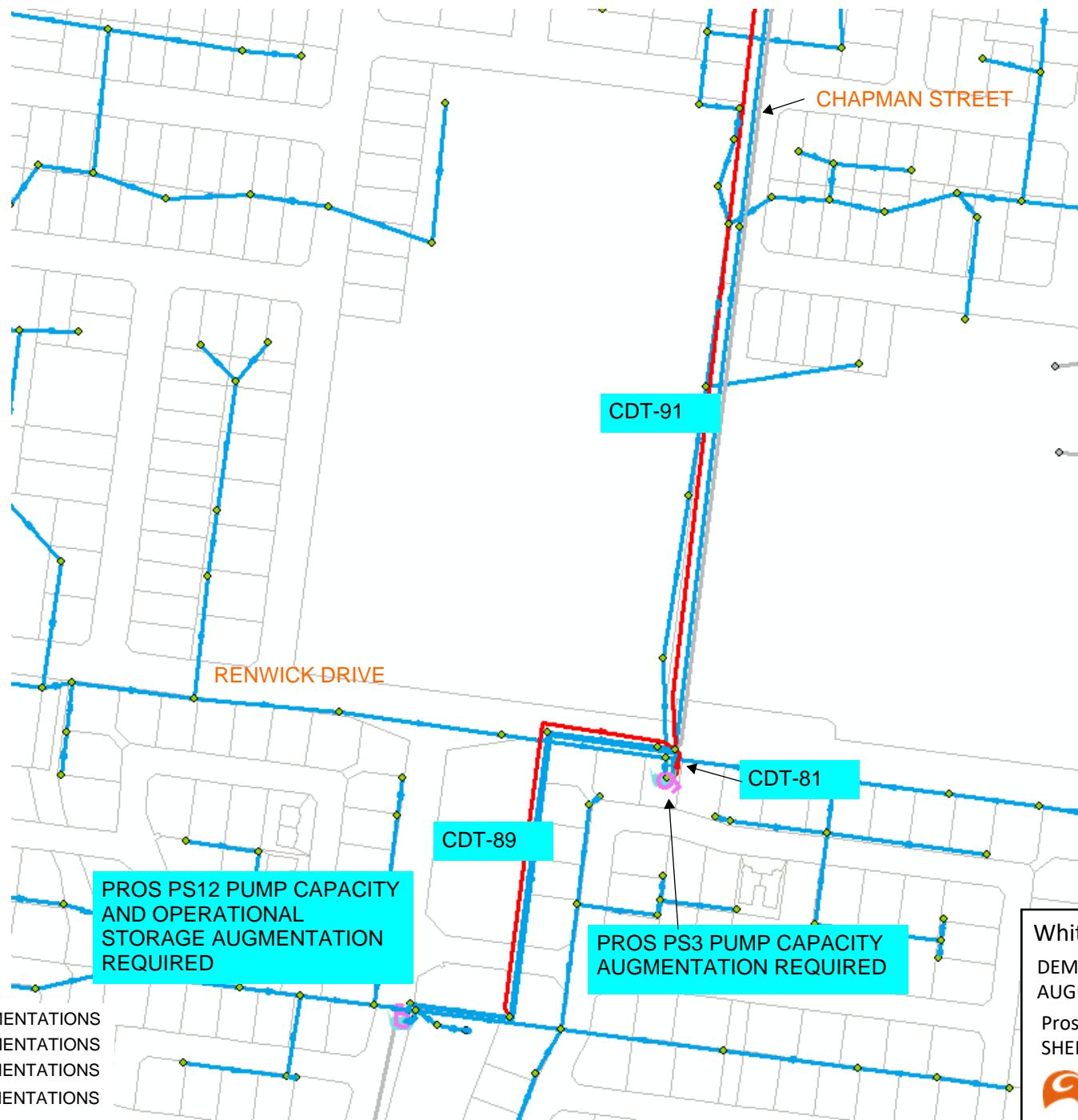
Design & Consultancy
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Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 2/6



Design & Consultancy
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- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 3/6



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- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

**PROS_6 PUMP CAPACITY
AUGMENTATION REQUIRED**

**PROS_PS9 PUMP CAPACITY
AUGMENTATION REQUIRED**

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 4/6



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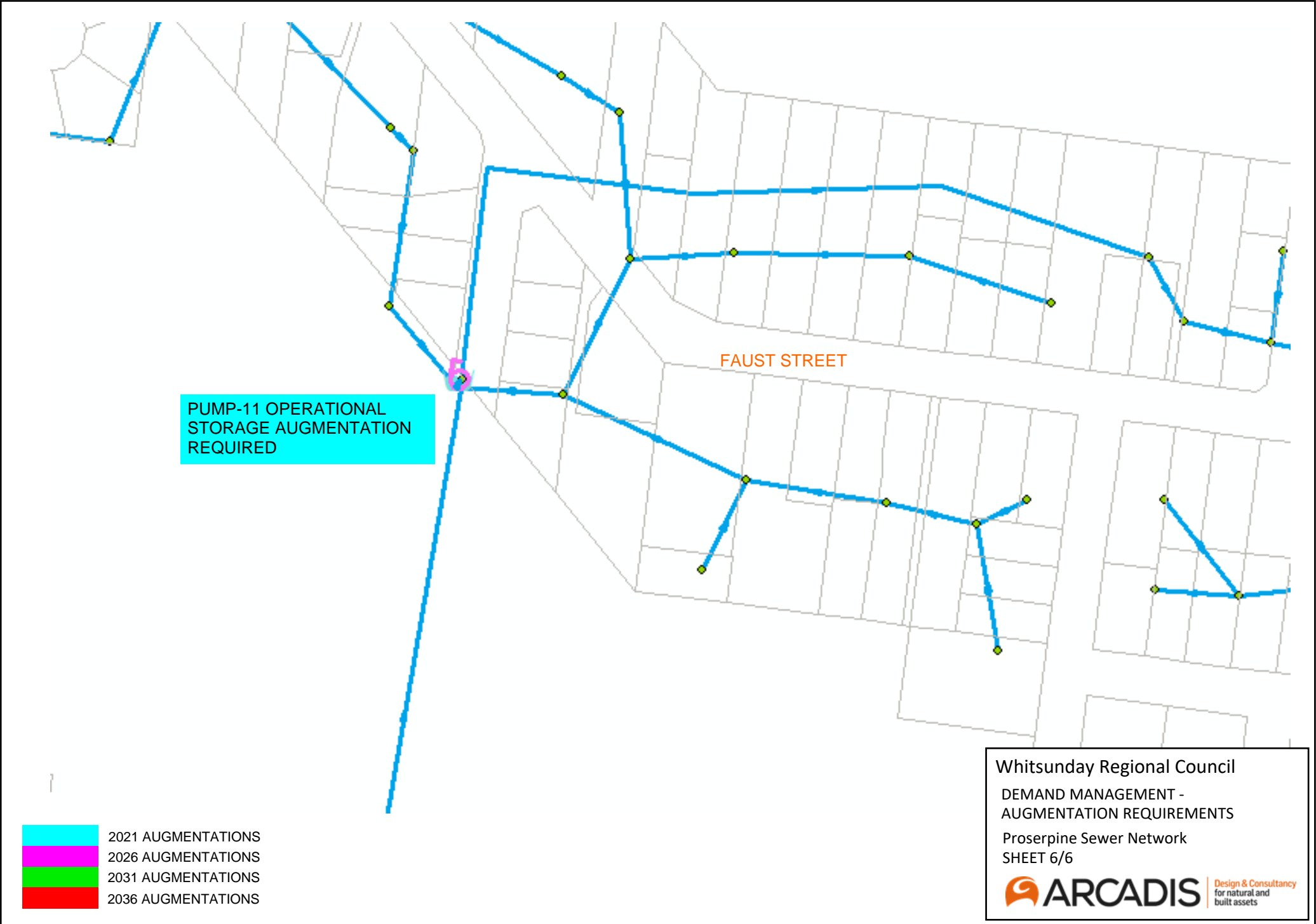
PROS_PS1 PUMP CAPACITY
AUGMENTATION REQUIRED

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Proserpine Sewer Network
 SHEET 5/6



Design & Consultancy
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PROJECT: Whitsundays Sewer Network Modelling
 DOCUMENT NUMBER: D007.10027536-AAC.01
 DATE:

Project Engineer: M.C.
 Software: InfoSWMM 5A

WHITSUNDAYS COLLINSVILLE SEWER NETWORK
 DEMAND MANAGEMENT ASSESSMENT - SEWER PUMP STATION ASSESSMENT SUMMARY

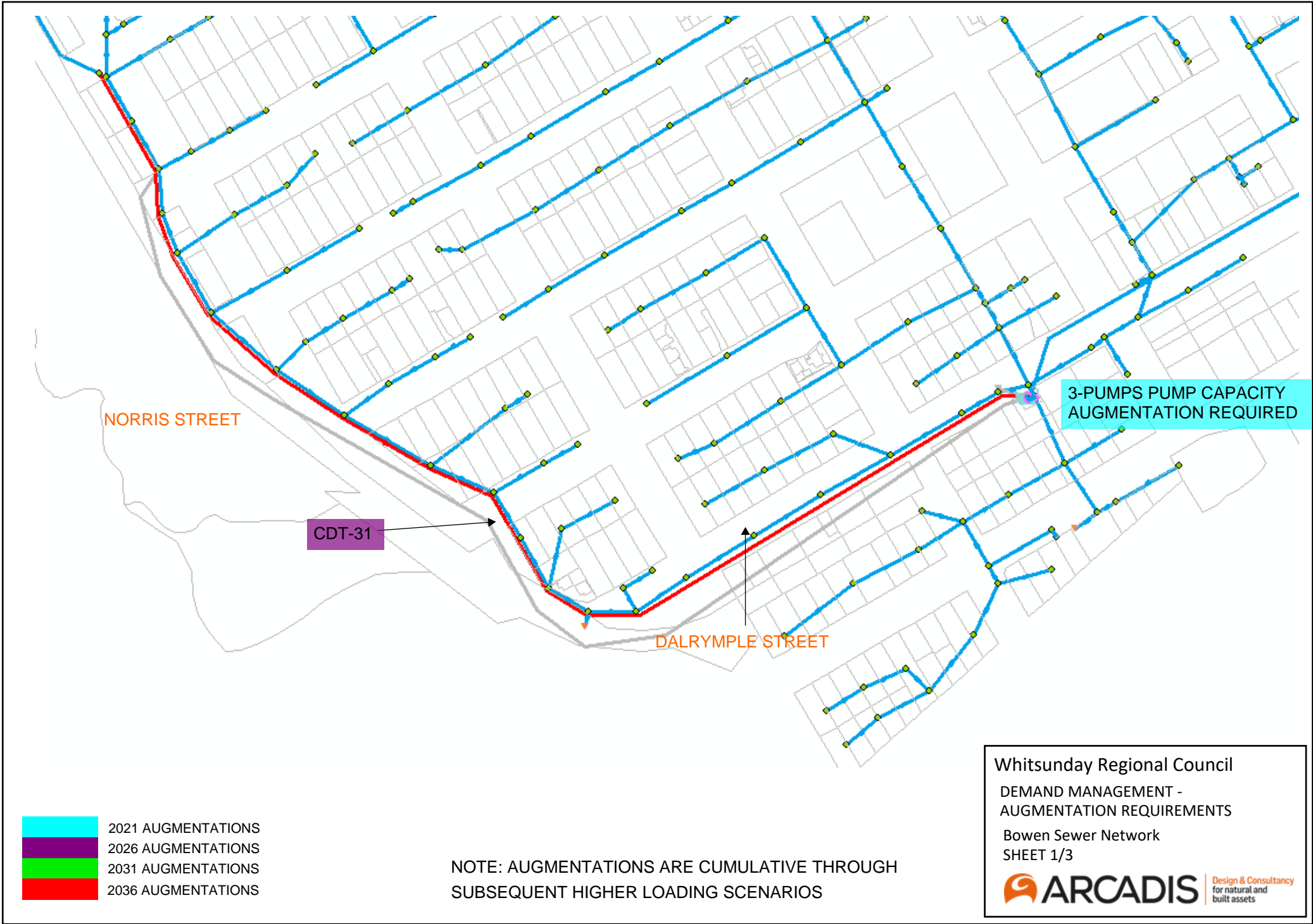


| PUMP | | WET WELL | | CURRENT | | |
|----------------|-------------------|-------------------|----------------|---------|-------------|-------------|
| Pump ID (Char) | Startup Depth (m) | Shutoff Depth (m) | Max. Depth (m) | Area | Op. Storage | Em. Storage |
| PROD_P51.1 | 0.7 | 0.3 | 5.2 | 3.08 | 1.592 | 5.715 |
| PROD_P51.1 | 2.6 | 1.1 | 8.1 | 7.31 | 10.965 | 38.012 |
| PROD_P510.1 | 0.78 | 0.35 | 3 | 2 | 0.8 | 3.84 |
| PROD_P511.1 | 0.61 | 0.21 | 3 | 2 | 0.8 | 4.18 |
| PROD_P512.1 | 0.8 | 0.4 | 6.3 | 2 | 0.8 | 10.4 |
| PROD_P52.1 | 1.1 | 0.5 | 7.85 | 7.31 | 5.808 | 41.8985 |
| PROD_P53.1 | 0.8 | 0.4 | 7.65 | 7.31 | 2.924 | 47.8805 |
| PROD_P54.1 | 1.8 | 0.8 | 6 | 1.33 | 1.33 | 1.69 |
| PROD_P55.1 | 1.4 | 0.5 | 5.77 | 1.33 | 1.197 | 5.4131 |
| PROD_P58.1 | 0.7 | 0.3 | 7 | 2.32 | 0.928 | 13.92 |
| PUMP.11 | 0.8 | 0.4 | 3 | 2 | 0.8 | 3.8 |

| PWWF FACTORS | | | | | | 0.930041152 | | | | | | 0.95473215 | | | | | | 0.979423883 | | | | | | PWWF | | With ideal pump curves | |
|--------------|-------------|------------------|-------------|-------------|------------------|-------------|-------------|------------------|-------------|-------------|------------------|-------------|-------------|------------------|-------------|-------------|------------------|-------------|-------------|------------------|-----------|--|--|------|--|------------------------|--|
| Op. Storage | Em. Storage | 2011 Op. Storage | Op. Storage | Em. Storage | 2036 Op. Storage | Op. Storage | Em. Storage | 2011 Op. Storage | Op. Storage | Em. Storage | 2036 Op. Storage | Op. Storage | Em. Storage | 2011 Op. Storage | Op. Storage | Em. Storage | 2036 Op. Storage | Op. Storage | Em. Storage | 2036 Op. Storage | HEAD GAIN | | | | | | |
| 3.94 | 151.40 | 52.57 | 4.05 | 155.42 | 53.97 | 4.15 | 159.44 | 55.36 | 4.24 | 162.79 | 56.525 | 32 | | | | | | | | | | | | | | | |
| 0.10 | 3.95 | 3.37 | 0.11 | 4.06 | 3.41 | 0.11 | 4.16 | 3.44 | 0.11 | 4.25 | 3.45 | 1.5 | | | | | | | | | | | | | | | |
| 1.01 | 10.66 | 11.77 | 1.06 | 40.71 | 14.13 | 1.09 | 41.76 | 14.50 | 1.11 | 42.64 | 14.805 | 40 | | | | | | | | | | | | | | | |
| 1.84 | 70.57 | 24.50 | 1.89 | 72.44 | 25.15 | 1.94 | 74.31 | 25.80 | 1.98 | 75.87 | 26.145 | 37 | | | | | | | | | | | | | | | |
| 0.56 | 21.51 | 7.47 | 0.57 | 22.08 | 7.67 | 0.59 | 22.65 | 7.86 | 0.60 | 23.13 | 8.03 | 4.6 | | | | | | | | | | | | | | | |
| 0.55 | 21.17 | 7.35 | 0.57 | 21.74 | 7.55 | 0.58 | 22.30 | 7.74 | 0.59 | 22.77 | 7.905 | 18.2 | | | | | | | | | | | | | | | |
| 0.76 | 29.25 | 10.16 | 0.78 | 30.03 | 10.43 | 0.80 | 30.80 | 10.70 | 0.82 | 31.45 | 10.92 | 16 | | | | | | | | | | | | | | | |

| UPGRADES REQUIRED | | 2036 | |
|--|------|------------------------------|--|
| | | Op. Storage Upgrade Required | |
| Emergency storage beyond volume within wet well required | | | |
| Emergency storage beyond volume within wet well required | 0.31 | | |
| Emergency storage beyond volume within wet well required | | | |
| Emergency storage beyond volume within wet well required | | | |
| Emergency storage beyond volume within wet well required | | | |
| Emergency storage beyond volume within wet well required | 0.02 | | |

NOTES
 PWWF reduced with pipe augmentations (R6.02 to S6.525)



NORRIS STREET

CDT-31

DALRYMPLE STREET

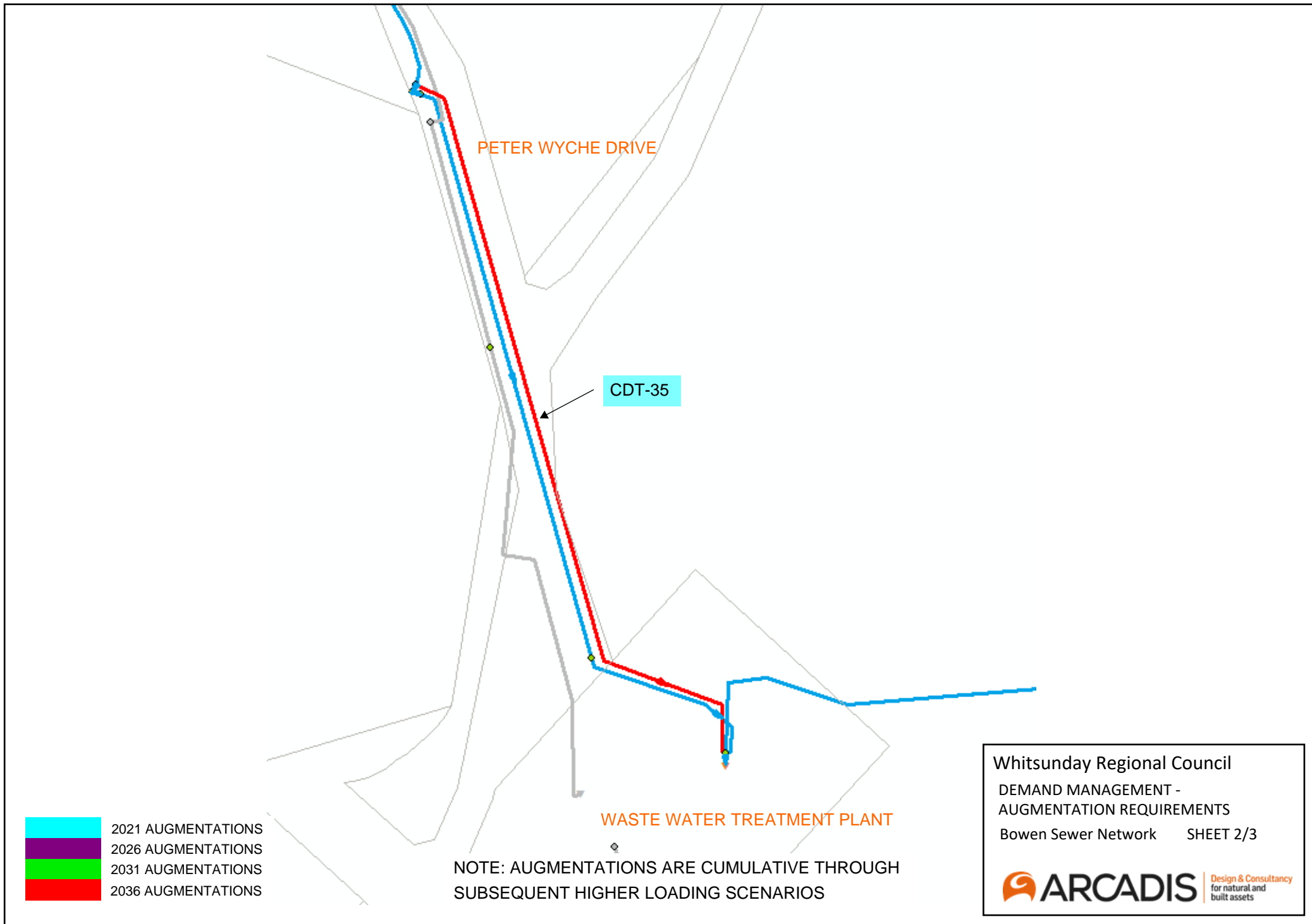
3-PUMPS PUMP CAPACITY AUGMENTATION REQUIRED

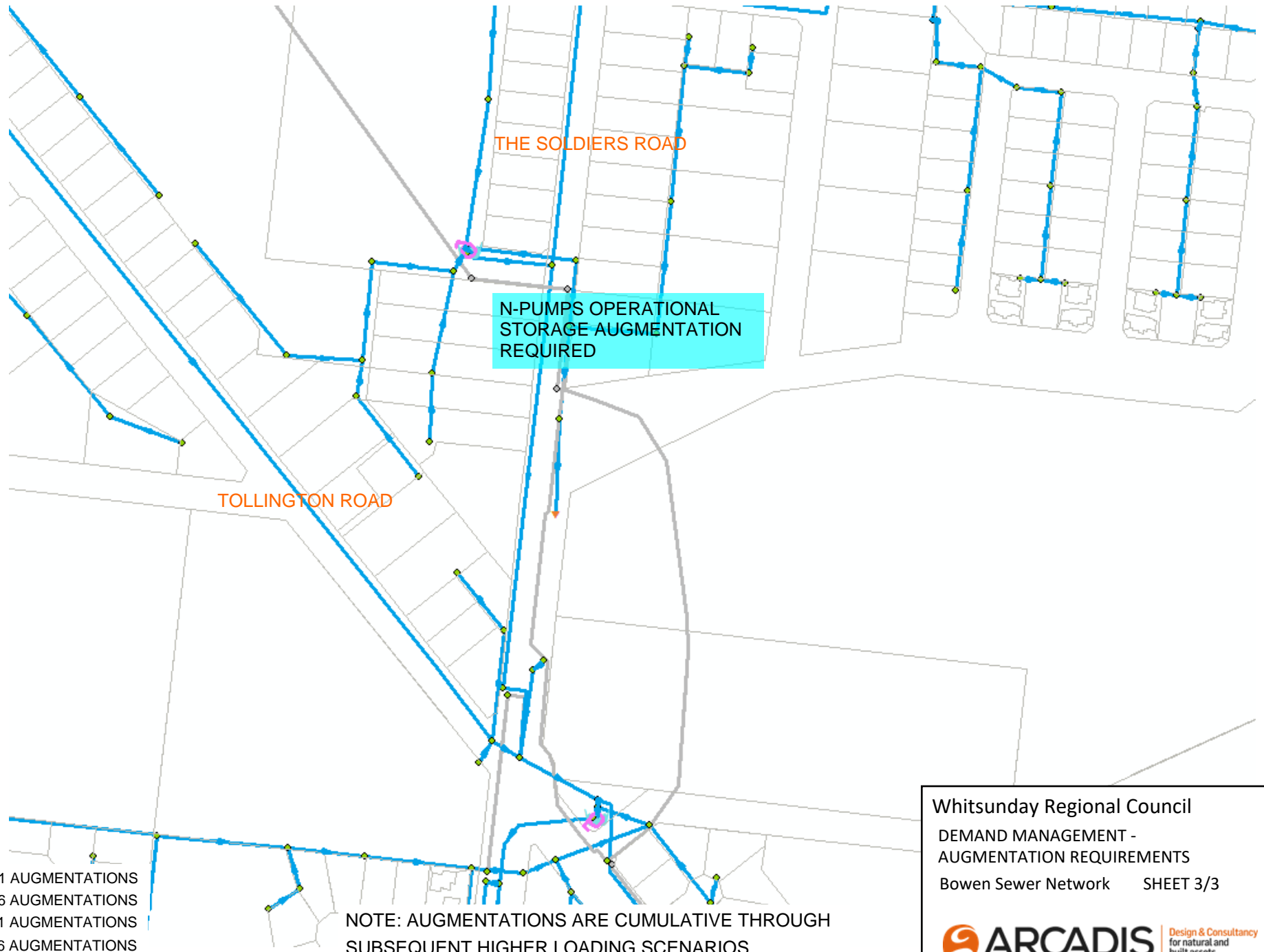
- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT - AUGMENTATION REQUIREMENTS
 Bowen Sewer Network
 SHEET 1/3

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THE SOLDIERS ROAD

N-PUMPS OPERATIONAL
STORAGE AUGMENTATION
REQUIRED

TOLLINGTON ROAD

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

NOTE: AUGMENTATIONS ARE CUMULATIVE THROUGH
SUBSEQUENT HIGHER LOADING SCENARIOS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Bowen Sewer Network SHEET 3/3



PROJECT: Whitsundays Sewer Network
 Modelling: Project Engineer: M.C.
 DOCUMENT NUMBER: D004-20027516-AAC-01 Software: INFOSWMM 5A
 DATE:



WHITSUNDAYS BOWEN SEWER NETWORK
 DEMAND MANAGEMENT ASSESSMENT - AUGMENTATION REQUIREMENT SUMMARY

| AUGMENTATION ID | LENGTH (m) | EXISTING PIPE SEGMENT | | DUPLICATION DN (mm) | DEPTH RANGE (m) | COST - \$/m | ADJUSTMENT FACTOR FOR SOA | SCALE FACTOR | 10% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW /DUPLICATION | PIPE REPLACEMENT | | | | 2021 | 2026 | 2031 | 2036 | |
|-----------------|------------|-----------------------|----------|---------------------|-----------------|-------------|---------------------------|--------------|-------------------------------|-----------------|--------------------------|------------------|-------------|----------------|-------------|------|------|------|------|----------------------|
| | | START NODE | END NODE | | | | | | | | | NOTES | ORIGINAL DN | REPLACEMENT DN | COST - \$/m | | | | | CAP & GROUT @ \$20/M |
| CDT-31 | 1268.96 | JCT-102 | 3 WELLS | 150 | | \$ 205.00 | 1.35 | 0.907 | 10% | 30% | \$ 444,177.31 | NEW RISING MAIN | 300 | GRAVITY | | | | | | |
| CDT-35 | 425.36 | P1-39 | P2-44 | 300 | | \$ 362.00 | 1.35 | 1.04 | 10% | 30% | \$ 303,090.65 | DUPLICATION RM | | 300 | | | | | | |
| TOTAL | | | | | | | | | | | | | | | | | | | | |

NOTES
 No flooding in 2021 but nearing top of manhole and causing backup in upstream line

PUMP AUGMENTATIONS REQUIRED

| PUMP STATION LABEL | AUGMENTATION TYPE | VALUE | COST - \$/A | TOTAL COST | 2021 | 2026 | 2031 | 2036 |
|--------------------|-------------------|--------------|-------------|--------------|------|------|------|------|
| PUMPS | PUMP CAPACITY | 2 x 22.5 MW* | \$4864/KW | \$193,820.00 | | | | |

*Assumes 70% efficiency at day

ADDITIONAL EMERGENCY STORAGE REQUIREMENTS HAVE NOT BEEN COSTED - TO BE VERIFIED AGAINST EXISTING UPSTREAM NETWORK CAPACITY (PIPEWORK + MANHOLES)


SCOTSVILLE ROAD

CDT-27

WASTE WATER TREATMENT PLANT

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
DEMAND MANAGEMENT -
AUGMENTATION REQUIREMENTS
Collinsville Sewer Network
SHEET 1/2



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2-C_PUMPS PUMP
CAPACITY AUGMENTATION
REQUIRED

STATION ROAD

- 2021 AUGMENTATIONS
- 2026 AUGMENTATIONS
- 2031 AUGMENTATIONS
- 2036 AUGMENTATIONS

Whitsunday Regional Council
 DEMAND MANAGEMENT -
 AUGMENTATION REQUIREMENTS
 Collinsville Sewer Network
 SHEET 2/2



Design & Consultancy
for natural and
built assets

PROJECT: Whitsundays Sewer Network
 Modelling: M.C.
 Project Engineer: M.C.
 DOCUMENT NUMBER: D000-2007/536-44C-01
 Software: InfoSWMM 5A
 DATE:



WHITSUNDAYS COLLINSVILLE SEWER NETWORK
 DEMAND MANAGEMENT ASSESSMENT - AUGMENTATION REQUIREMENT SUMMARY

| AUGMENTATION ID | LENGTH (m) | EXISTING PIPE SEGMENT | | DUPLICATION DN (mm) | DEPTH RANGE (m) | COST - \$/m | ADJUSTMENT FACTOR FOR SOL | SCALE FACTOR | 10% ADDITIONAL REGIONAL COSTS | 30% CONTINGENCY | COST OF NEW /DUPLICATION | PIPE REPLACEMENT | | | | | 2021 | 2026 | 2031 | 2036 | | |
|-----------------|------------|-----------------------|----------|---------------------|-----------------|-------------|---------------------------|--------------|-------------------------------|-----------------|--------------------------|------------------|-------------|----------------|-------------|----------------------|---------------|---------------|------|------|------------|---------------------------------------|
| | | START NODE | END NODE | | | | | | | | | NOTES | ORIGINAL DN | REPLACEMENT DN | COST - \$/m | CAP & GROUT @ \$20/M | | | | | TOTAL COST | COST INCREASE COMPARED TO DUPLICATION |
| CDT-22 | 174.724 | P1-2 | ICT-12 | 200 | | \$ 240.00 | 1.35 | 1.23 | 10% | 30% | \$ 97,511.31 | RESING MAIN | 225 | 375 | \$ 821.00 | \$ 3,405.48 | \$ 337,065.41 | \$ 239,554.11 | | | | |
| TOTAL | | | | | | | | | | | | \$ 97,511.31 | | | | | \$ 239,554.11 | | | | | |

NOTES
 This segment is solely to resolve velocity issues

PUMP AUGMENTATIONS REQUIRED

| PUMP STATION LABEL | AUGMENTATION TYPE | VALUE | COST - \$/m | TOTAL COST | 2021 | 2026 | 2031 | 2036 |
|--------------------|-------------------|---------------|-------------|--------------|------|------|------|------|
| LC-PUMPS | PUMP CAPACITY | 2 @ 7.7 l/s** | \$7204/M | \$116,608.46 | | | | |

*Assumes 70% efficiency at duty

ADDITIONAL EMERGENCY STORAGE REQUIREMENTS HAVE NOT BEEN COSTED - TO BE VERIFIED AGAINST EXISTING UPSTREAM NETWORK CAPACITY (PIPEWORK + MANHOLES)

PROJECT: Whitsundays Sewer Network Modelling
DOCUMENT NUMBER: D002 10027536.AAC.01
DATE:
Project Engineer: M.C
Software: InfoSWMM 5A



**WHITSUNDAYS COLLINSVILLE SEWER NETWORK
DEMAND MANAGEMENT ASSESSMENT - SEWER PUMP STATION ASSESSMENT SUMMARY**

| PUMP ID (Chal) | PUMP Startup Depth (m) | PUMP ShutOff Depth (m) | WET WELL | | CURRENT | |
|----------------|------------------------|------------------------|-----------|------|-------------|--------------|
| | | | Max Depth | Area | Op. Storage | Emr. Storage |
| I.C. PUMPS 1 | 1.15 | 0.61 | 5.11 | 15.8 | 8.532 | 47.828 |
| I.C. PUMPS 2 | 1.75 | 0.61 | 5.11 | 15.8 | 18.012 | 48.348 |
| I.C. PUMPS 1 | 1.21 | 0.71 | 4.61 | 3.66 | 1.81 | 11.4102 |
| I.C. PUMPS 2 | 2.27 | 0.71 | 4.61 | 3.66 | 5.7096 | 7.5306 |
| B.C. PUMPS 1 | 0.7 | 0.3 | 6.5 | 2.54 | 1.026 | 13.97 |
| B.C. PUMPS 2 | 0.9 | 0.3 | 6.5 | 2.54 | 3.524 | 19.862 |
| A.C. PUMPS 1 | 0.7 | 0.3 | 2.7 | 2.54 | 1.026 | 4.318 |
| A.C. PUMPS 2 | 0.9 | 0.3 | 2.7 | 2.54 | 3.524 | 8.81 |

| 0.9609375 | | | | | | 0.9661458 | | | | | | 0.9773958 | | | | | | PWW | | With ideal pump curves | |
|-------------|--------------|-----------|-------------|--------------|-------|-------------|--------------|-------|-------------|--------------|--------|-------------|--------------|--------|-------------|--------------|--------|-----------|--|------------------------|--|
| Op. Storage | Emr. Storage | 2021 8:00 | Op. Storage | Emr. Storage | 2026 | Op. Storage | Emr. Storage | 2031 | Op. Storage | Emr. Storage | 2036 | Op. Storage | Emr. Storage | 2036 | Op. Storage | Emr. Storage | 2036 | HEAD GAIN | | | |
| 3.33 | 127.93 | 44.42 | 3.35 | 128.62 | 44.66 | 3.46 | 132.78 | 46.10 | 3.47 | 133.13 | 46.225 | 3.47 | 133.13 | 46.225 | 3.47 | 133.13 | 46.225 | 28 | | | |
| 0.68 | 26.22 | 9.10 | 0.69 | 26.36 | 9.15 | 0.71 | 27.22 | 9.45 | 0.71 | 27.29 | 9.475 | 0.71 | 27.29 | 9.475 | 0.71 | 27.29 | 9.475 | 55 | | | |
| 0.68 | 26.22 | 9.10 | 0.69 | 26.36 | 9.15 | 0.71 | 27.22 | 9.45 | 0.71 | 27.29 | 9.475 | 0.71 | 27.29 | 9.475 | 0.71 | 27.29 | 9.475 | 0 | | | |
| 0.02 | 0.84 | 0.29 | 0.02 | 0.85 | 0.29 | 0.02 | 0.88 | 0.30 | 0.02 | 0.88 | 0.305 | 0.02 | 0.88 | 0.305 | 0.02 | 0.88 | 0.305 | 0 | | | |
| 0.07 | 2.71 | 0.94 | 0.07 | 2.73 | 0.95 | 0.07 | 2.82 | 0.98 | 0.07 | 2.82 | 0.98 | 0.07 | 2.82 | 0.98 | 0.07 | 2.82 | 0.98 | 4.8 | | | |

| STORAGE UPGRADES REQUIRED | 2036 Op. Storage Upgrade Required | NOTES |
|--|-----------------------------------|----------------------------------|
| Emergency storage beyond volume within wet well required | | |
| Emergency storage beyond volume within wet well required | | Pump augmentation required |
| OK | | Not enough depth to trigger pump |
| OK | | |

APPENDIX B

POTABLE WATER NETWORK RESERVOIR ASSESSMENT SUMMARY

APPENDIX C

SEWER NETWORK AUGMENTATION MAPPING AND SUMMARY

APPENDIX D

SEWER NETWORK PUMP STATION ASSESSMENT SUMMARY

APPENDIX E

POTABLE WATER & SEWER NETWORK MCA ASSESSMENTS

APPENDIX F

INITIAL WATER NETWORK AUGMENTATION OUTPUTS PRE-WORKSHOP

APPENDIX G

INITIAL SEWER NETWORK AUGMENTATION OUTPUTS PRE- WORKSHOP